DESIGN DRIVEN I CLIENT FOCUSED

March 30, 2020

City of Sherwood
Attention: Joy Chang
22560 SW Pine Street
Sherwood, OR 97140

Re: Tualatin-Sherwood Industrial Park - Land Use
LU 2020-001 Incompleteness Response
Project Number 2180459.04
Dear Joy:
Thank you for your incompleteness letter dated February 16, 2020, and the guidance it provided for the proposed fivebuilding Tualatin-Sherwood Industrial Park development. We have responded as noted in the items below; our replies follow staff's comments and additional material is listed in the exhibits below.

## INFORMATION NECESSARY TO COMPLETE APPLICATION

## 1. Clean Water Service Provider Letter.

Response: The applicant continues to coordinate with Clean Water Services staff in pursuit of a Service Provider Letter and will forward a copy of the Letter upon receipt.
2. Parking will not be allowed on SW Cipole Place per Engineering Comments. Narrative response tied to parking standards must be modified.
Response: The narrative has been updated to reflect the Engineering comments related to the proposed parking along SW Cipole Place. Parking will be located solely within the proposed parking lots for the five industrial buildings.
3. Ultimately 15 -copies of the complete application. Please generate once the application is deemed complete.

Response: An electronic copy of the application is being provided for staff's use. The applicant can also provide as many paper copies as are requested by staff.

While not specifically a completeness issue, the following must be addressed to comply with City requirements:
4. Engineering Comments dated January 28, 2020.

Response: Many of the Engineering comments have been incorporated into the revised civil plans. See further information below.

P 503.224.9560 : F 503.228.1285 : W MCKNZE.COM : RiverEast Center, 1515 SE Water Avenue, \#100, Portland, OR 97214

City of Sherwood

## TSCP - Land Use

Project Number 2180459.04
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## ENGINEERING COMPLETENESS REVIEW COMMENTS

The following responses have been provided by Jeff Shoemaker, PE, civil engineer at DOWL:

## Comments to Be Addressed prior to Packet Submittal to Planning Commission for Land Use Decision

## 1. Provide CWS service provider letter (SPL).

Response: The applicant continues to coordinate with Clean Water Services staff in pursuit of a Service Provider Letter and will forward a copy of the Letter upon receipt.
2. Need to get City Engineer approved Design Modification Request for the block length on future SW Blake Road in order to not extend SW Cipole Road to SW Blake Road (210.6E Intersection Spacing). Form on city website.
Response: A Design Modification Request has been submitted as a part of the updated package.
3. Need to get City Engineer approved Design Modification Request for cul-de-sac length in excess of standard (210.7 Cul-de-sacs, Eyebrows, Turnouts). Form on city website.
Response: A Design Modification Request has been submitted as a part of the updated package.
4. Need to get City Engineer approved Design Modification Request for cul-de-sac radius in excess of standard (Standard Drawing RD-10). Form on city website.
Response: A Design Modification Request has been submitted as a part of the updated package.
5. Need to get City Engineer approved Design Modification Request for no sidewalk on east side of SW Cipole Road (Standard Drawing RD-1). Form on city website.
Response: A Design Modification Request to eliminate the sidewalk and public utility easement on the east side of Cipole has been submitted as a part of the updated package.
6. No parking to be allowed on new street. Confirm not using on-street parking in parking calculations.

Response: This comment has been confirmed. No parking will be proposed on Cipole as requested by the City.
7. Need to show how subject development is getting phased and public improvements to be constructed with each phase.
Response: A preliminary construction sequencing plan has been attached.
8. Need to address how the new south leg of the SW Tualatin-Sherwood Road/SW Cipole Road intersection will function in relation to traffic signalization in the interim to SW Tualatin-Sherwood Road being widened.
Response: As agreed in our meeting with staff on February 18, 2020, the City will craft a condition as a part of the land use to satisfy Washington County requirements. Suggested language as follows: "Applicant shall coordinate access at the intersection of Cipole and Tualatin-Sherwood Road with Washington County."
9. Need to show how water quality treatment and hydro-modification is being provided for the street widening improvements along the west side of SW 124th Avenue.
Response: The drainage report dated March 4, 2020 has been updated to include the widening on 124th Avenue.

City of Sherwood

## TSCP - Land Use

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10. Label elevations on existing contours.

Response: Additional contour labels have been added to all grading sheets for additional clarity.
11. Need to provide information on how Cipole Road/T-S Road intersection is to work if construction entrance to site is located at the proposed Cipole Road extension. Addition of signalization on north side of intersection required? Possible alternative construction site entrance identified/coordinated with WACO?
Response: As agreed in our meeting with staff on February 18, 2020, the City will craft a condition as a part of the land use to satisfy Washington County requirements. Suggested language as follows: "Applicant shall coordinate construction access to the site with Washington County. Washington County approval shall be required for the issuance of site construction permits."

## Technical Review Comments from Engineering

1. Provide design information for reconfiguration of stormwater underground detention/treatment system for SW 124th Avenue.
Response: The drainage report dated March 4, 2020 has been updated to include the widening on 124 th Avenue.
2. Providing detention for the 25 -year event on top of the hydro-modification will make the project eligible for SDC detention credits.
Response: This comment has been noted.
3. Please note that meeting hydro-modification standards does not qualify the project for SDC credits for detention. You'll need to provide calculations for meeting the detention standards in the CWS design standards in order to quality for detention SDC credits.
Response: This comment has been noted.
4. The existing sanitary sewer shown at the southwest corner of the SW Tualatin-Sherwood Road/SW Oregon Street intersection is not accurate. Project will need to tie into the manhole at the southwest corner of the SW TualatinSherwood Road/SW Oregon Street intersection where the out pipe has a 12-inch diameter.
Response: The plans have been updated to show the sanitary line tying into the manhole at the SW corner of TualatinSherwood Road.
5. Extension of the public sanitary sewer shall be located behind the future south curb line of SW Tualatin-Sherwood Road with WACO concurrence.
Response: As discussed at the February 18, 2020 meeting, the County's planned location for the new storm line widening is behind the future southern curb line of Tualatin-Sherwood Road. We will need to work with Washington County and the City of Sherwood to locate the sanitary line alignment on Tualatin-Sherwood Road.
6. No public sanitary sewer mains are to be installed within private property, except within public utility easement dedicated to the City as approved by City Engineer.
Response: The sanitary configuration has been revised to eliminate proposed public utility easements for sanitary sewer outside of future ROW with the exception of the utility extension from the end of the Cipole cul-de-sac to future Blake Road.

City of Sherwood

## TSCP - Land Use

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7. The 16 -inch diameter public water line within SW Cipole Road north of SW Tualatin-Sherwood Road shall be extended southward along the proposed SW Cipole Road extension and/or public utility easement, being capped at the site developments southern property line. Another 16 -inch diameter line will also be run east along SW Tualatin Sherwood Road to SW 124th Avenue, then south along SW 124th Avenue to the future intersection with SW Blake Road, then westward and being capped at the west right-of-way line of SW 124th Avenue. These extensions will allow for future extension and completion of the looped public waterline within SW Blake Road by the future WWSP project. Blow off assemblies will be installed at the capped end of each water mainline stub.
Response: The alignment described above is reflected in the updated plans.
8. No public water line are to be installed within private property outside of the proposed SW Cipole Road extension right-of-way or public utility easement dedicated to the City as approved by the City Engineer. Easements for installation of public water service to fire vaults will be as close to the public right-of-way as possible.
Response: The water configuration has been updated to eliminate proposed public utility easements for water outside of future ROW with the exception of the utility extension from the end of the Cipole cul-de-sac to future Blake Road. Private to private water easements may still be required and have been reflected on the plans.
9. The existing storm basins in the preliminary storm report don't correlate with the existing contours.

Response: The drainage report dated March 4, 2020 has been updated to reflect the proper existing stormwater catchment basins.
10. Verify ground water depth at each pond and ensure that pond bottom is not below ground water level.

Response: The GeoDesign geotechnical report dated February 6, 2018 and supplementary memo dated December 23, 2019 reflect groundwater as follows:

- A groundwater elevation at approximately 176.5' at the closest boring location to the Tract E (TP-11). The tract E pond elevation is designed at 186.60.
- Groundwater not encountered at approximately $176.5^{\prime}$ at the closest boring location to the Tract C (B-2). The tract C pond elevation is designed at 184.85 .
- A ground water elevation at approximately 185 ' at the closest boring location to the Tract $B(B-6)$. The tract $B$ pond elevation is designed at 188.60.

11. Extend sanitary sewer, water and stormwater systems through the Cipole Road extension to the south property line in a straight alignment (no bends at south end). End sanitary and stormwater line extensions with cleanout. End waterline extension with a blow-off assembly.
Response: Plans have been updated to reflect a straight run from the end of the Cipole cul-de-sac to the termination in future Blake Road.
12. The invert information for SDMH-P4 is listed backwards for the pipelines. The treatment line invert should be the lower of the two, with the high flow bypass invert having the higher invert. Currently the high flow bypass invert is the lower of the two inverts, hence during normal rainfall event conditions no treatment is being performed.
Response: As discussed in the meeting with staff held March 4, 2020, DOWL has updated the plans to separate the storm line for Cipole Place collection from the line extending from future Blake Road, resolving any question on manhole SDMHP4.

City of Sherwood

## TSCP - Land Use

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13. Separate storm pipes outlets that are discharging into water quality ponds as far away from outlet structures as possible. Check to make sure that no short circuiting effects occur and that retention/treatment time is maintained.
Response: DOWL has updated the plans to maximize the distance from inlet to outlet of the ponds as much as feasible.
14. Need to install public storm sewer to serve Lot 3 and Lot 4 .

Response: Plans have been updated to reflect a public storm line and easements through Lots 1,2 , and 5 to serve Lots 1 , 3 , and 4.
15. Regional storm facilities require a sedimentation manhole prior to storm discharge to facility.

Response: Updated plans reflect sedimentation manholes on storm lines prior to discharge into the storm ponds.
Thank you for your consideration in beginning full review of the land use application. We understand that questions, comments, and suggestions from staff may come up within the review process. We welcome such dialogue and intend to respond promptly to inquiries and suggestions as they may arise. Please contact me at 971.346 .3742 or bvarricchione@mcknze.com if you have any questions.

Sincerely,


Brian Varricchione

## Enclosure(s): City of Sherwood Incompleteness Letter Dated February 16, 2020 Updated Application Package

c: Kirk Olsen - Trammell Crow Company Jeff Shoemaker - DOWL

February 16, 2020
Trammell Crow Company
Kirk Olsen
1300 SW 5 ${ }^{\text {th }}$ Avenue Suite 3050
Portland OR 97201

## RE: T-S Corporate Park, LU 2020-001 SP SUB CUP VAR Completeness Review

This letter is to confirm that the City received your application for a project type on January 17, 2020. A review by staff has determined that your application is incomplete at this time. Please provide the following for staff to deem the application complete and schedule this matter for a hearing.

1. Clean Water Services Service Provider Letter
2. Parking will not be allowed on SW Cipole Place per Engineering Comments. Narrative response tied to parking standards must be modified.
3. Ultimately 15-copies of the complete application. Please generate once the application is deemed complete.

While not specifically a completeness issue, the following must be addressed to comply with City requirements:
4. Engineering Comments dated January 28, 2020

Once your application is complete, we will schedule this matter for hearing. If you have any questions, please contact me at 503-625-4214 or changi@sherwoodoregon.gov.

In accordance with ORS 227.178(2) your application will be deemed complete once we have received:

1. All of the missing information noted; or
2. Some of the missing information and written notice that no additional information will be provided; or
3. Written notice that no additional information will be provided.

Please note that you have 180-days from the date of this letter to bring your application into completeness or the application becomes void per ORS 227.178(4).

Sincerely,


Attachment: COS Engineering Comments dated January 28, 2020
CC: David Kraska, WWSSC, via email
Corianne Burnett, WWSSC, via email
Bran Varricchione, Mackenzie, via email
Jeff Shoemaker, DOWL, via email

## Engineering Completeness Review Comments



To:
From:
Project:
Date:

Joy Chang, Senior Planner
Craig Christensen, P.E., Civil Engineer
Sherwood Industrial Park (LU 2020-001)
January 28, 2020

Engineering staff has reviewed the information provided for the above referenced project and has the following completeness comment(s):

Comments that need to be addressed prior to packet submittal to Planning Commission for Land Use Decision

1. Provide CWS service provider letter (SPL).
2. Need to get City Engineer approved Design Modification Request for the block length on future SW Blake Road in order to not extend SW Cipole Road to SW Blake Road (210.6E Intersection Spacing). Form on city website.
3. Need to get City Engineer approved Design Modification Request for cul-de-sac length in excess of standard (210.7 Cul-de-sacs, Eyebrows, Turnouts). Form on city website.
4. Need to get City Engineer approved Design Modification Request for cul-de-sac radius in excess of standard (Standard Drawing RD-10). Form on city website.
5. Need to get City Engineer approved Design Modification Request for no sidewalk on east side of SW Cipole Road (Standard Drawing RD-1). Form on city website.
6. No parking to be allowed on new street. Confirm not using on-street parking in parking calculations.
7. Need to show how subject development is getting phased and public improvements to be constructed with each phase.
8. Need to address how the new south leg of the SW Tualatin-Sherwood Road/SW Cipole Road intersection will function in relation to traffic signalization in the interim to SW Tualatin-Sherwood Road being widened.
9. Need to show how water quality treatment and hydro-modification is being provided for the street widening improvements along the west side of SW $124^{\text {th }}$ Avenue.
10. Label elevations on existing contours.
11. Need to provide information on how Cipole Road/T-S Road intersection is to work if construction entrance to site is located at the proposed Cipole Road extension. Addition of signalization on north side of intersection required? Possible alternative construction site entrance identified/coordinated with WACO?

## Technical Review Comments from Engineering

1. Provide design information for reconfiguration of stormwater underground detention/treatment system for SW $124^{\text {th }}$ Avenue.
2. Providing detention for the 25 -year event on top of the hydro-modification will make the project eligible for SDC detention credits.
3. Please note that meeting hydro-modification standards does not qualify the project for SDC credits for detention. You'll need to provide calculations for meeting the detention standards in the CWS design standards in order to quality for detention SDC credits.
4. The existing sanitary sewer shown at the southwest corner of the SW Tualatin-Sherwood Road/SW Oregon Street intersection is not accurate. Project will need to tie into the manhole at the southwest corner of the SW Tualatin-Sherwood Road/SW Oregon Street intersection where the out pipe has a 12 -inch diameter.
5. Extension of the public sanitary sewer shall be located behind the future south curb line of SW Tualatin-Sherwood Road with WACO concurrence.
6. No public sanitary sewer mains are to be installed within private property, except within public utility easement dedicated to the City as approved by City Engineer.
7. The 16 -inch diameter public water line within SW Cipole Road north of SW TualatinSherwood Road shall be extended southward along the proposed SW Cipole Road extension and/or public utility easement, being capped at the site developments southern property line. Another 16 -inch diameter line will also be run east along SW TualatinSherwood Road to SW $124^{\text {th }}$ Avenue, then south along SW $124^{\text {th }}$ Avenue to the future intersection with SW Blake Road, then westward and being capped at the west right-of-way line of SW $124^{\text {th }}$ Avenue. These extensions will allow for future extension and completion of the looped public waterline within SW Blake Road by the future WWSP project. Blow off assemblies will be installed at the capped end of each water mainline stub.
8. No public water line are to be installed within private property outside of the proposed SW Cipole Road extension right-of-way or public utility easement dedicated to the City as approved by the City Engineer. Easements for installation of public water service to fire vaults will be as close to the public right-of-way as possible.
9. The existing storm basins in the preliminary storm report don't correlate with the existing contours.
10. Verify ground water depth at each pond and ensure that pond bottom is not below ground water level.
11. Extend sanitary sewer, water and stormwater systems through the Cipole Road extension to the south property line in a straight alignment (no bends at south end). End sanitary and stormwater line extensions with cleanout. End waterline extension with a blow-off assembly.
12. The invert information for SDMH-P4 is listed backwards for the pipelines. The treatment line invert should be the lower of the two, with the high flow bypass invert having the higher invert. Currently the high flow bypass invert is the lower of the two inverts, hence during normal rainfall event conditions no treatment is being performed.
13. Separate storm pipes outlets that are discharging into water quality ponds as far away from outlet structures as possible. Check to make sure that no short circuiting effects occur and that retention/treatment time is maintained.
14. Need to install public storm sewer to serve Lot 3 and Lot 4 .

Project: $\quad$ Sherwood Industrial Park (LU 2020-001)
Date: $\quad$ February 7, 2020
Page: 3 of 3
15. Regional storm facilities require a sedimentation manhole prior to storm discharge to facility.

END OF COMMENTS

# PRELIMINARY <br> SUBDIVISION, SITE PLAN REVIEW, CONDITIONAL USE, AND VARIANCE 

## To

City of Sherwood

## For

T-S Corporate Park

## Dated

January 17, 2020
Revised March 30, 2020
Project Number
2180459.04

MACKENZE
Since 1960
RiverEast Center| 1515 SE Water Ave, Suite 100, Portla nd, OR 97214
PO Box 14310, Portland, OR 97293| T503.224.9560| www.mcknze.com

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## ATTACHMENTS

1. Application Form and Owner Authorization Letter
2. Title Report
3. Deed
4. Tax Map
5. Vicinity Map
6. Plans
7. Notes from August 16, 2018 Pre-Application Conference
8. Notes from July 18, 2019 Pre-Application Conference
9. Neighborhood Meeting Documentation
10. TriMet Information on Nearby Transit Service
11. Traffic Impact Analysis
12. Washington County Design Exception Approval and Supporting Evidence
13. Cipole Place Alternatives Analysis Drawings
14. Wetland Delineation Reports
15. Natural Resource Assessment Report
16. Preliminary Stormwater Report
17. Pride Disposal Service Provider Letter
18. Arborist Report
19. Clean Water Services Service Provider Letter
20. Engineering Design Modification Request for Cul-de-Sac Length
21. Engineering Design Modification Request for Cul-de-Sac Geometry
22. Engineering Design Modification Request for Blake Road Block Length
23. Engineering Design Modification Request to Exclude Sidewalk and PUE on East Side of Cipole Place
24. Oregon Department of State Lands Wetland Delineation Approval
25. Construction Sequencing Diagrams

## MATERIALS PROVIDED SEPARATELY

1. Application fee
2. Mailing labels for properties within 1,000 feet of site
3. Land Use Submittal Checklists
4. Electronic copy of all submittal materials

## I. PROJECT SUMMARY

| Applicant: | Trammell Crow Company, Attn: Kirk Olsen 1300 SW 5th Avenue, Suite 3050 <br> Portland, OR 97201 <br> KOIsen@trammellcrow.com |
| :---: | :---: |
| Owner: | Willamette Water Supply System Commission, Attn: David Kraska 1500 NW Bethany Boulevard, Suite 305 <br> Beaverton, OR 97006 <br> david.kraska@tvwd.org |
| Site Address: | 12822 SW Tualatin-Sherwood Road Sherwood, OR 97140 |
| Washington County Tax Lot: | Tax Lot 2S128D001100 |
| Site Area: | 46.5 acres |
| Zoning: | Employment Industrial (EI) |
| Comprehensive Plan: | Industrial |
| Adjacent Zoning: | North: City of Sherwood General Industrial (GI) and City of Tualatin General Manufacturing (MG) <br> East: City of Tualatin Manufacturing Business Park (MBP) <br> South: City of Sherwood Employment Industrial (EI) and Washington County Future Development, 20-acre (FD-20) <br> West: Washington County Future Development, 20-acre (FD-20) |
| Existing Structures: | None (Vacant Lot) |
| Request: | Preliminary Subdivision, Site Plan Review, Conditional Use, and Variance application for a five-building industrial park, totaling 535,194 square feet within the Tonquin Employment Area (TEA) |
| Project Contact: | Mackenzie, Attn: Brian Varricchione 1515 SE Water Avenue, Suite 100 <br> Portland, OR 97214 <br> (503) 224-9560 <br> bvarricchione@mcknze.com |

## II. INTRODUCTION

## Description of Request

The applicant, Trammell Crow Company, requests Type III Preliminary Subdivision, Site Plan Review, Conditional Use, and Variance approval for five industrial buildings totaling approximately 535,194 square feet (SF) with associated parking and site improvements on an approximately 46.5-acre Employment Industrial (EI) zoned site in the City of Sherwood, Oregon (Washington County Tax Lot 2S128D001100). Two pre-application conferences regarding the proposed development were conducted on August 16, 2018 and July 18, 2019, with the City of Sherwood.

As the El zone has restrictions which limit the size of standalone warehousing and distribution uses to 150,000 square feet unless a Conditional Use Permit is obtained, this application requests a Conditional Use Permit to authorize Building C to have an area of 183,292 SF. The application also requests variance relief from the cul-de-sac standards of Section 16.106.040.E.

## Existing Site and Surrounding Land Use

The 46.53-acre site, located at the southwest corner of SW Tualatin-Sherwood Road and SW 124th Avenue, slopes downhill from south to north with steeper grades near the south property line. Blackberries and trees exist on the southern and western portions of the site. A wetland delineation by Pacific Habitat Services (PHS) has identified three wetlands on the site (Attachments 14 and 24), the southernmost of which is contiguous with similar features in the undeveloped property south of the site. Per the Department of State Lands Statewide Wetland Inventory Map, a 4.20-acre freshwater emergent wetland exists approximately a quarter of an acre away from the property. Due to area topography, stormwater runoff flows into the site from the southwesterly neighboring undeveloped land, making portions of the project area susceptible to wetlands and water dependent plant species. The PHS Natural Resource Assessment report (Attachment 15) provides recommendations for on-site wetlands preservation to maintain the ecological integrity of these resource features.

Several structures associated with the site's previous farming activities have been removed from the eastern portion of the site. The rest of the property remains undeveloped. Properties north of the site are within the city limits of Sherwood and Tualatin. These developments are industrial in nature and include Conrad Lumber Company, Columbia Corrugated Box Co., Inc., and Packaging Resources. To the west of the property is the City of Tualatin municipal water storage tank in unincorporated Washington County. The undeveloped properties to the east are in the City of Tualatin, and the undeveloped properties to the south are in the City of Sherwood or unincorporated Washington County.

The site was recently annexed into the City of Sherwood (application AN 19-002) and is identified as Parcel 1 of Partition Plat 2019-029, recorded at Washington County on September 19, 2019. The two parcels created by the partition are now separated by the future alignment of Blake Road extending westward from 124th Avenue south of the site. The site is located within the EI zone, which was created specifically for the Tonquin Employment Area.


Figure 1: Site Aerial Photo

## Proposed Development

The project proposes the construction of five speculative warehouse buildings for manufacturing and warehouse use, totaling approximately 535,194 square feet. The overall site will provide 671 on-site parking spaces.

| Building Square Footage and Parking |  |  |
| :---: | :---: | :---: |
| Building | Building Square Footage | Parking Count |
| A | 87,490 SF | 152 |
| B | 56,576 SF | 124 |
| C | 183,292 SF | 181 |
| D | 145,624 SF | 127 |
| E | 62,212 SF | 87 |
| Total | $\mathbf{5 3 5 , 1 9 4 ~ S F}$ | $\mathbf{6 7 1}$ |

Site access will be achieved from SW Tualatin-Sherwood Road at an existing signalized intersection where SW Cipole Road forms the north leg. Washington County Land Use and Transportation staff has previously indicated that the site would not be permitted to have direct driveway access to SW 124th Avenue. SW Cipole Place is proposed as a cul-de-sac extending into the site to allow for ample vehicular access and circulation throughout the proposed industrial campus. Public utilities will be extended to the site to accommodate industrial development.

The applicant has constructed similar buildings in the Tualatin-Sherwood Road corridor (in the City of Tualatin) and has utilized this familiarity to inform the design of the project. The speculative buildings, which will be similar to that illustrated in the plans (Attachment 6), are designed to accommodate a range of industrial tenants, whether manufacturing, light industrial, or warehouse/distribution. The buildings will utilize concrete tilt-up construction with ample glass at office locations to emphasize customer- and public-facing entrances. The buildings can accommodate a range of tenant space demands and may have single or multiple users. Each building will provide numerous loading docks for users and appropriate parking consistent with the applicant's experience with market demand in the Tualatin-Sherwood Road corridor.

## Subdivision Request

To provide maximum flexibility for end users, many of whom wish to purchase their own sites rather than leasing, the applicant is proposing a five-lot subdivision with varying lot sizes, which in turn accommodate varying building sizes. The subdivision also proposes the creation of five tracts, three of which are for public stormwater management facilities and two of which will contain wetlands and tree areas. No development is proposed within the wetland areas.

There is a chance that the applicant will later choose not to subdivide the property, in which case the final plat would never be filed. As a result, the applicant requests that conditions of approval be specific to each land use approval so it is clear which conditions would not apply in the event that the property remains a single parcel.

## Conditional Use Permit Request

In the El zone, standalone warehousing and distribution uses exceeding 150,000 SF require Conditional Use Permit (CUP) approval. This application requests a CUP to allow Building C, containing 183,292 SF, to accommodate a future warehousing/distribution tenant exceeding 150,000 SF without requiring a separate land use review and approval, if such a user were to propose utilizing Building C .

## Public Improvements and Transportation

## Right-of-Way Dedication and Public Improvements

The site abuts SW Tualatin-Sherwood Road and SW 124th Avenue, both of which are classified as fivelane arterial roadways under Washington County jurisdiction. This street standard requires a minimum 102-foot right-of-way ( 51 feet from centerline) for SW Tualatin-Sherwood Road and a minimum 98 -foot right-of-way ( 49 feet from centerline) for SW 124th Avenue. As required the conditions of approval when the property was divided into two parcels (when the site was in unincorporated Washington County), the property owner dedicated this required right-of-way on the recently recorded partition plat. Based on direction from County staff, the applicant anticipates that the County will require additional right-of-way dedication to accommodate turn lanes.

The applicant proposes to dedicate additional right-of-way along both SW Tualatin-Sherwood Road and SW 124th Avenue to allow for street improvements to arterial standards. SW 124th Avenue, which was recently constructed by Washington County, restricts access to abutting properties. Washington County's Tualatin-Sherwood Road (Teton Avenue to Langer Farms Parkway) project will widen Tualatin-Sherwood Road to five lanes (including the frontage along the subject parcel), with construction beginning in the summer of 2021, so the applicant does not propose to improve SW Tualatin-Sherwood Road. The applicant proposes to widen SW 124th Avenue and add sidewalk.

The applicant proposes a new public cul-de-sac, Cipole Place, opposite the existing Cipole Road where it intersects SW Tualatin-Sherwood Road.

The site does not have City or County public utilities adjacent to the site except for storm lines in abutting streets. As illustrated on Sheets C6.0-C6.6 in Attachment 6, the applicant proposes to extend water and sanitary sewer infrastructure from their nearest locations in and near SW Tualatin-Sherwood Road. Stormwater from the proposed development will be managed by multiple shared facilities (extended dry basins) as illustrated on Sheets C5.0-C5.2.

The applicant proposes to dedicate a public utility easement (PUE) from the terminus of the proposed Cipole Place cul-de-sac to the southern site boundary, to provide a public utilities corridor to serve additional industrial development to the south in the future.

## Transportation Impact Analysis

Kittelson \& Associates transportation engineers projected site trip generation (Attachment 11) based on Land Use Code 130 - Industrial Park within the Institute of Transportation Engineers' (ITE) Trip Generation Manual, 10th edition. The Transportation Impact Analysis (TIA) indicates that the proposed development will generate 1,844 weekday trips, 219 of which will occur in the AM peak hour and 219 of which will occur within the PM peak hour. ${ }^{1}$ The report analyzed traffic operations in the vicinity in the years 2021 and 2025, both with and without the proposed development:

- In 2021, the SW Oregon Street/SW Tualatin-Sherwood Road intersection is expected to exceed mobility standards (i.e., experience unsatisfactory performance, with intersection delay that exceeds target parameters) in the PM peak hour, with or without the proposed development.
- In 2021, the proposed development would cause the SW Oregon Street/SW Tonquin Road intersection to exceed mobility standards in the PM peak hour.
- In 2021, all other intersections in the study area are anticipated to meet mobility standards in both the AM and PM peak hours.
- In 2025 (following Washington County's planned improvements to SW Tualatin-Sherwood Road), the SW Oregon Street/SW Tonquin Road intersection is expected to exceed mobility standards in the PM peak hour, with or without the proposed development.
- In 2025, all other intersections in the study area are anticipated to meet mobility standards in both the AM and PM peak hours.

The TIA provides a recommended proportionate cost share allocation towards the future conversion of the SW Oregon Street/SW Tonquin Road intersection, either to a roundabout or a signalized intersection.

The TIA also notes that the site is currently served by TriMet bus route 97 on SW Tualatin-Sherwood Road, with service every half hour from 6:20 to 9:30 AM and from 3:10 to 7:00 PM on weekdays and no service on weekends.

[^0]
## Site Access and Cul-de-Sac Variance

The applicant seeks to construct a public cul-de-sac, not an internal drive as noted in the transportation planning documents, which will allow the property to be subdivided. By creating multiple parcels accessible from a public roadway, the applicant will be able to offer each building for sale (and/or for lease). A north-south connecting roadway, instead of a cul-de-sac, was studied and remains impractical. The length of the proposed cul-de-sac is longer than code in order to serve the eastern section of the property, so a variance is requested.

The applicant has verified the following related to roadway infrastructure at the property:

1) Prior transportation planning documents call for one access point to the property.

- Located at existing signalized intersection on Tualatin-Sherwood Road
- Assumed to be an "internal drive"
- No other access points to adjacent roads are permitted

2) Transportation planning documents do not recommend a through-road or connecting roadway that bisects the property.
3) There is no significant system-wide traffic benefit with a connecting roadway; a cul-desac does not negatively impact the regional system in the short or long term.
4) A connecting road is not feasible, since its design would result in grades between $6 \%$ and 15\%.

- Road grades over $5 \%$ are impractical, especially for a road serving an industrial park
- Dangerous conditions would be created at intersections (driveways and Blake Road)

5) A connecting road would have negative impacts, including:

- $\quad 64$ to 220 jobs lost (due to reduced building area)
- $\quad \$ 5.4$ million to $\$ 18.6$ million lost property tax revenue (due to reduced building area) over a 50 -year period
- $\$ 610,000$ to $\$ 1.2$ million additional cost for roadway extension from cul-de-sac

The City of Sherwood and Washington County transportation planning documents call for a single access point to the property at the signalized intersection of SW Cipole Road and SW Tualatin-Sherwood Road; no other access point to the property is proposed. This anticipated access point at Cipole Road was assumed to be "an internal drive." However, instead of a private drive serving the five-building industrial park, the applicant proposes to construct a public street. A public street extending into the property will enable the applicant to subdivide the property, thereby allowing each building to be sold, as opposed to leased, to an occupying company.

The "internal drive" access point recommended in the transportation planning documents conflicted with Washington County code which requires access to an arterial road (i.e., SW Tualatin-Sherwood Road) to be from another arterial or collector. Thus, the applicant sought and was granted a Design Exception from Washington County (Attachment 12). The County is allowing access to the property at the SW Cipole Road intersection that is a lower class of roadway or driveway; an arterial or collector street is not a requirement at the access point.

None of the City or County transportation planning documents call for a roadway of any classification through the property. Rather, the documents propose a north-south arterial along the property's eastern
boundary (i.e., SW 124th Avenue) and an east-west collector to the south of the property (i.e., Blake Road). The planners concluded any north-south roadway through the property would be impractical due to topographical conditions, among other considerations/constraints. The topographical challenges have been exacerbated since the proposed Blake Road alignment has shifted 600 feet to the north as part of the recent property partition approved by Washington County.

To achieve a relatively flat site for efficient industrial use, the site is designed as a multi-building industrial campus oriented to SW Tualatin-Sherwood Road. If not for market demand from users that seek to own their buildings, it would be possible to keep the site as a single parcel and utilize a central, shared private drive. However, the applicant wants to have the option to divide the property into a one-building-per-lot configuration, which requires public street frontage and public utility connections for each of the proposed lots. The purpose of the proposed public cul-de-sac is to allow all five lots to meet public street frontage and utility connection requirements - and ultimately for users to be able to purchase buildings.

Based on City staff requests, the applicant analyzed a connecting roadway compared to a cul-de-sac. Data from the TIA shows there is no significant system-wide benefit of a north-south road that bisects the property compared to a roadway that terminates as a cul-de-sac. Traffic engineers' analysis recommends that the proposed five-building development can be constructed as planned with the cul-de-sac while meeting the traffic mobility and safety standards established for the surrounding transportation system.

In addition to traffic operational analyses, the applicant studied the construction feasibility of a connecting roadway southward to the future alignment of Blake Road in multiple scenarios. Since the street would need to overcome a significant elevation difference of 45 feet, the resulting road grades would be between $6 \%$ and $15 \%$. These grades are simply non-conforming to a roadway serving an industrial park with trucks making turns into driveways. The grades would be dangerous and unattractive to users that prefer grades less than $5 \%$. Further, it would add between $\$ 610,000$ to $\$ 1.2$ million in project cost.

Side slopes from the roadway would decrease the building area of the project. Reduced building areabetween 38,000 SF and $132,100 \mathrm{SF}$-would result in up to 220 fewer jobs and $\$ 18.6$ million in lost property tax revenue over the life of the buildings (assumed to be 50 years). For all the reasons stated above, the cul-de-sac design solution was selected as the most appropriate roadway design.

The proposed cul-de-sac (SW Cipole Place) has a length of approximately 550 feet. Its length is due to

- the fact that there is no access permitted from SW 124th Avenue,
- the location and shape of the 2.4 -acre wetland in the central part of the site,
- the applicant's desire to avoid impacting the natural resource, and
- the need for the eastern portion of the property to be served by the road and utilities.

The applicant is requesting a variance to exceed Sherwood's 200 -foot length standard for cul-de-sacs and to utilize a cul-de-sac rather than construct a through street connection to SW Blake Road. This standard seems to be geared toward residential development, rather than large-lot industrial park development, and there appear to be situations where this length variance has been granted (e.g., SW Greengate Place which is 1,500 feet long and constructed relatively recently). The Tualatin Valley Fire \& Rescue fire marshal has approved the cul-de-sac and its length, which is not an unusual condition (e.g., the recentlyconstructed 975 -foot cul-de-sac and 915 -foot cul-de-sac in Tualatin, near SW 115th Avenue in the Koch Business Park). The applicant has also submitted an associated Engineering Design Modification request (Attachment 20).

Detailed justification for the variance and an explanation of the alternatives analysis (Attachment 13) is found in the responses to Chapter 16.84 below.

## III. NARRATIVE AND COMPLIANCE

The following narrative addresses the specific Sherwood Zoning and Community Development Code (Sherwood Municipal Code Title 16) approval criteria and development standards that apply to the proposed project.

## Division II. - Land Use and Development

## Chapter 16.31 - Industrial Land Use Districts

### 16.31.010-Purpose

A. Employment Industrial (EI) - The El zoning district provides employment areas that are suitable for, and attractive to, key industries and industry clusters that have been identified by the State of Oregon and the City's economic development strategy as important to the state and local economy. The following are preferred industry sectors for areas zoned El: Clean Technology; Technology and Advanced Manufacturing; and Outdoor Gear and Active Wear.

Land zoned El shall provide for large and medium-sized parcels for industrial campuses and other industrial sites that can accommodate a variety of industrial companies and related businesses. Areas zoned El are also intended to provide the opportunity for flex building space within smalland medium-sized industrial campuses and business parks to accommodate research and development companies, incubator/emerging technology businesses, related materials and equipment suppliers, and/or spin-off companies and other businesses that derive from, or are extensions of, larger campus users and developments. Retail and commercial uses are allowed only when directly supporting area employers and employees.

Industrial establishments and support services shall not have objectionable external features and shall feature well-/andscaped sites and attractive architectural design, as determined by the Hearing Authority.
Response: The proposed development is speculative, so specific users are not known at this time. The applicant will seek users consistent with the City's economic development objectives and zoning regulations. Depending on market demand, users may include manufacturing, warehouse/distribution, or other permitted uses.

### 16.31.020-Uses

A. The table below identifies the land uses that are permitted outright $(P)$, permitted conditionally $(C)$ and not permitted ( $N$ ) in the industrial zoning districts. The specific land use categories are described and defined in Chapter 16.88.
B. Uses listed in other sections of this Code, but not within this specific table are prohibited.
C. Any use not otherwise listed that can be shown to be consistent or associated with the uses permitted outright or conditionally in the industrial zones or contribute to the achievement of the objectives of the industrial zones may be permitted outright or conditionally, utilizing the provisions of Chapter 16.88.
D. Additional limitations for specific uses are identified in the footnotes of this table.

| 16.31.020 - Permitted Uses within the El Zone (Excerpt) |  |
| :---: | :---: |
| Office and Professional Support Services | E1 |
| Business and Professional Offices ${ }^{3}$ | P |
| Business Support such as duplicating, photocopying, mailing services, fax, and computer facilities ${ }^{3}$ | P |
| Any incidental business, service, processing storage or display, not otherwise permitted, that is essential to and customarily associated with a use permitted outright, provided said incidental use is conducted entirely within an enclosed building. | $p$ |
| Industrial |  |
| Manufacture, compounding, processing, assembling, packaging, treatment, fabrication of products contained wholly within an enclosed building provided exterior odor and noise is consistent with municipal code standards and there is no unscreened storage and not otherwise regulated elsewhere in the code | P |
| Manufacture, compounding, processing, assembling, packaging, treatment, fabrication of products not otherwise prohibited elsewhere in the code provided other off-site impacts are compliant with local, state and federal regulations | C |
| Manufacture, compounding, processing, assembling, packaging, treatment, or fabrication of acids, paints, dyes, soaps, ammonia, chlorine, sodium compounds, fertilizer, herbicides, insecticides and similar chemicals | $N$ |
| Distribution, warehousing and storage associated with a permitted use operating on the same site | P |
| Distribution and warehousing up to 150,000 square feet, provided product(s) are stored within an enclosed building ${ }^{9}$ | P |
| Distribution and warehousing greater than 150,000 square feet provided product(s) are stored within an enclosed building ${ }^{9}$ | C |
| Medical or dental laboratories, including biomedical compounding | P |
| Laboratories (not medical or dental) | $P$ |
| Research and development and associated manufacturing | $P$ |
| Notes: <br> ${ }^{1}$ See special criteria for the El zone, 16.31 .030 and the Tonquin Employment Area (TEA), <br> ${ }^{3}$ Limited in size to five thousand $(5,000)$ square feet in a single outlet and no more than $(20,000)$ square feet in multiple outlets in the same development project. <br> ${ }^{9}$ For standalone warehousing and distribution only. Warehousing and distribution associa approved use is ancillary and permitted without size limitations. |  |

Response: The proposed development is speculative in nature, with no specific users at this time. Future uses of the development may include manufacturing, warehouse/distribution, or other allowed uses of the Employment Industrial Zone. This application includes a request for a conditional use permit to allow a potential standalone warehouse/distribution use in Building C to be over 150,000 SF. This standard is met.

### 16.31.030 - Development Standards

A. Generally

No lot area, setback, yard, landscaped area, open space, off-street parking or loading area, or other site dimension or requirement, existing on, or after, the effective date of this Code shall be reduced below the minimum required by this Code. Nor shall the conveyance of any portion of a lot, for other than a public use or right-of-way, leave a lot or structure on the remainder of said lot with less than minimum Code dimensions, area, setbacks or other requirements, except as permitted by Chapter 16.84 (Variances and Adjustments).
B. Development Standards

Except as otherwise provided, required minimum lot areas and dimensions and setbacks shall be:

| 16.31.030 - Development Standards by Zone |  |  |  |
| :---: | :---: | :---: | :---: |
| Development Standards | 4 | GI | El |
| Lot area - Industrial Uses: | 10,000 SF | 20,000 SF | 3 acres $^{9}$ |
| Lot area - Commercial Uses (subject to Section 16.31.050) | 10,000 SF | 20,000 SF | 10,000 SF |
| Lot width at front property line: | 100 feet |  |  |
| Lot width at building line: | 100 feet |  |  |
| Front yard setback ${ }^{11}$ | 20 feet | None | 20 feet |
| Side yard setback ${ }^{10}$ | None | None | None |
| Rear yard setback ${ }^{11}$ | None | None | None |
| Corner lot street side ${ }^{11}$ | 20 feet | None | 20 feet |
| Height ${ }^{11}$ | 50 feet |  |  |
| Notes: <br> ${ }^{9}$ Lots within the El zone that were legal lots of record prior to October 5, 2010 and smaller than the minimum lot size required in the table below may be developed if found consistent with other applicable requirements of Chapter 16.31 and this Code. Further subdivision of lots smaller than three (3) acres shall be prohibited unless Section 16.31 .050 applies. <br> ${ }^{10}$ When a yard is abutting a residential zone or public park, there shall be a minimum setback of forty (40) feet provided for properties zoned Employment Industrial and Light Industrial Zones, and a minimum setback of fifty (50) feet provided for properties zoned General Industrial. <br> ${ }^{11}$ Structures located within one-hundred (100) feet of a residential zone shall be limited to the height requirements of that residential zone. |  |  |  |

Response: The proposed T-S Corporate Park is located wholly in the El zone, which requires a minimum lot width of 100 feet, including 100 feet of frontage at the front property line. Lot 1 will have over 100 feet of street frontage along SW Tualatin-Sherwood Road; Lots 2 and 3 will have sufficient frontage on SW Cipole Court, and Lots 4 and 5 will have sufficient frontage on SW 124th Avenue. The proposed subdivision will divide the property into five lots and five tracts with no corner lots proposed. As shown in the preliminary subdivision plan (see Sheet C8.00, Attachment 6), all lots will be greater than 3 acres in size and maintain lot widths in excess of 100 feet. The site does not abut a residential zone on any side and thus requires a minimum front setback of 20 feet ( 20 feet for a street side setback), and 0 -foot setback for all non-street side and rear setbacks. As reflected in Attachment 6 Sheets A0.10-A0.12, all five proposed industrial buildings are sited over 60 feet from adjacent lot lines on all sides. The height of the buildings ranges from 40.5 feet to 48 feet, complying with the 50 foot height limit within the El zone. The industrial zone development standards for nonresidential development in the El zone are met.

### 16.31.040 - Employment Industrial (EI) Restrictions

## A. Use Restrictions

1. Retail and professional services that cater to daily customers, such as restaurants and financial, insurance, real estate, legal, medical and dental offices, shall be limited in the EI zone.
a. New buildings for stores, branches, agencies or other retail uses and services shall not occupy more than five thousand $(5,000)$ square feet of sales or service area in a single outlet and no more than twenty thousand $(20,000)$ square feet of sales or service area in multiple outlets in the same development project, and
b. New buildings for stores, branches, agencies or other retail uses and services shall not be located on lots or parcels smaller than five (5) acres in size. A "development project" includes all improvements proposed through a site plan application.
Response: No retail or professional services that cater to daily customers are proposed. The nature of the site will be wholly industrial for speculative warehousing, manufacturing and light industrial uses. This standard does not apply.
2. Notwithstanding the provisions of Section 16.31.050 "Commercial Nodes Use Restrictions," commercial development permitted under 16.31.050(1)(a) may only be proposed concurrent with or after industrial development on the same parcel. Commercial development may not occur prior to industrial development on the same parcel.
Response: The proposed development is industrial in nature. No commercial uses are proposed at this time; however, this standard will apply to tenanting and operation of the property following development.

## B. Land Division Restrictions

1. Lots of record prior to October 5, 2010 that are smaller than the minimum lot size required in the El zone may be developed if found consistent with other applicable requirements of Chapter 16.31 and this code. Further subdivision of lots smaller than three (3) acres shall be prohibited unless Section 16.31 .050 applies.
2. Lots or parcels larger than fifty (50) acres may be divided into smaller lots and parcels pursuant to a Planned Unit Development approved by the city so long as the resulting division yields at least one (1) lot or parcel of at least 50 acres in size.
3. Lots or parcels fifty (50) acres or larger, including those created pursuant to subsection (2) above, may be divided into any number of smaller lots or parcels pursuant to a Planned Unit Development approved by the city so long as at least forty (40) percent of the area of the lot or parcel has been developed with industrial uses or uses accessory to industrial use.
Response: The proposed project includes the subdivision of the 46.5 -acre parcel into five lots, all of which are greater than three acres, see table below and Sheet A0.10 in Attachment 6. Subsections 1, 2 and 3 are not applicable because the subject property contains a total area of 46.5 acres - larger than the minimum 3-acre lot area in the El zone, but smaller than 50 acres.

| Proposed Lot Areas |  |  |
| :---: | :---: | :---: |
| Lot | Area (SF) | Area (ac) |
| 1 | 231,767 SF | 5.32 ac |
| 2 | $162,691 \mathrm{SF}$ | 3.73 ac |
| 3 | $392,410 \mathrm{SF}$ | 9.00 ac |
| 4 | $348,540 \mathrm{SF}$ | 8.00 ac |
| 5 | $196,251 \mathrm{SF}$ | 4.51 ac |

16.31.050 - Tonquin Employment Area (TEA) Commercial Nodes Use Restrictions
A. Within the Tonquin Employment Area (TEA), only commercial uses that directly support industrial uses located within the TEA are permitted as conditional uses.
B. Commercial development, not to exceed a total of five (5) contiguous acres in size, may be permitted.
C. Commercial development may not be located within three hundred (300) feet of SW 124th Avenue or SW Oregon Street, and must be adjacent to the proposed east-west collector street.
Response: Commercial development or uses are not proposed at this time. The nature of the T-S Corporate Park is proposed to be wholly industrial; however, a future tenant could seek Conditional Use Permit approval to locate within the T-S Corporate Park. This standard is met.

### 16.31.060 - Community Design

For standards relating to off-street parking and loading, energy conservation, historic resources, environmental resources, landscaping, access and egress, signs, parks and open space, on-site storage, and site design, the applicable provisions of Divisions V, VIII and IX will apply.
Response: The proposed development has been designed to meet the provisions of the Sherwood Development Code Divisions V and VIII as presented herein. Division IX does not apply as there are no historic resources on site. These standards are addressed elsewhere in the narrative.

### 16.31.070-Floodplain

Except as otherwise provided, Section 16.134 .020 shall apply.
Response: According to Flood Insurance Rate Map 41067C0606F, dated October 19, 2018, the site is not within a regulated floodplain. This standard does not apply.

## Chapter 16.58 - Clear Vision and Fence Standards

### 16.58.010 - Clear Vision Areas

A. $\quad$ clear vision area shall be maintained on the corners of all property at the intersection of two (2) streets, intersection of a street with a railroad, or intersection of a street with an alley or private driveway.
B. A clear vision area shall consist of a triangular area, two (2) sides of which are lot lines measured from the corner intersection of the street lot lines for a distance specified in this regulation; or, where the lot lines have rounded corners, the lot lines extended in a straight line to a point of intersection, and so measured, and the third side of which is a line across the corner of the lot joining the non-intersecting ends of the other two (2) sides.
C. A clear vision area shall contain no planting, sight obscuring fence, wall, structure, or temporary or permanent obstruction exceeding two and one-half (21/2) feet in height, measured from the top of the curb, or where no curb exists, from the established street center line grade, except that trees exceeding this height may be located in this area, provided all branches and foliage are removed to the height of seven (7) feet above the ground on the sidewalk side and ten (10) feet on the street side.
The following requirements shall govern clear vision areas:

1. In all zones, the minimum distance shall be twenty (20) feet.
2. In all zones, the minimum distance from corner curb to any driveway shall be twenty-five (25) feet.
3. Where no setbacks are required, buildings may be constructed within the clear vision area.

Response: Clear vision areas are illustrated on the plan sheets at street intersections and driveway locations. No buildings and no sight-obscuring obstructions are proposed within the clear vision areas. This standard is met.
16.58.020-Fences, Walls and Hedges.
C. Applicability: The following standards apply to walls, fences, hedges, lattice, mounds, and decorative toppers. The standards do not apply to vegetation, sound walls and landscape features up to four (4) feet wide and at least twenty (20) feet apart.
D. Location-Residential Zone:

1. Fences up to forty-two (42) inches high are allowed in required front building setbacks.
2. Fences up to six (6) feet high are allowed in required side or rear building setbacks, except fences adjacent to public pedestrian access ways and alleys shall not exceed forty-two (42) inches in height unless there is a landscaped buffer at least three (3) feet wide between the fence and the access way or alley.
3. Fences on corner lots may not be placed closer than eight (8) feet back from the sidewalk along the corner-side yard.
4. All fences shall be subject to the clear vision provisions of Section 16.58.010.
5. A sound wall is permitted when required as a part of a development review or concurrent with a road improvement project. A sound wall may not be taller than twenty (20) feet.
6. Hedges are allowed up to eight (8) feet tall in the required side and rear setbacks.

Response: The subject parcel is zoned Employment Industrial (EI) and is not in a residential zone. These standards do not apply.
E. Location-Non-Residential Zone:

1. Fences up to eight (8) feet high are allowed along front, rear and side property lines, subject to Section 16.58.010. (Clear Vision) and building department requirements.
2. A sound wall is permitted when required as a part of a development review or concurrent with a road improvement project. A sound wall may not be taller than twenty (20) feet.
3. Hedges up to twelve (12) feet tall are allowed, however, when the non-residential zone abuts a residential zone the requirements of section 16.58.030.d.6. shall apply.
Response: Four-foot-tall black chain link fencing is proposed around the stormwater management facilities, which is below the eight-foot maximum. Black chain link fencing would also be the likely material for fall protection at the tops of tall retaining walls. This standard is met.
F. General Conditions-All Fences:
4. Fences must be structurally sound and maintained in good repair. A fence may not be propped up in any way from the exterior side.
5. Chain link fencing is not allowed in any required residential front yard setback.
6. The finished side of the fence must face the street or the neighboring property. This does not preclude finished sides on both sides.
7. Buffering: If a proposed development is adjacent to a dissimilar use such as a commercial use adjacent to a residential use, or development adjacent to an existing farming operation, a buffer plan that includes, but is not limited to, setbacks, fencing, landscaping, and maintenance via a homeowner's association or managing company must be submitted and approved as part of the preliminary plat or site plan review process per Section 16.90.020 and Chapter 16.122.
8. In the event of a conflict between this Section and the clear vision standards of Section 16.58.010, the standards in Section 16.58.010 prevail.
9. Fences and walls cannot be located within or over a public utility easement without an approved right-of-way permit.
10. The height of a fence or wall is measured from the actual adjoining level of finished grade measured six (6) inches from the fence. In the event the ground is sloped, the lowest grade within six (6) inches of the fence is used to measure the height.

Response: Four-foot-tall black chain link fencing is proposed around the stormwater management facilities. Since the site is industrial rather than residential, chain link fencing is acceptable. Black chain link fencing would also be the likely material for fall protection at the tops of tall retaining walls. The fencing is not proposed to violate the conditions outlined above. This standard is met.

## Chapter 16.60-Yard Requirements

### 16.60.010-Through Lots

On a through lot the front yard requirements of the zone in which such a lot is located shall apply to the street frontage where the lot receives vehicle access; except where access is from an alley, the front yard requirements shall apply to the street opposite the alley.
Response: The proposed development includes through lots that will receive vehicle access from the proposed cul-de-sac (Lots 4 and 5). Development on these lots meet the front yard requirements of the El zone, 20 feet, as measured from Cipole Place rather than 124th Avenue. This standard is met.

### 16.60.020 - Corner Lots

On a corner lot, or a reversed corner lot of a block oblong in shape, the short street side may be used as the front of the lot provided:
A. The front yard setback shall not be less than twenty-five (25) feet; except where otherwise allowed by the applicable zoning district and subject to vision clearance requirements.
B. The side yard requirements on the long street side shall conform to the front yard requirement of the zone in which the building is located.
Response: No corner lots are proposed as all property abutting street intersections are occupied by proposed stormwater tracts. This standard does not apply.
16.60.030 - Yards
A. Except for landscaping, every part of a required yard (also referred to as minimum setback) shall be open and unobstructed from its lowest point to the sky, except that architectural features such as awnings, fire escapes, open stairways, chimneys, or accessory structures permitted in accordance with Chapter 16.50 (Accessory Structures) may be permitted when so placed as not to obstruct light and ventilation.
B. Where a side or rear yard is not required, and a primary structure is not erected directly on the property line, a primary structure must be set back at least three (3) feet.
Response: Minimum setbacks at this location are 20 feet abutting streets and zero feet elsewhere. As illustrated on Attachment 6 Sheets A0.10-A0.12, no buildings are proposed within minimum setbacks, and no buildings are proposed within three feet of a property line. This standard is met.
16.60.040 - Lot Sizes and Dimensions
A. If a lot or parcel, or the aggregate of contiguous lots or parcels, recorded or platted prior to the effective date of this Code, has an area or dimension which does not meet the requirements of this Code, the lot or aggregate lots may be put to a use permitted outright, subject to the other requirements of the zone in which the property is located.
Response: The development is proposed to be fully compliant with area and dimension standards for the El zone. This standard does not apply.

## B. Exceptions

1. Residential uses are limited to a single-family dwelling, or to the number of dwelling units consistent with the density requirements of the zone. However, a dwelling cannot be built on a lot with less area than thirty-two hundred $(3,200)$ square feet, except as provided in Chapter 16.68.
2. Yard requirements of the underlying zone may be modified for infill developments as provided in Chapter 16.68 (Infill Development).
Response: No residential uses are proposed, and no setback/yard modifications are requested for infill development. This standard does not apply.

### 16.70.020 - Neighborhood Meeting

A. The purpose of the neighborhood meeting is to solicit input and exchange information about the proposed development.
B. Applicants of Type III, IV and V applications are required to hold a meeting, at a public location for adjacent property owners and recognized neighborhood organizations that are within 1,000 feet of the subject application, prior to submitting their application to the City. Affidavits of mailing, sign-in sheets and a summary of the meeting notes must be included with the application when submitted. Applicants for Type II land use action are encouraged, but not required to hold a neighborhood meeting.

1. Projects requiring a neighborhood meeting in which the City or Urban Renewal District is the property owner or applicant shall also provide published and posted notice of the neighborhood meeting consistent with the notice requirements in 16.72.020.
Response: A neighborhood meeting was conducted on December 4, 2019 to discuss the proposed development, as documented in Attachment 9. This standard has been met.

## Division III. - Administrative Procedures

## Chapter 16.72 - Procedures for Processing Development Permits

### 16.72.010-Generally

A. Classifications

Except for Final Development Plans for Planned Unit Developments, which are reviewed per Section 16.40.030, all quasi-judicial development permit applications and legislative land use actions shall be classified as one of the following:
2. Type II

The following quasi-judicial actions shall be subject to a Type II review process:
a. Land Partitions
b. Expedited Land Divisions - The Planning Director shall make a decision based on the information presented, and shall issue a development permit if the applicant has complied with all of the relevant requirements of the Zoning and Community Development Code. Conditions may be imposed by the Planning Director if necessary to fulfill the requirements of the adopted Comprehensive Plan, Transportation System Plan or the Zoning and Community Development Code.
c. "Fast-track" Site Plan review, defined as those site plan applications which propose less than 15,000 square feet of floor area, parking or seating capacity of public, institutional, commercial or industrial use permitted by the underlying zone, or up to a total of $20 \%$ increase in floor area, parking or seating capacity for a land use or structure subject to a Conditional Use Permit, except as follows: auditoriums, theaters, stadiums, and those applications subject to Section 16.72.010.A.4.
d. "Design Upgraded" Site Plan review, defined as those site plan applications which propose between 15,001 and 40,000 square feet of floor area, parking or seating capacity and which propose a minimum of eighty percent (80\%) of the total
possible points of design criteria in the "Commercial Design Review Matrix" found in Section 16.90.020.D.6.d.
e. Industrial "Design Upgraded" projects, defined as those site plan applications which propose between 15,001 and 60,000 square feet of floor area, parking or seating capacity and which meet all of the criteria in Section 16.90.020.D.7.b.
f. Homeowner's association street tree removal and replacement program extension.
g. Class B Variance
h. Street Design Modification
i. Subdivisions between 4-10 lots
j. Medical marijuana dispensary permit

Response: The applicant proposes to subdivide the property into five lots so a Type II land use review will be triggered as part of this development request. However, due to the extent of development requiring site plan review, a CUP, and Class A variance, the applicant is seeking consolidated subdivision review with those Type III and IV applications.
3. Type III

The following quasi-judicial actions shall be subject to a Type III review process:
a. Conditional Uses
b. Site Plan Review - between 15,001 and 40,000 square feet of floor area, parking or seating capacity except those within the Old Town Overlay District, per Section 16.72.010.A.
c. Subdivisions between 11-50 lots.

Response: The proposed development includes five speculative buildings, one of which (Building C) exceeds $150,000 \mathrm{SF}$. To allow for the possibility that the building will be used for warehouse/distribution, the applicant is requesting a Conditional Use Permit, which requires Type III review.
4. Type IV

The following quasi-judicial actions shall be subject to a Type IV review process:
a. Site Plan review and/or "Fast Track" Site Plan review of new or existing structures in the Old Town Overlay District.
b. All quasi-judicial actions not otherwise assigned to a Hearing Authority under this section.
c. Site Plans - Greater than 40,000 square feet of floor area, parking or seating capacity.
d. Site Plans subject to Section 16.90.020.D.6.f.
e. Industrial Site Plans subject to Section 16.90.020.D.7.b.
f. Subdivisions - over 50 lots.
g. Class A Variance

Response: The proposed development consists of five industrial buildings, each of which has greater than 40,000 SF of floor area and parking so Type IV site plan review is required. The extent of development will include a Class A Variance triggering a Type IV review process. The industrial site plan, however, will not be subject to section 16.90.020.D.7.b as the development project will meet the provisions of section 16.90.020.D.7.a.
5. Type V

The following legislative actions shall be subject to a Type V review process:
a. Plan Map Amendments
b. Plan Text Amendments
c. Planned Unit Development - Preliminary Development Plan and Overlay District. Response: The proposal does not include Plan Map Amendments, Plan Text Amendments, or a Planned Unit Development Preliminary Development Plan and Overlay District. This standard does not apply.

## C. Approval Criteria

1. The approval criteria for each development permit application shall be the approval standards and requirements for such applications as contained in this Code. Each decision made by a Hearing Authority or Appeal Authority shall list the approval criteria and indicate whether the criteria are met. It is the applicant's burden to demonstrate to the Hearing Authority and Appeal Authority how each of the approval criteria are met. An application may be approved with conditions of approval imposed by the Hearing Authority or Appeal Authority. On appeal, the Appeal Authority may affirm, reverse, amend, refer, or remand the decision of the Hearing Authority.
2. In addition to Section 1 above, all Type IV quasi-judicial applications shall also demonstrate compliance with the Conditional use criteria of Section 16.82.020.
Response: The applicant presents this narrative/findings document, drawings and other evidence for the proposed development to meet requirements for submitted applications as contained in this Code, and to demonstrate compliance with applicable standards and approval criteria. A Conditional Use permit will be requested in conjunction with this development, subject to the provisions of Section 16.82.020.

## Division IV. - Planning Procedures

## Chapter 16.82 - Conditional Uses

16.82.010-Generally
A. Authorization

Uses permitted in zoning districts as conditional uses may be established, enlarged, or altered by authorization of the Commission in accordance with the standards and procedures established in this Chapter. If the site or other conditions are found to be inappropriate for the use requested, the Commission or Hearings Officer (cited below as Hearing Authority) may deny the conditional use.
B. Changes in Conditional Uses

Changes in use or expansion of a legal non-conforming use, structure or site, or alteration of structures or uses classified as conditional uses, that either existed prior to the effective date of this Code or were established pursuant to this Chapter shall require the filing of a new application for review conforming to the requirements of this Chapter if the proposed changes would increase the size, square footage, seating capacity or parking of existing permitted improvements by twenty percent (20\%) or more.
C. Application and Fee

An application for a Conditional Use Permit (CUP) shall be filed with the City and accompanied by the appropriate fee pursuant to Section 16.74.010. The applicant is responsible for submitting a complete application which addresses all criteria of this Chapter and other applicable sections of this Code.

Response: Based on the table in Section 16.31.020, in the El Zone, standalone distribution and warehousing up to $150,000 \mathrm{SF}$ is a permitted use, provided product(s) are stored within an enclosed building. Such operations exceeding 150,000 SF require Conditional Use Permit approval.

This application includes a new CUP request to allow Building C, containing 183,292 SF, to accommodate a future warehousing/distribution tenant without further land use review, as authorized by subparagraph A. Subparagraph B is not applicable because the proposal is a new CUP request rather than a change in an existing one. The applicant has submitted a complete application and fee payment consistent with Subparagraph C. This standard is met.

### 16.82.020 - Permit Approval

A. Hearing Authority Action

1. The Hearings Authority shall conduct a public hearing pursuant to Chapter 16.72 and take action to approve, approve with conditions, or deny the application. Conditions may be imposed by the Hearings Authority if necessary to fulfill the requirements of the adopted Comprehensive Plan, Transportation System Plan, or the Code. The decision shall include appropriate findings of fact as required by this Section, and an effective date.
Response: These provisions establish the authority of the Hearings Authority and provide procedural guidance; they require no evidence from the applicant.
2. Conditional uses may be approved at the hearing for a larger development (i.e. business campus or industrial park), to include future tenants of such development, if the range of uses allowed as conditional uses are considered, and specifically approved, at the time of original application.
Response: The intended development is to be generally industrial in nature, including warehousing, distribution, and light industrial uses. Although this development proposal does not include any known future conditional use tenants, it is reasonable to anticipate that a warehousing/distribution tenant may find the 183,292 SF Building C suitable. The applicant requests conditional use approval to allow Building $C$ to be occupied by future warehouse and distribution facilities greater than 150,000 SF, up to its total area of 183,292 SF, without a further land use procedure. This will enable Building C to compete effectively for such tenants against other prospective sites where no discretionary land use review would be required. This request is consistent with the authority provided in Subsection 2.

## B. Final Site Plan

Upon approval of a conditional use by the Hearing Authority, the applicant shall prepare a final site plan for review and approval pursuant to Section 16.90. The final site plan shall include any revisions or other features or conditions required by the Hearing Authority at the time of the approval of the conditional use.
Response: The applicant will provide construction plans following CUP approval, including a final site plan consistent with these requirements. Compliance can be assured through a condition of approval.
C. Use Criteria

No conditional use shall be granted unless each of the following is found:

1. All public facilities and services to the proposed use, including but not limited to sanitary sewers, water, transportation facilities, and services, storm drains, electrical distribution, park and open space and public safety are adequate; or that the construction of improvements needed to provide adequate services and facilities is guaranteed by binding agreement between the applicant and the City.
Response: As discussed in the Division VI and Division VII compliance findings, existing utility services and streets in the vicinity of the subject property have sufficient capacity to serve the site, with extensions to be provided by the applicant to extend utilities to the site and within the proposed cul-de-sac street to serve the proposed lots and buildings. This standard is met.
2. Proposed use conforms to other standards of the applicable zone and is compatible with abutting land uses in regard to noise generation and public safety.
Response: The subject property, located in the City's Employment Industrial zone, is suitable for a wide variety of industrial uses. All five of the proposed buildings are designed to accommodate a range of light industrial, manufacturing, or warehousing/distribution tenants, similar to the many that exist in the vicinity of the subject property, i.e., within the Tualatin-Sherwood Road corridor between commercial centers to the east (in Tualatin) and west (near Oregon Highway 99 W in Sherwood). The proposed development is therefore consistent and compatible with nearby land uses. Abutting land uses include municipal water storage to the west and a planned water treatment facility to the south, neither of which would be negatively impacted by noise from a warehouse/distribution center (there are no abutting land uses to the north or east based on the Development Code's definition of "abut"). Conditional Use Permit approval is required only for Building C because it alone exceeds 150,000 SF. This standard is met.
3. The granting of the proposal will provide for a facility or use that meets the overall needs of the community and achievement of the goals and/or policies of the Comprehensive Plan, the adopted City of Sherwood Transportation System Plan and this Code.
Response: As noted above, all of the buildings are proposed for a range of light industrial, manufacturing, or warehousing/distribution use, which is consistent with the purpose of the El zone and comprehensive planning for the Tonquin Employment Area in which the subject property is located. Building $C$ is in alignment with the overall proposal for development and use of the property consistent with its comprehensive plan designation and zoning; however, a standalone warehouse/distribution user that exceeds 150,000 SF would require CUP approval to locate there. The applicant has provided a traffic impact analysis for the development as a whole, demonstrating the capacity of the transportation system to accommodate resulting traffic, assuming full occupancy of all proposed buildings, including Building C . This standard is met.
4. Surrounding property will not be adversely affected by the use, or that the adverse effects of the use on the surrounding uses, the neighborhood, or the City as a whole are sufficiently mitigated by the conditions proposed.
Response: As noted above, the proposed Warehouse/Distribution use and development are consistent with and compatible with the uses surrounding the subject property in the TualatinSherwood Road industrial corridor as well as the planning for development of the Tonquin Employment Area. No impacts requiring mitigation actions are anticipated. This criterion is met.
5. The impacts of the proposed use of the site can be accommodated considering size, shape, location, topography and natural features.
Response: As noted above, the proposed Warehouse/Distribution use and development are consistent with and compatible with the uses surrounding the subject property in the TualatinSherwood Road industrial corridor as well as the planning for development of the Tonquin Employment Area. This criterion is met.
6. The use as proposed does not pose likely significant adverse impacts to sensitive wildlife species or the natural environment.
Response: The subject property contains wetlands that have not been designated as a significant habitat resource area. The applicant has retained Pacific Habitat Services (PHS) to prepare an expert inventory of wetland resource values within the subject property and make recommendations for resource conservation. As a result, development as proposed in the upland areas of the subject property will not adversely affect sensitive wildlife species or significant wetland natural resource features. While the southern portion of the site has been designated by

Metro as upland habitat, the proposed tree removal in some of this habitat area will be performed in full compliance with the City's tree preservation standards to maintain the ecological functions of the tree areas. This criterion is met.
7. For wireless communication facilities, no Conditional Use Permit will be granted unless the following additional criteria is found:
a. The applicant demonstrates to the satisfaction of the City that the wireless communication facility cannot be located in an IP zone due to the coverage needs of the applicant.
b. The proposed wireless communication facility is designed to accommodate colocation or it can be shown that the facility cannot feasibly accommodate colocation.
c. The applicant demonstrates a justification for the proposed height of the tower or antenna and an evaluation of alternative designs which might result in lower heights.
d. The proposed wireless communication facility is not located within one-thousand $(1,000)$ feet of an existing wireless facility or that the proposed wireless communication facility cannot feasibly be located on an existing wireless communication facility.
e. The proposed wireless communication facility is located a minimum of threehundred (300) feet from residentially zoned properties.
Response: These provisions are not applicable because the proposal does not include wireless communication facilities.
8. The following additional criteria apply to transportation facilities and improvements subject to Conditional Use approval per Chapter 16.66. These are improvements and facilities that are (1) not designated in the adopted City of Sherwood Transportation System Plan (TSP), and are (2) not designed and constructed as part of an approved land use application.
a. The project preserves or improves the safety and function of the facility through access management, traffic calming, or other design features.
b. The project includes provisions for bicycle and pedestrian access and circulation consistent with the Comprehensive Plan, the requirements of this Code, and the TSP.
c. Proposal inconsistent with TSP: If the City determines that the proposed use or activity or its design is inconsistent with the TSP, then the applicant is required to apply for and obtain a plan and/or zoning amendment prior to or in conjunction with Conditional Use Permit approval.
d. State transportation system facility or improvement projects: The Oregon Department of Transportation (ODOT) must provide a narrative statement with the application demonstrating compliance with all of the criteria and standards in Sections 16.82.020.C.1-6 and 8.a-8.d. Where applicable, an Environmental Impact Statement or Environmental Assessment may be used to address one or more of these criteria.
Response: These provisions are not applicable because the proposal does not include transportation facilities and improvements subject to Conditional Use approval per Chapter 16.66.

## D. Additional Conditions

In permitting a conditional use or modification of an existing conditional use, additional conditions may be applied to protect the best interests of the surrounding properties and neighborhoods, the City as a whole, and the intent of this Chapter. These conditions may include but are not limited to the following:

1. Mitigation of air, land, or water degradation, noise, glare, heat, vibration, or other conditions which may be injurious to public health, safety or welfare in accordance with environmental performance standards.
2. Provisions for improvement of public facilities including sanitary sewers, storm drainage, water lines, fire hydrants, street improvements, including curb and sidewalks, and other above and underground utilities.
3. Increased required lot sizes, yard dimensions, street widths, and off-street parking and loading facilities.
4. Requirements for the location, number, type, size or area of vehicular access points, signs, lighting, landscaping, fencing or screening, building height and coverage, and building security.
5. Submittal of final site plans, land dedications or money-in-lieu of parks or other improvements, and suitable security guaranteeing conditional use requirements.
6. Limiting the number, size, location, height and lighting of signs.
7. Requirements for the protection and preservation of existing trees, soils, vegetation, watercourses, habitat areas and drainage areas.
8. Requirements for design features which minimize potentially harmful environmental impacts such as noise, vibration, air pollution, glare, odor and dust.
Response: The proposed CUP is to allow a potential future warehousing/distribution user larger than 150,000 SF in Building C, the only one of five proposed industrial buildings with floor area exceeding $150,000 \mathrm{SF}$. As discussed in other sections of this narrative, the project will include construction of utility system extensions and street improvements to satisfy all applicable City of Sherwood standards for the project as a whole, including the 183,292 SF Building C. The proposed use and development are consistent with the subject property's El zoning, and compatible with the industrial zoning and development surrounding it. For these reasons, no imposition of additional conditions is necessary or warranted to protect the best interests of the surrounding properties and neighborhoods, the City as a whole, and the intent of this Chapter. This criterion is met without additional conditions.

## E. Time Limits

Unless approved under Section 16.82.020.A. 2 for a larger development to include future tenants of such development, authorization of a conditional use shall be void after two (2) years or such lesser time as the approval may specify unless substantial construction, in the City's determination, has taken place. The Hearing Authority may extend authorization for an additional period, not to exceed one (1) year, upon a written request from the applicant showing adequate cause for such extension, and payment of an extension application fee as per Section 16.74.010.
Response: This provision provides procedural guidance for implementation following approval and requires no evidence from the applicant.

## F. Revocation

Any departure from approved plans not authorized by the Hearing Authority shall be cause for revocation of applicable building and occupancy permits. Furthermore, if, in the City's determination, a condition or conditions of CUP approval are not or cannot be satisfied, the CUP approval, or building and occupancy permits, shall be revoked.
Response: This provision provides procedural guidance for implementation following approval and requires no evidence from the applicant.

## Chapter 16.84-Variances

### 16.84.010-Purpose

This Chapter provides standards and procedures for variances, which are modifications to land use or development standards that are not otherwise permitted elsewhere in this Code as exceptions to Code standards. This Chapter provides flexibility, while maintaining the purposes and intent of the Code. No variances shall be granted to allow the use of property for a purpose not authorized within the zone in which the proposed use is located. In granting a variance, conditions may be imposed when necessary to protect the best interests of surrounding properties and neighborhoods, and otherwise achieve the purposes of the adopted Comprehensive Plan, the Transportation System Plan, and other Code provisions. Response: The applicant has provided evidence below responding to applicable approval criteria for the requested variance to exceed the 200-foot length standard in Section 16.106.040.E and to utilize a cul-desac rather than construct a through street connection to SW Blake Road.

### 16.84.020-Applicability

A. Exceptions and Modifications versus Variances

A code standard or approval criterion may be modified without approval of a variance if the applicable code section expressly allows exceptions or modifications. If the code provision does not expressly provide for exceptions or modifications then a variance is required to modify that code section and the provisions of Chapter 16.84 apply.
Response: Deviations from the cul-de-sac standard in Section 16.106.040.E are not expressly listed as allowable exceptions or modifications, so the applicant is requesting variance approval. This standard is met.
B. Combining Variances with Other Approvals; Permit Approvals by Other Agencies. Variance requests may be combined with and reviewed concurrently by the City approval body with other land use and development applications (e.g., development review, site plan review, subdivision, conditional use, etc.); however, some variances may be subject to approval by other permitting agencies, such as ODOT in the case of State Highway access.
Response: The applicant requests that the City review the variance application concurrently with the site plan review, conditional use, and subdivision application. This standard is met.
C. Adjustments and variances cannot be applied to change any existing Planned Unit Development (PUD).
Response: This site is not within an existing PUD. This standard does not apply.

### 16.84.030-Types of Variances

As provided in this Section, there are three types of variances: Adjustments, Class $A$ variance and Class $B$ variance; the type of variance required depends on the extent of the variance request and the discretion involved in the decision making process.
C. Class A Variances

1. Generally
a. The Class A variance procedure may be used to modify a standard for three (3) or fewer lots, including lots yet to be created through a partition process.
b. An applicant who proposes to vary a standard for lots yet to be created through a subdivision process may not utilize the Class A variance procedure. Approval of a Planned Unit Development shall be required to vary a standard for lots yet to be created through a subdivision process, where a specific code section does not otherwise permit exceptions.
c. A Class A Variance shall not be approved that would vary the "permitted, conditional or prohibited uses" of a land use district.
Response: The applicant is requesting a Class A Variance to vary cul-de-sac length from the 200foot maximum limit to allow the proposed 550-foot length for SW Cipole Place and to waive the standard for a paved bicycle and pedestrian path south of the cul-de-sac. This section is not applicable because the proposed variance would not vary the standards for lots (subparagraphs a and b) or uses (subparagraph c).
2. Approval Process:
a. Class A Variances shall be processed using a Type IV procedure, as governed by Chapter 16.84, using the approval criteria in subsection 3, below.
Response: The applicant requests a Class A Variance and the request has been prepared and submitted consistent with the requirements stipulated for a Type IV procedure. The information required for a Type IV application has been submitted to the City of Sherwood along with the corresponding application fee. Review of and issuance of a decision on the request shall occur consistent with the relevant provisions from Chapter 16.72 and Section 16.84.030. Findings in response to the applicable review criteria are presented below. This standard is met.
b. In addition to the application requirements contained in Chapter 16.72.010, the applicant shall provide a written narrative describing the reason for the variance, why it is required, alternatives considered, and compliance with the criteria in subsection 3.
Response: This section provides the rationale for the SW Cipole Place cul-de-sac variance request and a description of the multiple alternative designs for the site and the roadway examined by the applicant. Compliance with subsection 3 is provided in the responses to that section below.

## Background

As discussed in the narrative introduction, the applicant is proposing a subdivision for the sole reason of being able to offer buildings for sale or lease to potential users in a one-building-per-lot final configuration. Trammell Crow Company's experience with industrial development in the Tualatin-Sherwood Road corridor reveals that many users prefer to own their own sites, and that by doing so, they make greater investment in the community and increase the likelihood of manufacturing jobs. If market demand did not call for the flexibility for users to purchase their own lots, then all five proposed buildings could be constructed on a single lot, with no associated subdivision. In this scenario, a public street would not be needed to provide access, and the development could be constructed with a single private internal drive from SW Tualatin-Sherwood Road and no vehicle or pedestrian/bicycle connection to SW Blake Road.

Since neither the City of Sherwood 2014 Transportation Plan (TSP) nor applicable industrial development standards require a vehicular or pedestrian connection through the property, the applicant proposes a single point of access across from Cipole Road, the only location that is feasible and approved by Washington County. Washington County has approved a Design Exception (Attachment 12) for non-arterial/non-collector access to SW Tualatin-Sherwood Road at the existing signalized Cipole Road intersection, as this provides a safe protected location for large trucks, employees, and customers of the site and does not create a new intersection on an existing arterial. The County is not
permitting driveway connections to 124 th Avenue since this is an access-controlled roadway with grades that would not accommodate connections at locations other than Blake Road. Access cannot be obtained from the west due to the location of the existing municipal water reservoir, the unimproved/substandard condition of existing roadways (e.g., Dahlke Lane and the access to the reservoir), and the close proximity of the Dahlke Lane/Oregon Street intersection to the signal at the Oregon Street/Tualatin-Sherwood Road intersection.

## Assessment of Cul-de-Sac vs. Through Street

Prior transportation planning efforts for the City in general and the Tonquin Employment Area have not identified the need for a public street extending southward from the SW Cipole Road/SW Tualatin-Sherwood Road intersection. As detailed in the supporting materials in Attachment 12, Figure 18 in the TSP depicts the south approach of this intersection with an arrow, indicating it is a conceptual street connection, not a proposed roadway. Significantly, the 2015 Tonquin Employment Area Market Analysis, Business Recruitment Strategy, and Implementation Plan² (the TEA Implementation Plan) notes that "...we are assuming an internal drive will be located here instead" of an extension of Cipole Road south of Tualatin-Sherwood Road.

As further detailed in the supporting materials in Attachment 12, neither the City TSP, the TEA Implementation Plan, nor the Washington County TSP Functional Classification Urban Area Map 6 illustrate an existing or proposed street at this location, or anywhere south of Tualatin-Sherwood Road between Oregon Street and SW 124th Avenue.

The Traffic Impact Analysis (Attachment 11) compared the resulting roadway operations for a cul-de-sac and a through street to Blake Road, concluding the following:

Traffic Operations: Regardless of whether or not SW Cipole Road is extended through the site, the adjacent study intersections are all anticipated to meet the jurisdictional mobility standard. While the extension of SW Cipole Road results in slightly improved operations at the SW Cipole Road / SW Tualatin-Sherwood Road intersection, operations remain the same or slightly deteriorate at the SW 124th Avenue / SW Tualatin-Sherwood Road, SW Cipole Road/Blake Road and SW 124th Avenue / Blake Road intersections. Therefore, there appears to be no significant system-wide benefit to extending SW Cipole Road through the site to connect with the future Blake Road.

Traffic Safety: A connection to Blake Road would add an access point to the roadway network, introducing conflict. Limiting SW Cipole Road to a cul-de-sac ending would result in fewer unprotected left-turn conflict points on the surrounding roadway network, especially those involving large trucks.

[^1]In addition to not being necessary or advantageous for traffic operations, the potential extension of Cipole Place to Blake Road would also increase street maintenance costs for the City and increase the potential for conflicts between trucks and passenger vehicles that may opt to cut through Cipole Place rather than use SW 124th Avenue for northsouth travel. An extension of Cipole Road to Blake Road would connect to Blake on a horizontal roadway curve which presents several concerns since curves can pose sight distance problems; if a perpendicular intersection were instead created then it may result in steeper road grades and would increase site impacts due to additional right-of-way and fill slope requirements.

Importantly, the Fire Marshal from Tualatin Valley Fire \& Rescue has indicated that, from the perspective of emergency service, "The proposed cul de sac is allowed and a secondary access would not be required if all buildings are fully sprinklered." The applicant does plan to construct buildings with sprinkler systems for fire suppression. As illustrated on Sheet C3.3 in Attachment 6, the proposed cul-de-sac terminates in a bulb with a paved radius of 54 feet to allow for fire truck turnarounds.

Roadway grades over 3\% (which would be required if the road extended south to Blake Road) affect intersection sight distance needs. If a relatively flat grade is not provided at the Cipole/Blake intersection, additional intersection sight distance will be needed, which exacerbates the intersection design.

## Assessment of Cul-de-Sac Length

The City's 200-foot standard is primarily geared toward residential development rather than large-lot industrial development. The site size, configuration, dimensions, and locations of wetlands make a 200 -foot cul-de-sac infeasible for an industrial park. At 200 feet, the cul-de-sac would be too far north to provide access to Building E without impacting wetlands. Additionally, a length of 200 feet or less may provide inadequate space for trucks to queue during peak operations, potentially leading to conflicts if southbound vehicles spill back onto SW Tualatin-Sherwood Road. Accordingly, the applicant is requesting a longer cul-de-sac to provide for safe operations by alleviating queuing concerns, and to reduce resource impacts by allowing driveways to be routed around wetlands. The proposed site layout extends the cul-de-sac south and west away from the wetlands, with the resulting access point to Building E being situated on the cul-de-sac bulb 550 feet south of Tualatin-Sherwood Road. This is the shortest length possible without causing wetland impacts. The applicant has also submitted an associated Engineering Design Modification request (Attachment 20).

## Economic Development Opportunity

The site is a designated Industrial area in Metro's Urban Growth Management Functional Plan (UGMFP) Title 4 Industrial and Other Employment Areas map (October 2014). Section 3.07.410 of the UGMFP stipulates in part that "To improve the economy, Title 4 seeks to provide and protect a supply of sites for employment..." Accordingly, after the Tonquin Employment Area (TEA) was brought into the urban growth boundary, the City of Sherwood designated the TEA for industrial development and established the EI zone with limits on the size and scope of non-industrial uses. Now that the property has been annexed into the City and zoned El, the applicant seeks to maximize opportunities for industrial development and employment.

## Alternatives Analysis

The proposal to construct a cul-de-sac itself was not a design decision that was approached casually, but rather reflected extensive alternatives analysis that examined the feasibility of the street southward to connect to the future alignment of Blake Road. The steep slope and configuration of the site poses development constraints for the largefootprint industrial buildings envisioned for the TEA and allowed in the El zone. To address the requirement for large, flat sites, there are two basic approach options for designing the site and the roadway. The first focuses on the site with the primary objective of accommodating the property's industrial Comprehensive Plan designation, with roadway design subordinate to site planning. The second approach's objective is to create a street design that falls within engineering standards and best practices, subordinating site design. To advance the City's and Metro's economic development goals for the TEA, the applicant is implementing the first approach.

The site slopes downward steeply from south to north, with an elevation drop of approximately 45 feet ${ }^{3}$ from the future Blake Road location to SW Tualatin-Sherwood Road. The majority of the site has slopes over 7\%, which is generally the upper limit for accommodating large-footprint industrial structures, as steeper slopes do not accommodate large, rectangular buildings, truck courts, and associated parking areas without significant grading efforts and associated development costs. The localized areas of lesser slopes contain existing wetlands that the applicant proposes to preserve. Without significant site grading, the site would provide space for only one large industrial building with much less capacity than the proposed plan.

In keeping with the first approach outlined above, the applicant proposes to maximize building area and achieve efficient cross-lot circulation by grading the site as depicted in Attachment 6, Sheets C4.0-C4.2. This layout establishes the buildings' finished floor elevations at approximately the same level, with only a three-foot difference among buildings. Much of the existing hillside will be excavated to accommodate the building footprints and parking areas, with site walls present around most of the site perimeter (some of these would be engineered retaining walls, while others would be cut faces in rock areas). In general, Lots 3 and 4 and the western portion of Lot 1 will be lower than off-site abutting land, while Lots 2 and 5 and the northern portion of Lot 1 will be higher than off-site abutting land. The resulting SW Cipole Place roadway design utilizes a 3\% slope (Attachment 6, Sheet C3.3), which is appropriate for a street with significant truck traffic. ${ }^{4}$ The elevation of the cul-de-sac bulb would remain approximately 30 feet lower than Blake Road.

As part of the alternatives analysis, for Option 1 the design team examined the implications of utilizing the same Cipole Place profile as shown in Attachment 6, Sheet

[^2]C3.3, but then extending the roadway southward to connect to the future Blake Road (Attachment 13, pages 1-3). Key points about the Option 1 design are:

- The road was designed with a design speed of 25 miles per hour, per standards for local streets.
- The road grade from SW Tualatin-Sherwood Road to the main vehicle entries in the cul-de-sac bulb was set at $3 \%$ to facilitate truck operations and allow reasonable access to the northeast portion of the site without crossing the wetlands.
- This option minimizes wall height for Cipole Place (north of the cul-de-sac bulb) and minimizes impact to vegetated corridor.
- The roadway vertical curves were designed to minimum $K$ values as specified in the Engineering Design Manual, requiring street lighting to meet American Association of State Highway and Transportation Officials (AASHTO) and City of Sherwood standards.
- The resulting roadway profile (Attachment 13, page 2) illustrates that a portion of the street would need to be constructed at a $14.9 \%$ slope, thus necessitating City Engineer approval (City engineering standards dictate that road slopes may not exceed $15 \%$, and any slopes over $12 \%$ require special approval by the City Engineer).
- Any vehicle turning onto Cipole Place from Blake Road will not be able to see the driveway entries until after they have crested the vertical curve and are descending down the $14.9 \%$ slope, creating a hazardous traffic condition.
- The resulting cross-section fill slopes extend into the site by up to 100 feet (Attachment 13, page 1), impacting additional vegetated corridor and reducing developable area.

Option 1 reduces the size of Building D by more than $38,600 \mathrm{SF}$ compared to the applicant's proposal (Attachment 13, page 3), and requiring placement of a significant volume of fill that would impact existing wetland features. Based on the reduction in building area, the applicant would expect an associated reduction of 64 jobs, using an average rate of 600 SF per employee based on Appendix 6 of Metro's 2014 Urban Growth Report. Furthermore, assuming a property tax rate of $\$ 1.25$ per SF per year, growing at $3 \%$ per year, this reduction in building area would result in lost property tax revenue of over $\$ 5.4$ million over the expected life of a building ( 50 years). Finally, the associated cost of extending the roadway using this road profile would be $\$ 610,000$ greater than the applicant's proposal.

In keeping with the second approach outlined above, the design team examined a second alternative (Option 2) which would extend Cipole Place to Blake Road using a constant slope, and then adjusting site grades to fit the roadway (Attachment 13, pages 4-6). Key points about the Option 2 design are:

- The road was designed with a design speed of 25 miles per hour, per standards for local streets.
- The roadway profile (Attachment 13, Sheet 5) utilizes a $6 \%$ slope from SW Tualatin-Sherwood Road to Blake Road. While this slope requires no special approval from the City Engineer, it is steeper than comfortable for trucks (3\% would be preferable for trucks, and a maximum of $5 \%$ is recommended).
- The roadway vertical curves were designed to minimum $K$ values as specified in the Engineering Design Manual, requiring street lighting to meet AASHTO and City of Sherwood standards.
- Wall heights along the east side of Cipole Place north of the cul-de-sac bulb are the same as in Option A, but due to the higher roadway slope this results in additional grading at 3:1 side slopes, impacting the vegetated corridor and wetlands.
- Due to the higher roadway profile, the resulting cross-section fill slopes extend into the site by over 400 feet in some locations (Attachment 13, page 4), requiring fill in Wetland A and imposing additional vegetated corridor impacts while also reducing developable area.
- Due to increased vegetated corridor and wetland impacts, a dedicated area for on-site mitigation is illustrated west of Cipole Place, using up valuable land.
- The wetland impacts would require state and Federal permitting, a timeconsuming and unpredictable process.
- To provide tenant and truck access to buildings both east and west of Cipole Place, sloped drive aisles would need to be constructed at slopes of approximately 4-4.5\%, which pushes buildings farther away from the street and results in smaller building footprints.
- These site slopes are higher than some industrial users are willing to accept due to impacts on truck operations, which then reduces the attractiveness of the site for potential tenants/purchasers. For instance, Amazon's standards stipulate a maximum slope of $3.5 \%$ in truck areas at their facilities.
- The longer truck aisles needed to access the site result in the complete elimination of an entire building since trucks can no longer access it and a shared truck court is no longer feasible.

The Option 2 alternative reduces the total building area by over 132,000 SF (Attachment 13, page 6) compared to the applicant's proposal. Based on the reduction in building area, the applicant would expect an associated reduction of 220 jobs, using an average rate of 600 SF per employee based on Appendix 6 of Metro's 2014 Urban Growth Report. Furthermore, assuming a property tax rate of $\$ 1.25$ per SF per year, growing at $3 \%$ per year, this reduction in building area would result in lost property tax revenue of over $\$ 18.6$ million over the expected life of a building ( 50 years). Finally, the associated cost of extending the roadway using this road profile would be $\$ 1.2$ million greater than the applicant's proposal.

Compared to the applicant's proposal, both alternatives decrease building area, which yields a corresponding reduction in job opportunity for the community. Taken together, the resulting increased costs of extending the street farther south, the constrained circulation, and the reduced square footage yields would make industrial development infeasible. Furthermore, the TIA points out potential roadway conflicts that would occur if Cipole Road were extended south to Blake Road.

The analysis above provides evidence that the applicant has thoroughly evaluated multiple alternatives before arriving at the proposed site and roadway design, and that the proposed design is superior to the alternatives. This standard is met.
3. Approval Criteria: The City shall approve, approve with conditions, or deny an application for a Class A Variance based on the following criteria:
a. The proposed variance will not be materially detrimental to the purposes of this Code, to any other applicable policies and standards, and to other properties in the same land use district or vicinity;
Response: A Variance is requested to elements of Section 16.106.040.E, which would restrict cul-de-sac length to 200 feet and would require pedestrian/bicycle connections from the cul-de-sac bulb to other streets. As illustrated on Sheet C3.3, the applicant proposes a cul-de-sac length of approximately 550 feet for SW Cipole Place and proposes not to provide an impractically steep pedestrian/bike path to SW Blake Road.

The purposes of the Code are outlined in Section 16.02.020, which states that:
This Code is enacted to:
A. Encourage the most appropriate use of land.
B. Conserve and stabilize the value of property.
C. Preserve natural resources.
D. Facilitate fire and police protection.
E. Provide adequate open space for light and air.
F. Minimize congestion on streets.
G. Promote orderly growth of the City.
H. Prevent undue concentrations of population.
I. Facilitate adequate provision of community facilities.
J. Promote in other ways the public health, safety, convenience, and general welfare.
K. Enable implementation of the Sherwood Comprehensive Plan in compliance with State Land Use Goals.

Lengthening the cul-de-sac from 200 feet to approximately 550 feet would not be detrimental to these purposes as it would result in orderly and efficient use of industrial land, lead to increased employment within city limits, and maintain acceptable traffic operations as detailed in Attachment 11.

As the cul-de-sac will be entirely self-contained within the limits of the Corporate Park, the additional roadway length does not negatively impact abutting properties or those in the vicinity. Extending the length of the cul-de-sac will provide adequate space for southbound trucks to queue in the cul-de-sac without spilling back into SW TualatinSherwood Road, thereby decreasing the potential for unsafe traffic conditions at the intersection, which could occur if the roadway were limited to 200 feet. Furthermore, abutting properties do not need access to SW Cipole Place in order to develop, as properties to the north have access to SW Tualatin-Sherwood Road; properties to the east have access to SW 124th Avenue; properties to the south can take access to Blake Road when it gets constructed; and properties to the west can take access from Dahlke Lane. As demonstrated in Attachment 11, traffic operations are not negatively affected by utilizing a cul-de-sac rather than a through street.

The proposed variance will have a greater positive impact on the surrounding area than the alternatives, by allowing for increased building sizes as compared to constructing a through street to Blake Road. Because usable square footage will be higher, employment opportunities and tax base growth will also increase.

Based on the factors described above, a 550-foot industrial cul-de-sac rather than a through street or a 200-foot cul-de-sac will not result in a materially detrimental impact
on surroundings properties or roadways, or on employees, customers, and guests who will be working at and visiting the site.

Allowing the 550 -foot cul-de-sac, as proposed, will also enable development of the site in an efficient manner that is compatible with surrounding land uses. Construction of the street as proposed allows the balance of the site to be designed with an efficient building placement and vehicle circulation pattern that respects the location and value of the onsite wetlands; by contrast, driveway alignments necessary with a 200 -foot cul-de-sac length would cause significantly greater impacts. These characteristics are consistent with Industrial Land Use Policies 1 and 2 of Chapter 4 of the Sherwood Comprehensive Plan II, which state the following.

Policy 1 - Industrial uses will be located in areas where they will be compatible with adjoining uses, and where necessary services and natural amenities are favorable.

Policy 2 - The City will encourage sound industrial development by all suitable means to provide employment and economic stability to the community.

The degree of flexibility sought by the proposed Variance also aligns with Community Design Policy 4, as referenced below, through applying a flexible site design approach that effectively maintains transportation operational standards while allowing for a financially viable industrial development that does not allocate valuable land to roadway crosssection fill slopes which diminish the usability of the site.

Policy 4 - Promote creativity, innovation and flexibility in structural and site design.

Approving the proposed variance would provide economic benefit to the City as it would accommodate larger buildings, leading to a corresponding increase in the number of jobs available for area residents, improving the City's jobs-housing balance.

Finally, with respect to providing a pedestrian/bicycle connection to a neighboring street, due to the elevation difference between the proposed SW Cipole Place cul-de-sac and SW Blake Road, a path between the two would need to be steeply sloped and traverse a steep cut/fill bank with high retaining walls. Even with safety railings, a path through this location could pose safety risks, especially in wet or icy/snowy conditions. Situated within an industrially zoned area, there is little reason to anticipate significant pedestrian or bike traffic. Routing by way of SW 124th Avenue, with sidewalks and bike lanes when fully improved, will be much safer, so the applicant is proposing pedestrian connections from Buildings D and E to 124th Avenue.

Given these findings, the proposed Variance is consistent with the criterion cited above.
b. A hardship to development exists which is peculiar to the lot size or shape, topography, or other similar circumstances related to the property over which the applicant has no control, and which are not applicable to other properties in the vicinity (e.g., the same land use district);
Response: The site's developable area is constrained both by the locations of the wetlands (which the applicant wishes to preserve in their natural state), by the site size
and shape (which differs from other sites in the TEA), and by the site topography. From south to north, the site has an elevation drop of approximately 45 feet from the future Blake Road location to SW Tualatin-Sherwood Road, which has the effect of making a through connection to Blake Road impracticable for the reasons discussed above. Since Washington County has indicated that no connections will be permitted to SW 124th Avenue, then the only remaining option for a street is a cul-de-sac extending southward from SW Tualatin-Sherwood Road. Limiting this cul-de-sac to 200 feet is not viable since it could cause truck queuing spillbacks onto Tualatin-Sherwood Road and would require the driveway for Lot 5 to pass through the wetlands, negatively impacting their condition. The north-south dimension of the site necessitates a longer roadway to provide access to all the proposed Lots and buildings.

The site's topography, particularly the elevation difference between the proposed cul-desac and (future) SW Blake Road also makes construction of a safe pedestrian/bike facility impractical at this location.

Based on the evidence provided, this standard is met.
c. The use proposed will be the same as permitted under this title and City standards will be maintained to the greatest extent that is reasonably possible while permitting reasonable economic use of the land;
Response: Approval of the proposed Class A Variance will have no effect on the types of uses occurring at the site; the applicant proposes speculative industrial buildings which are consistent with allowed uses in the El zone. Other than the cul-de-sac length, applicable development standards are proposed to be met with this development. Allowing a 550 -foot cul-de-sac allows development of 535,000 SF of industrial space, which constitutes a significant boost to the local economy as intended by the Tonquin Employment Area plan. Allowing the variance and not requiring a through street to Blake Road results in a roadway that complies with engineering design standards for road grade, suitable for semi tractor-trailer trucks and fire trucks while remaining financially viable for the applicant. By contrast, due to the elevation difference from north to south, extending SW Cipole Place to Blake Road would have required side slopes that decreased developable area and resulted in less building area, at which point the project may no longer be viable. This standard is met.
d. Existing physical and natural systems, such as but not limited to traffic, drainage, natural resources, and parks will not be adversely affected any more than would occur if the development occurred as specified by the subject Code standard;
Response: It will remain possible to construct all necessary transportation facilities along the SW Tualatin-Sherwood Road and SW 124th Avenue frontages, consistent with the applicable design standards for arterial roadways, as specified in the Sherwood Transportation System Plan and proposed through the corresponding Site Plan Review and Preliminary Subdivision application. In effect, as discussed above, the proposed cul-de-sac will be functionally equivalent to a single, central private drive serving all tenants within the industrial park, while allowing subdivision of the property to locate each building on its own lot.

The transportation impact study (Attachment 11) evaluates the effect of the proposed development on the transportation system and provides evidence in support of the requested variance. Specifically, this analysis compares both network concepts and
demonstrates no net benefit to the transportation system if SW Cipole Place were a through street. The TIA demonstrates that the development will have a negligible impact on the operations of nearby streets, and that the proposed cul-de-sac is appropriate given the trip generation and capacity of nearby roadways. Therefore, approving the variance is consistent with the general purpose of promoting safety and maintaining an efficient transportation network.

The preliminary storm report (Attachment 16) demonstrates that stormwater management will be performed in accordance with drainage best practices and Clean Water Services standards; the variance does not affect the ability to provide appropriate stormwater management.

Wetland Delineation Reports and Natural Resource Assessment Report (Attachments 14 and 15) provide evidence that the development avoids wetlands, which is achieved by curving the cul-de-sac alignment southwesterly from the SW Cipole Road/SW TualatinSherwood Road intersection to minimize habitat and water quality impacts on the existing wetlands.

Allowing a cul-de-sac longer than 200 feet at this location will have no impact on parks since no parks are located on or near the site.

Based on the above factors and considerations, this standard is met.

## e. The hardship is not self-imposed; and

Response: The presence of natural resources (wetlands) on site, the significant elevation gain from north to south precluding a through street or pedestrian/bike path to Blake Road, and the lot depth, are all conditions beyond the control of the applicant. These conditions are existing and not "self-imposed," so the need for the variance was not created by the applicant. The elevation changes alone pose a significant design constraint for industrial development, as the grading required to achieve the large, flat sites needed for large industrial buildings results in a road profile that does not allow for safe or convenient access to Blake Road. This condition was exacerbated when the alignment of Blake Road was shifted northward approximately 550 feet (see supporting materials in Attachment 12) to accommodate the needs of the planned Willamette Water Supply Program water treatment facility; the northward shift shortened the horizontal distance between SW Tualatin-Sherwood Road and Blake Road without decreasing the elevation change. As a result, attempting to construct a through street to Blake Road results in a steeper roadway that is not conducive to industrial development. This standard is met.
f. The variance requested is the minimum variance that would alleviate the hardship.
Response: The proposed use of a 550-foot cul-de-sac rather than a 200 -foot cul-de-sac or a through street to Blake Road represents the minimum reduction necessary to alleviate the site design constraints discussed above. If the property were flatter, then a roadway profile could be established that would accommodate large, flat industrial sites and still maintained traffic safety. Alternately, if the site was planned for non-industrial uses, the need for large, flat sites would be reduced, the roadway could be established with a continuous slope, and the site could be designed with smaller multi-family residential or commercial buildings built into the hillside. However, planning for the Tonquin

Employment Area as well as current zoning do not support residential or commercial uses, and the applicant is not seeking authorization for them.

As noted, approving the Variance will enable efficient use of the site by accommodating large industrial buildings while still maintaining site access. At the scale of a 46.5 -acre development site, authorizing a 550 -foot cul-de-sac rather than a 200 -foot cul-de-sac or a through street to Blake Road is a relatively small variance. Granting the variance allows for a financially viable development that will result in $535,000 \mathrm{SF}$ of industrial building square footage, which will benefit the City as well as the region, consistent with the longterm vision for the Tonquin Employment Area. This standard is met.

In order to provide a through connection to Blake Road or limit the cul-de-sac to 200 feet as specified in the code, the resulting building areas would have to be considerably smaller, thereby constraining the spectrum of potential industrial businesses that would otherwise be likely to occupy the site, while also decreasing the financial viability of the project for the developer. The proposed Variance is a reasonable and measured modification that serves to offset the hardship imposed by strict application of the code standards.

## Division V. - Community Design

## Chapter 16.90-Site Planning

### 16.90.020 - Site Plan Review

A. Site Plan Review Required

Site Plan review is required prior to any substantial change to a site or use that does not meet the criteria of a minor or major modification, issuance of building permits for a new building or structure, or for the substantial alteration of an existing structure or use.
For the purposes of Section 16.90.020, the terms "substantial change" and "substantial alteration" mean any development activity as defined by this Code that generally requires a building permit and may exhibit one or more of the following characteristics:

1. The activity alters the exterior appearance of a structure, building or property and is not considered a modification.
2. The activity involves changes in the use of a structure, building, or property from residential to commercial or industrial and is not considered a modification.
3. $\quad$ The activity involves non-conforming uses as defined in Chapter 16.48.
4. The activity constitutes a change in a City approved plan, per Section 16.90.020 and is not considered a modification.
5. The activity is subject to site plan review by other requirements of this Code.6.The activity increases the size of the building by more than $100 \%$ (i.e. the building more than doubles in size), regardless of whether it would be considered a major or minor modification.
Response: The applicant is requesting Site Plan Review for the five proposed buildings. Since the site is currently undeveloped, the proposal does not qualify as a major or minor modification or a substantial alteration. This standard is met.
B. Exemption to Site Plan Requirement
6. Single and two family uses
7. Manufactured homes located on individual residential lots per Section 16.46.010, but including manufactured home parks.
Response: No single and two family uses, or manufactured homes are proposed as part of this development. This standard does not apply.
C. Reserved
D. Required Findings

No site plan approval will be granted unless each of the following is found:

1. The proposed development meets applicable zoning district standards and design standards in Division II, and all provisions of Divisions V, VI, VIII and IX.
2. The proposed development can be adequately served by services conforming to the Community Development Plan, including but not limited to water, sanitary facilities, storm water, solid waste, parks and open space, public safety, electric power, and communications.
3. Covenants, agreements, and other specific documents are adequate, in the City's determination, to assure an acceptable method of ownership, management, and maintenance of structures, landscaping, and other on-site features.
Response: Findings that demonstrate compliance with the applicable development standards from Divisions II, IV, V, VI, and VIII are presented herein. Division IX does not apply as there are no historic resources on site. As substantiated by relevant portions of those findings, the subject development has been designed in a manner that will ensure adequate service can be provided by existing public and private utilities. Following construction, ongoing maintenance of the site and related improvements will be provided by the property owner(s) and building tenants. This standard is met.
4. The proposed development preserves significant natural features to the maximum extent feasible, including but not limited to natural drainage ways, wetlands, trees, vegetation (including but not limited to environmentally sensitive lands), scenic views, and topographical features, and conforms to the applicable provisions of Division VIII of this Code and Chapter 5 of the Community Development Code.
Response: The natural features that have been documented at the site are existing trees and wetlands (Attachments 14, 15, and 18). Findings below address tree preservation and wetland vegetated corridor mitigation; in particular, standards from Division VIII. This standard is met.
5. For developments that are likely to generate more than 400 average daily trips (ADTs), or at the discretion of the City Engineer, the applicant must provide adequate information, such as a traffic impact analysis (TIA) or traffic counts, to demonstrate the level of impact to the surrounding transportation system. The developer is required to mitigate for impacts attributable to the project, pursuant to TIA requirements in Section 16.106.080 and rough proportionality requirements in Section 16.106.090. The determination of impact or effect and the scope of the impact study must be coordinated with the provider of the affected transportation facility.
Response: The applicant has submitted a Traffic Impact Analysis that demonstrates the anticipated effect of the proposed development on the surrounding transportation system (Attachment 11). The analysis has been prepared consistent with provisions contained in Section 16.106.080. This standard is met.
6. The proposed commercial, multi-family, institutional or mixed-use development is oriented to the pedestrian and bicycle, and to existing and planned transit facilities. Urban design standards include the following:
a. Primary, front entrances are located and oriented to the street, and have significant articulation and treatment, via facades, porticos, arcades, porches, portal, forecourt, or stoop to identify the entrance for pedestrians. Additional entrance/exit points for buildings, such as a postern, are allowed from secondary streets or parking areas.
b. Buildings are located adjacent to and flush to the street, subject to landscape corridor and setback standards of the underlying zone.
c. The architecture of buildings are oriented to the pedestrian and designed for the long term and be adaptable to other uses. Aluminum, vinyl, and $T$-111 siding are prohibited. Street facing elevations have windows, transparent fenestration, and divisions to break up the mass of any window. Roll up and sliding doors are acceptable. Awnings that provide a minimum 3 feet of shelter from rain are required unless other architectural elements are provided for similar protection, such as an arcade.
d. As an alternative to the standards in Section 16.90.020.D.6.a-c, the following Commercial Design Review Matrix may be applied to any commercial, multifamily, institutional or mixed use development (this matrix may not be utilized for developments within the Old Town Overlay). A development must propose a minimum of 60 percent of the total possible points to be eligible for exemption from the standards in Section 16.90.020.D.6.a-c. In addition, a development proposing between 15,001 and 40,000 square feet of floor area, parking or seating capacity and proposing a minimum of 80 percent of the total possible points from the matrix below may be reviewed as a Type II administrative review, per the standards of Section 16.72.010.A.2.
e. As an alternative to the standards in Sections 16.90.020.D.6.a-c, the Old Town Design Standards (Chapter 16.162) may be applied to achieve this performance measure.
f. As an alternative to the standards in Sections 16.90.020.D.6.a. -e, an applicant may opt to have a design review hearing before the Planning Commission to demonstrate how the proposed development meets or exceeds the objectives in Section 16.90.010.B of this Code. This design review hearing will be processed as a Type IV review with public notice and a public hearing.
Response: The project site is located within the Employment Industrial zone and the proposed uses are industrial rather than commercial, multi-family, institutional or mixed-use. This standard does not apply.
7. Industrial developments provide employment opportunities for citizens of Sherwood and the region as a whole. The proposed industrial development is designed to enhance areas visible from arterial and collector streets by reducing the "bulk" appearance of large buildings. Industrial design standards include the following:
a. Portions of the proposed industrial development within 200 feet of an arterial or collector street and visible to the arterial or collector (i.e. not behind another building) must meet any four of the following six design criteria:
(1) A minimum $15 \%$ window glazing for all frontages facing an arterial or collector.
(2) A minimum of two (2) building materials used to break up vertical facade street facing frontages (no T-111 or aluminum siding).
(3) Maximum thirty-five (35) foot setback for all parts of the building from the property line separating the site from all arterial or collector streets (required visual corridor falls within this maximum setback area).
(4) Parking is located to the side or rear of the building when viewed from the arterial or collector.
(5) Loading areas are located to the side or rear of the building when viewed from the arterial or collector. If a loading area is visible from an arterial or collector, it must be screened with vegetation or a screen made of materials matching the building materials.
(6) All roof-mounted equipment is screened with materials complimentary to the building design materials.
Response: The project will meet design criteria $1,2,5$, and 6 . The proposed buildings will utilize a minimum of $15 \%$ window glazing on all frontages facing an arterial (SW TualatinSherwood Road and SW 124th Avenue) and a minimum of two building materials used to break up vertical façade street facing frontages, see Sheets AA2.10, BA2.10, CA2.10CA2.11, DA2.10-DA2.11, and EA2.10 (Attachment 6). Loading areas are located to the side or rear of the building when viewed from the public right-of-way (Sheet A0.10), and all roof-mounted equipment is screened with parapets constructed of the same materials as the adjoining portions of the buildings. These design elements enhance the overall building façade visible from the public right-of-way and reduce the "bulk" appearance of the large buildings. This standard is met.
b. As an alternative to Section 16.90.020.D.7.a, an applicant may opt to have a design review hearing before the Planning Commission to demonstrate how the proposed development meets or exceeds the applicable industrial design objectives below (this design review hearing will be processed as a Type IV review):
(1) Provide high-value industrial projects that result in benefits to the community, consumers and developers.
(2) Provide diversified and innovative working environments that take into consideration community needs and activity patterns.
(3) Support the City's goals of economic development.
(4) Complement and enhance projects previously developed under the industrial design standards identified in Section 16.90.020.D.7.
(5) Enhance the appearance of industrial developments visible from arterials and collectors, particularly those considered "entrances" to Sherwood, including but not limited to: Highway 99W, Tualatin-Sherwood Road and Oregon Street.
(6) Reduce the "bulk" appearance of large industrial buildings as viewed from the public street by applying exterior features such as architectural articulation, windows and landscaping.
(7) Protect natural resources and encourage integration of natural resources into site design (including access to natural resources and open space amenities by the employees of the site and the community as a whole).
Response: The project will meet the provisions of section 16.90.020.D.7.a, so the applicant is not seeking approval under the provisions of subsection b . This section does not apply.
8. Driveways that are more than twenty-four (24) feet in width shall align with existing streets or planned streets as shown in the Local Street Connectivity Map in the adopted Transportation System Plan (Figure 17), except where prevented by topography, rail lines, freeways, pre-existing development, or leases, easements, or covenants.
Response: Access to the site is proposed by the construction of SW Cipole Place, with no additional driveways proposed on SW Tualatin-Sherwood Road or SW 124th Place. There are no existing or planned streets along SW Cipole Place so there is no opportunity for driveways to align with these streets. This standard does not apply.
E. Approvals

The application is reviewed pursuant to Chapter 16.72 and action taken to approve, approve with conditions, or deny the application for site plan review. Conditions may be imposed by the Review Authority if necessary to fulfill the requirements of the adopted Comprehensive Plan, Transportation System Plan or the Zoning and Community Development Code. The action must include appropriate findings of fact as required by Section 16.90.020. The action may be appealed to the Council in accordance with Chapter 16.76.
F. Time Limits

Site plan approvals are void after two (2) years unless construction on the site has begun, as determined by the City. The City may extend site plan approvals for an additional period not to exceed one (1) year, upon written request from the applicant showing adequate cause for such extension, and payment of an extension application fee as per Section 16.74.010. A site plan approval granted on or after January 1, 2007 through December 31, 2009, is extended until December 31, 2013.
Response: The submittal will meet the provisions of the section above. This standard is met.

## Chapter 16.92-Landscaping

16.92.010-Landscaping Plan Required

All proposed developments for which a site plan is required pursuant to Section 16.90 .020 shall submit a landscaping plan that meets the standards of this Chapter. All areas not occupied by structures, paved roadways, walkways, or patios shall be landscaped or maintained according to an approved site plan.
Response: The proposed development has been designed to meet the provisions of Chapter 16.92 Landscaping, of the Sherwood Municipal Code, please refer to the landscape sheets in Attachment 6. This standard is met.

### 16.92.020-Landscaping Materials

A. Type of Landscaping

Required landscaped areas shall include an appropriate combination of native evergreen or deciduous trees and shrubs, evergreen ground cover, and perennial plantings. Trees to be planted in or adjacent to public rights-of-way shall meet the requirements of this Chapter. Plants may be selected from the City's "Suggested Plant Lists for Required Landscaping Manual" or suitable for the Pacific Northwest climate and verified by a landscape architect or certified landscape professional.

1. Ground Cover Plants
a. All of the landscape that is not planted with trees and shrubs must be planted in ground cover plants, which may include grasses. Mulch is not a substitute for ground cover, but is allowed in addition to the ground cover plants.
b. Ground cover plants other than grasses must be at least the four-inch pot size and spaced at distances appropriate for the plant species. Ground cover plants must
be planted at a density that will cover the entire area within three (3) years from the time of planting.
2. Shrubs
a. All shrubs must be of sufficient size and number to be at full growth within three (3) years of planting.
b. $\quad$ Shrubs must be at least the one-gallon container size at the time of planting.
3. Trees
a. $\quad$ Trees at the time of planting must be fully branched and must be a minimum of two (2) caliper inches and at least six (6) feet in height.
b. Existing trees may be used to meet the standards of this chapter, as described in Section 16.92.020.C.2.
Response: Attachment 6 presents the proposed landscaping plans for the subject site. As required by the standards cited above, trees will have a minimum caliper of two inches at time of installation, shrubs will have a minimum container size of one gallon, and groundcovers will have a minimum pot size of four inches. Final landscaping plans will be submitted as part of materials provided to the City of Sherwood for review and approval of site and building permits. The review of these plans will confirm installation of trees, shrubs, and groundcovers at or above the minimum specifications notes above. This standard is met.

## B. Plant Material Selection and Preparation

1. Required landscaping materials shall be established and maintained in a healthy condition and of a size sufficient to meet the intent of the approved landscaping plan. Specifications shall be submitted showing that adequate preparation of the topsoil and subsoil will be undertaken.
2. Landscape materials should be selected and sited to produce a hardy and droughtresistant landscape area. Selection of the plants should include consideration of soil type, and depth, the amount of maintenance required, spacing, exposure to sun and wind, the slope and contours of the site, and compatibility with existing native vegetation preserved on the site.
Response: The preliminary landscaping plans have been prepared consistent with criteria 1 and 2. Final landscaping plans will be submitted as part of materials provided to the City of Sherwood for review and approval of site and building permits. The review of these plans will confirm installation of trees, shrubs, and groundcovers at or above the minimum specifications notes above. This standard is met.

## C. Existing Vegetation

1. All developments subject to site plan review per Section 16.90.020 and required to submit landscaping plans per this section shall preserve existing trees, woodlands and vegetation on the site to the maximum extent possible, as determined by the Review Authority, in addition to complying with the provisions of Section 16.142.(Parks, Trees and Open Space) and Chapter 16.144 (Wetland, Habitat, and Natural Resources).
Response: As noted above, the site is currently fully vegetated (Attachment 18). A total of 505 of the existing trees are proposed for retention, the majority of which are located along SW TualatinSherwood Road or within the wetland areas, Tracts A and D. Retention of additional trees is not possible due to the footprint and locations of the proposed buildings, as well as the need to provide adequate vehicular parking and circulation areas for the propose uses. Approximately 98.8 percent of the trees proposed for removal will be replanted on site through installation of 502 deciduous and evergreen trees that will be distributed along the perimeter of the site, around the edges of buildings, and within the vehicle parking areas (Attachment 6, Sheets L0.01-L1.21).

As discussed in response to criteria from Section 16.142.070, the proposed tree removal does not violate a minimum $30 \%$ remaining canopy requirement.

Based on these findings, the proposed development preserves existing trees to the maximum extent possible, while allowing the intensity of new Industrial development expected in the El zone. The number of trees retained within the site and the number of trees planted within the development, upon maturity, will provide a comparable canopy. Please see below for additional findings in response to Section 16.142. This standard is met.
2. Existing vegetation, except those plants on the Nuisance Plants list as identified in the "Suggested Plant Lists for Required Landscaping Manual" may be used to meet the landscape standards, if protected and maintained during the construction phase of the development.
a. If existing trees are used, each tree six (6) inches or less in diameter counts as one (1) medium tree.
b. Each tree that is more than six (6) inches and up to nine (9) inches in diameter counts as two (2) medium trees.
c. Each additional three (3) inch diameter increment above nine (9) inches counts as an additional medium tree.
Response: The applicant proposes to retain 505 existing trees within the boundaries of the site, as shown on Sheets C7.0-C7.6 (Attachment 6) and detailed in the arborist report (Attachment 18). While existing trees will be utilized to satisfy the tree canopy standards of Chapter 16.142, existing trees are not proposed to be used to satisfy any landscape standards. This standard does not apply.
D. Non-Vegetative Features

1. Landscaped areas as required by this Chapter may include architectural features interspersed with planted areas, such as sculptures, benches, masonry or stone walls, fences, rock groupings, bark dust, semi-pervious decorative paving, and graveled areas.
Response: Landscaping coverage calculations presented by the applicant are exclusive of any of the features listed above. The total landscaping coverage exceeds the minimum requirements despite not counting these areas. This standard does not apply.
2. Impervious paving shall not be counted toward the minimum landscaping requirements unless adjacent to at least one (1) landscape strip and serves as a pedestrian pathway.
Response: Due to the amount of trees retained, pedestrian pathways are not proposed to be counted towards the minimum landscaping standards. This standard does not apply.
3. Artificial plants are prohibited in any required landscaped area.

Response: Artificial plants are not proposed as part of required landscaping to satisfy applicable development standards. This standard is met.

### 16.92.030 - Site Area Landscaping and Perimeter Screening Standards

## A. Perimeter Screening and Buffering

1. Perimeter Screening Separating Residential Zones:

A minimum six-foot high sight-obscuring wooden fence, decorative masonry wall, or evergreen screen, shall be required along property lines separating single and two-family uses from multi- family uses, and along property lines separating residential zones from
commercial, institutional/public or industrial zones subject to the provisions of Chapter 16.48.020 (Fences, Walls and Hedges).
a. For new uses adjacent to inventoried environmentally sensitive areas, screening requirements shall be limited to vegetation only to preserve wildlife mobility. In addition, the Review Authority may require plants and other landscaping features in locations and sizes necessary to protect the privacy of residences and buffer any adverse effects of adjoining uses.
b. The required screening shall have breaks, where necessary, to allow pedestrian access to the site. The design of the wall or screening shall also provide breaks or openings for visual surveillance of the site and security.
c. Evergreen hedges used to comply with this standard shall be a minimum of thirtysix (36) inches in height at maturity, and shall be of such species, number and spacing to provide the required screening within one (1) year after planting.
Response: As detailed in the introduction, the site does not abut residential zoning or residential uses. This standard does not apply.
2. Perimeter Landscaping Buffer
a. A minimum ten (10) foot wide landscaped strip comprised of trees, shrubs and ground cover shall be provided between off-street parking, loading, or vehicular use areas on separate, abutting, or adjacent properties.
Response: As shown on Attachment 6 (Sheets L2.1, L2.2, and L1.0), a perimeter landscape buffer is provided along SW Tualatin-Sherwood Road, SW 124th Avenue, and along shared property lines along the west and south boundaries of the site. This buffer is at least 10 feet wide and increases to 15 feet in width along SW Tualatin-Sherwood Road and SW 124th Avenue to comply with the applicable Visual Corridor standards. This standard is met.
b. $\quad$ The access drives to a rear lots in the residential zone (i.e. flag lot) shall be separated from abutting property(ies) by a minimum of forty-two-inch sightobscuring fence or a forty-two-inch to an eight (8) feet high landscape hedge within a four-foot wide landscape buffer. Alternatively, where existing mature trees and vegetation are suitable, Review Authority may waive the fence/buffer in order to preserve the mature vegetation.
Response: This site is zoned El; thus, this standard is not applicable.

## 3. Perimeter Landscape Buffer Reduction

If the separate, abutting property to the proposed development contains an existing perimeter landscape buffer of at least five (5) feet in width, the applicant may reduce the proposed site's required perimeter landscaping up to five (5) feet maximum, if the development is not adjacent to a residential zone. For example, if the separate abutting perimeter landscaping is five (5) feet, then applicant may reduce the perimeter landscaping to five (5) feet in width on their site so there is at least five (5) feet of landscaping on each lot.
Response: No reductions to the perimeter landscape buffer width of 10 feet are proposed through this application. This standard does not apply.
B. Parking Area Landscaping

1. Purpose

The standard is a landscape treatment that uses a combination of trees, shrubs, and ground cover to provide shade, storm water management, aesthetic benefits, and
screening to soften the impacts of large expanses of pavement and vehicle movement. It is applied to landscaped areas within and around the parking lot and loading areas.
2. Definitions
a. Parking Area Landscaping: Any landscaped area on the site that is not required as perimeter landscaping § 16.92.030 (Site Landscaping and Screening).
b. Canopy Factor
(1) Landscape trees are assigned a canopy factor to determine the specific number of required trees to be planted. The canopy factor is calculated based on the following formula:
Canopy Factor $=$ Mature Height (in feet) $\times$ Canopy Spread (in feet) $\times$ Growth Rate Factor x . 01
(2) Growth Rate Factor: The growth rate factor is three (3) for fast-growing trees, two (2) for medium growing trees, and one (1) for slow growing trees. The growth rate of a tree is identified in the "Suggested Plant Lists for Required Landscaping Manual."
Response: The submitted landscaping plans provide detailed information and calculations on the classification of proposed landscaping trees as either "small," "medium," or "large" canopy trees, which are based on the methods described above. This standard is met.

## 3. Required Landscaping

There shall be at least forty-five (45) square feet parking area landscaping for each parking space located on the site. The amount of required plant materials are based on the number of spaces as identified below.
Response: For the 671 parking spaces that are proposed, a total of 30,195 SF ( 0.69 acres) of parking area landscaping is required. After excluding the required parking lot perimeter screening ( 0.7 acres), the site (excluding tracts) has over 4.5 acres of parking area landscaping, using the definition in subsection $2 . a$, above. This standard is met.
4. Amount and Type of Required Parking Area Landscaping
a. Number of Trees required based on Canopy Factor

Small trees have a canopy factor of less than forty (40), medium trees have a canopy factor from forty (40) to ninety (90), and large trees have a canopy factor greater than ninety (90);
(1) Any combination of the following is required:
(i) One (1) large tree is required per four (4) parking spaces;
(ii) One (1) medium tree is required per three (3) parking spaces; or
(iii) One (1) small tree is required per two (2) parking spaces.
(iv) At least five (5) percent of the required trees must be evergreen.
(2) Street trees may be included in the calculation for the number of required trees in the parking area.
Response: As shown on Attachment 6 (Sheet L0.02), 0 "large" trees, 170 "medium" trees, and 332 "small" trees are proposed for installation. The ratios cited above would permit a maximum of 1,174 parking spaces based on the number of "large," "medium," and "small" trees proposed for installation. As only 671 parking spaces are provided on site, this standard is met.
b. Shrubs:
(1) Two (2) shrubs are required per each space.

Response: Given 671 proposed parking spaces, the landscaping plans are required to include at least 1,342 shrubs. The landscape plans in Attachment 6 illustrate the areas where shrubs are proposed to satisfy this provision, as will be verified at the time of construction permits. This standard is met.
(2) For spaces where the front two (2) feet of parking spaces have been landscaped instead of paved, the standard requires one (1) shrub per space. Shrubs may be evergreen or deciduous.
Response: The front two feet of parking spaces are proposed to be paved. This standard does not apply.
c. Ground cover plants:
(1) Any remainder in the parking area must be planted with ground cover plants.
(2) The plants selected must be spaced to cover the area within three (3) years. Mulch does not count as ground cover.
Response: Groundcover plants and turf are proposed as the balance of landscaping not otherwise accounted for by shrubs and trees (Attachment 6). The proposed density and spacing is anticipated to achieve full coverage within three years of installation. This standard is met.
5. Individual Landscape Islands Requirements
a. Individual landscaped areas (islands) shall be at least ninety (90) square feet in area and a minimum width of five (5) feet and shall be curbed to protect the landscaping.
b. $\quad$ Each landscape island shall be planted with at least one (1) tree.
c. Landscape islands shall be evenly spaced throughout the parking area.

Response: Each of the landscaping islands proposed within the parking area is at least eight feet wide and at least 140 SF in area. All islands are sufficiently dimensioned to support at least one tree, and are relatively evenly distributed throughout the parking area. These standards are met.
d. Landscape islands shall be distributed according to the following:
(1) Residential uses in a residential zone: one (1) island for every eight (8) contiguous parking spaces.
Response: The site is zoned EI, which is not a residential zone. This standard is not applicable.
(2) Multi or mixed-uses, institutional and commercial uses: one (1) island for every ten (10) contiguous parking spaces.
Response: The proposed industrial use is not multi or mixed-use, institutional, or commercial. This standard is not applicable.
(3) Industrial uses: one (1) island for every twelve (12) contiguous parking spaces.
Response: As discussed in section 16.94.020-Off-Street Parking Standards and illustrated in the site plan C3.0 (Attachment 6), the new parking rows will generally have an island once every 8-9 cars. This standard is met.
e. $\quad$ Storm water bio-swales may be used in lieu of the parking landscape areas and may be included in the calculation of the required landscaping amount.
Response: Three stormwater bio-swales (extended dry basins) are proposed as part of the project. The accumulated area of all three bioswales has not been included within the parking landscape area calculations as sufficient parking area landscaping is provided without counting the stormwater facilities. This standard does not apply.

## f. Exception to Landscape Requirement

Linear raised or marked sidewalks and walkways within the parking areas connecting the parking spaces to the on-site buildings may be included in the calculation of required site landscaping provide that it:
(1) Trees are spaced a maximum of thirty (30) feet on at least one (1) side of the sidewalk.
(2) The minimum unobstructed sidewalk width is at least six (6) feet wide.

The sidewalk is separated from the parking areas by curbs, bollards, or other means on both sides.
Response: The landscaping exception described in the criterion cited above is not proposed as part of the subject project. This standard is not applicable.

## 6. Landscaping at Points of Access

When a private access-way intersects a public right-of-way or when a property abuts the intersection of two (2) or more public rights-of-way, landscaping shall be planted and maintained so that minimum sight distances shall be preserved pursuant to Section 16.58.010.

Response: Landscaping at the proposed cul-de-sac, SW Cipole Place, and corner of SW TualatinSherwood Road and SW 124th Avenue have been selected to maintain minimum sight distances, as required by Section 16.58.010. This standard is met.

## 7. Exceptions

a. For properties with an environmentally sensitive area and/or trees or woodlands that merit protection per Chapters 16.142 (Parks, Trees and Open Space) and 16.144 (Wetland, Habitat and Natural Areas) the landscaping standards may be reduced, modified or "shifted" on-site where necessary in order to retain existing vegetation that would otherwise be removed to meet the above referenced landscaping requirements.
b. The maximum reduction in required landscaping buffer permitted through this exception process shall be no more than fifty (50) percent. The resulting landscaping buffer after reduction may not be less than five (5) feet in width unless otherwise permitted by the underlying zone. Exceptions to the required landscaping may only be permitted when reviewed as part of a land use action application and do not require a separate variance permit.
Response: As shown on Sheets C7.0-C7.6 in Attachment 6, many of the existing trees within the site cannot be retained due to the proposed placement of building footprints and vehicle parking and circulation areas. The available point of access will solely be from SW Cipole Place. The proposed cul-de-sac will dictate the location of these improvements and leave little to no flexibility for tree retention. As such, the applicant is not seeking the option of relief from the landscaping standards cited above. This standard does not apply.
C. Screening of Mechanical Equipment, Outdoor Storage, Service and Delivery Areas All mechanical equipment, outdoor storage and manufacturing, and service and delivery areas, shall be screened from view from all public streets and any adjacent residential zones. If unfeasible to fully screen due to policies and standards, the applicant shall make efforts to minimize the visual impact of the mechanical equipment.
Response: All new service and delivery areas will be screened from view from all public streets, and there are no adjacent residential zones. Trash enclosures are proposed in five areas of the site to satisfy refuse disposal needs of the future warehousing and light industrial needs. These enclosures will be screened by enclosures constructed with concrete walls and operable gates. Except for rooftop mechanical units, which will be screened by building parapets, no other mechanical equipment or outdoor storage is proposed at this time. However, the site use is speculative in nature and future tenants may require these features for their operations. The applicable approval process will be pursued if required to meet tenant needs. This standard is met.

## D. Visual Corridors

Except as allowed by subsection 6. above, new developments shall be required to establish landscaped visual corridors along Highway 99W and other arterial and collector streets, consistent with the Natural Resources and Recreation Plan Map, Appendix C of the Community Development Plan, Part II, and the provisions of Chapter 16.142 (Parks, Trees, and Open Space). Properties within the Old Town Overlay are exempt from this standard.
Response: The proposed landscaping plans have been designed to provide approximately 15 -foot-wide Visual Corridors along SW Tualatin-Sherwood Road and SW 124th Avenue. The responses to Chapter 16.142 address the approval standards within that chapter. This standard is met.

### 16.92.040 - Installation and Maintenance Standards

A. Installation

All required landscaping must be in-ground, except when in raised planters that are used to meet minimum Clean Water Services storm water management requirements. Plant materials must be installed to current nursery industry standards. Plant materials must be properly supported to ensure survival. Support devices such as guy wires or stakes must not interfere with vehicular or pedestrian movement.
B. Maintenance and Mitigation of Landscaped Areas

1. Maintenance of existing non-invasive native vegetation is encouraged within a development and required for portions of the property not being developed.
2. All landscaping shall be maintained in a manner consistent with the intent of the approved landscaping plan.
3. Any required landscaping trees removed must be replanted consistent with the approved landscaping plan and comply with § 16.142, (Parks, Trees and Open Space).
Response: The proposed landscaping plans have been designed to ensure compliance with the standards cited above. Ongoing maintenance of installed landscaping will be the responsibility of the property owner(s), as required by these standards.

## C. Irrigation

The intent of this standard is to ensure that plants will survive the critical establishment period when they are most vulnerable due to lack of watering. All landscaped areas must provide an irrigation system, as stated in Option 1, 2, or 3.

1. Option 1: A permanent built-in irrigation system with an automatic controller installed.
2. Option 2: An irrigation system designed and certified by a licensed landscape architect or other qualified professional as part of the landscape plan, which provides sufficient water
to ensure that the plants become established. The system does not have to be permanent if the plants chosen can survive independently once established.
3. Option 3: Irrigation by hand. If the applicant chooses this option, an inspection will be required one (1) year after final inspection to ensure that the landscaping has become established.
Response: As noted on Attachment 6 Sheet L0.01, permanent irrigation is proposed for this project. This standard is met.

## D. Deferral of Improvements

Landscaping shall be installed prior to issuance of occupancy permits, unless security equal to one hundred twenty-five (125) percent of the cost of the landscaping is filed with the City. "Security" may consist of a performance bond payable to the City, cash, certified check, or other assurance of completion approved by the City. If the installation of the landscaping is not completed within one (1) year, the security may be used by the City to complete the installation.
Response: If landscaping is not installed prior to occupancy permits, the applicant will provide the appropriate guarantees as required by the City. This standard is met.

## Chapter 16.94-Off-Street Parking and Loading

### 16.94.010 - General Requirements

A. Off-Street Parking Required

No site shall be used for the parking of vehicles until plans are approved providing for off-street parking and loading space as required by this Code. Any change in uses or structures that reduces the current off-street parking and loading spaces provided on site, or that increases the need for off-street parking or loading requirements shall be unlawful and a violation of this Code, unless additional off-street parking or loading areas are provided in accordance with Section 16.94.020, or unless a variance from the minimum or maximum parking standards is approved in accordance with Chapter 16.84 Variances.
Response: The project site is presently undeveloped except for unimproved off-street parking associated with the former single-family dwelling and associated agricultural outbuildings which have now been demolished. As discussed in the findings below and attached site and parking plans in Attachment 6, the proposed project will provide off-street parking as required to meet market demand for industrial parks in the Tualatin-Sherwood Road corridor.

## B. Deferral of Improvements

Off-street parking and loading spaces shall be completed prior to the issuance of occupancy permits, unless the City determines that weather conditions, lack of available surfacing materials, or other circumstances beyond the control of the applicant make completion impossible. In such circumstances, security equal to one hundred twenty five (125) percent of the cost of the parking and loading area is provided the City. "Security" may consist of a performance bond payable to the City, cash, certified check, or other assurance of completion approved by the City. If the installation of the parking or loading area is not completed within one (1) year, the security may be used by the City to complete the installation.
Response: No deferral of improvements is anticipated at this time. Off-street parking and loading spaces are proposed for completion prior to issuance of occupancy permits. Should future circumstances necessitate a deferral, the required security will be provided. This standard is met.
C. Options for Reducing the Required Parking Spaces

1. Two (2) or more uses or, structures on multiple parcels of land may utilize jointly the same parking and loading spaces when the peak hours of operation do not substantially overlap, provided that satisfactory evidence is presented to the City, in the form of deeds, leases, or contracts, clearly establishing the joint use.
a. Within commercial, institutional and public, or industrial zones, shared parking may be provided on lots that are within five hundred (500) feet of the property line of the use to be served.
b. $\quad$ Shared parking is allowed if the application can show that the combined peak use is available by a parking study that demonstrates:
(1) There is a sufficient number of parking spaces to accommodate the requirements of the individual businesses; or
(2) That the peak hours of operation of such establishments do not overlap, and
(3) That an exclusive permanent easement over a delineated area has been granted for parking space use.
Response: Joint use of the same parking spaces is not proposed as the development as a whole has sufficient parking. This standard does not apply.
2. Mixed use projects are developments where a variety of uses occupies a development project or complex. For example, an eating establishment, professional office building and movie theater are all components of a mixed use site. It does not include a secondary use within a primary use such as an administrative office associated with a retail establishment. In mixed-use projects, the required minimum vehicle parking shall be determined using the following formula:
a. Primary use: i.e. that with the largest proportion of total floor area within the development at one hundred (100) percent of the minimum vehicle parking required for that use.
b. Secondary Use: i.e. that with the second largest percentage of total floor area within the development, at ninety (90) percent of the vehicle parking required for that use.
c. Subsequent use or uses, at eighty (80) percent of the vehicle parking required for that use.
Response: To be conservative, all parking calculations have been performed using 100\% of the minimum vehicle parking standard. As the proposed project is speculative, the exact mixture of warehousing to industrial users is unknown. However, this standard will be applied at the time tenant improvements are submitted for building permit review. This standard will be met.

## D. Prohibited Uses

Required parking, loading and maneuvering areas shall not be used for long-term storage or sale of vehicles or other materials, and shall not be rented, leased or assigned to any person or organization not using or occupying the building or use served.
Response: The proposed project does not include parking areas intended for long term storage or sale of vehicles or other materials. Parking shall be restricted to use by employees, visitors, deliveries, and others who are occupying or serving an allowed user. This standard is met.

## E. Location

1. Residential off-street parking spaces:
a. Shall be located on the same lot or development as the residential use.
b. $\quad$ Shall not include garages or enclosed buildings with the exception of a parking structure in multifamily developments where three (3) or more spaces are not individually enclosed. (Example: Underground or multi-level parking structures).
Response: Residential uses are not proposed with this application. This standard is not applicable.
2. For other uses, required off-street parking spaces may include adjacent on-street parking spaces, nearby public parking and shared parking located within five hundred (500) feet of the use. The distance from the parking, area to the use shall be measured from the nearest parking space to a building entrance, following a sidewalk or other pedestrian route. The right to use private off-site parking must be evidenced by a recorded deed, lease, easement, or similar written notarized letter or instrument.
Response: City Engineering staff has informed the applicant that on-street parking will not be permitted on SW Cipole Place. Similarly, on-street parking will not be available on SW TualatinSherwood Road or SW 124th Avenue. As a result, there is no opportunity to count on-street parking spaces. This standard does not apply.
3. Vehicle parking is allowed only on improved parking shoulders that meet City standards for public streets, within garages, carports and other structures, or on driveways or parking lots that have been developed in conformance with this code. Specific locations and types of spaces (car pool, compact, etc.) for parking shall be indicated on submitted plans and located to the side or rear of buildings where feasible.
a. All new development with forty (40) employees or more shall include preferential spaces for carpool/vanpool designation. Carpool and vanpool parking spaces shall be located closer to the main employee entrance than all other parking spaces with the exception of ADA parking spaces. Carpool/vanpool spaces shall be clearly marked as reserved for carpool/vanpool only.
b. Existing development may redevelop portions of designated parking areas for multi-modal facilities (transit shelters, park and ride, and bicycle parking), subject to meeting all other applicable standards, including minimum space standards.
Response: As demonstrated on Sheet C3.0 in Attachment 6, vehicle parking will occur in parking lots improved to City standards. As future employee counts are unknown, carpool and vanpool spaces are proposed to be addressed at the time of tenant improvement permits in accordance with these requirements. This standard will be met.

## F. Marking

All parking, loading or maneuvering areas shall be clearly marked and painted. All interior drives and access aisles shall be clearly marked and signed to show the direction of flow and maintain vehicular and pedestrian safety.
Response: As demonstrated on Sheet C3.0 in Attachment 6, all on-site parking, loading and maneuvering areas will be clearly marked, painted, and signed to City standards. This standard is met.

## G. Surface and Drainage

1. All parking and loading areas shall be improved with a permanent hard surface such as asphalt, concrete or a durable pervious surface. Use of pervious paving material is encouraged and preferred where appropriate considering soils, location, anticipated vehicle usage and other pertinent factors.
2. Parking and loading areas shall include storm water drainage facilities approved by the City Engineer or Building Official.
Response: All parking and loading areas will be improved with a permanent hard surface in compliance with stormwater requirements. This standard is met.
H. Repairs

Parking and loading areas shall be kept clean and in good repair. Breaks in paved surfaces shall be repaired. Broken or splintered wheel stops shall be replaced. Painted parking space boundaries and directional symbols shall be maintained in a readable condition.
Response: The proposed project site will include all new parking and loading areas that will be constructed for durability and compliance with City standards and will be maintained over the course of future occupancy. Since the parking areas are new, no maintenance of existing facilities is required. This standard is met.

## I. Parking and Loading Plan

An off-street parking and loading plan, drawn to scale, shall accompany requests for building permits or site plan approvals, except for single and two-family dwellings, and manufactured homes on residential lots. The plan shall show but not be limited to:

1. Delineation of individual parking and loading spaces and dimensions.
2. Circulation areas necessary to serve parking and loading spaces.
3. Location of accesses to streets, alleys and properties to be served, and any curb cuts.
4. Landscaping as required by Chapter 16.92.
5. Grading and drainage facilities.
6. Signing and bumper guard specifications.
7. Bicycle parking facilities as specified in Section 16.94.020.C.
8. Parking lots more than one (1) acre in size shall provide street-like features including curbs, sidewalks, and street trees or planting strips.
Response: Off-street parking and loading is included on Sheet C3.0 of Attachment 6, with associated landscaping illustrated on Sheets L1.10-L1.21. This standard is met.

## J. Parking Districts

The City may establish a parking district (i.e., permits or signage) in residential areas in order to protect residential areas from spillover parking generated by adjacent commercial, employment or mixed-use areas, or other uses that generate a high demand for parking. The district request shall be made to the City Manager, who will forward a recommendation to the City Council for a decision.
Response: The project site is not located in, adjacent, or near a residential area. This standard does not apply.
K. Structured parking and on-street parking are exempt from the parking space maximums in Section 16.94.020.A.

Response: The proposed project does not include structured parking or on-street parking This standard does not apply.

### 16.94.020 - Off-Street Parking Standards

A. Generally

Where square feet are specified, the area measured shall be the gross building floor area primary to the functioning of the proposed use. Where employees are specified, persons counted shall be those working on the premises, including proprietors, during the largest shift at peak season. Fractional space requirements shall be counted as a whole space. The Review Authority may determine alternate off - street parking and loading requirements for a use not specifically listed in this Section based upon the requirements of comparable uses.

| Table 1: Minimum and Maximum Parking Standards (Excerpts) |  |  |  |
| :---: | :---: | :---: | :---: |
| Land Use | Minimum Parking Standard | Maximum Permitted Parking Zone A ${ }^{1}$ | Maximum Permitted Parking Zone $B^{2}$ |
| Industrial | 1.6 | None | None |
| Warehouse (gross sf; parking ratios apply to warehouses 150,000 gsf or greater) | 0.3 | 0.4 | 0.5 |
| Note: <br> ${ }^{1}$ Parking Zone A reflects the maximum number of permitted vehicle parking spaces allowed for each listed land use. Parking Zone A areas include those parcels that are located within one-quarter ( $1 / 4 /$ mile walking distance of bus transit stops, one-half ( $1 / 2$ ) mile walking distance of light rail station platforms, or both, or that have a greater than twenty-minute peak hour transit service. <br> ${ }^{2}$ Parking Zone B reflects the maximum number of permitted vehicle parking spaces allowed for each listed land use. Parking Zone B areas include those parcels that are located at a distance greater than one-quarter ( $1 / 4$ ) mile walking distance of bus transit stops, one-half ( $1 / 2$ ) mile walking distance of light rail station platforms, or both. |  |  |  |

Response: The site is in Parking Zone B because it is not located within 0.25 miles walking distance for bus transit where regular 20-minute peak hour transit service is available, or within 0.5 miles walking distance for high capacity transit where 20-minute peak hour transit service is available (Attachment 10).

Per Table 1 of Section 16.94.020(A), industrial users must provide a minimum of 1.6 stalls per 1,000 SF of gross floor area, with no maximum number of spaces. This parking ratio applies to all industrial users including standalone warehouses of 149,999 SF or smaller. Table 1 also indicates that standalone warehouse uses in excess of 150,000 SF and within Parking Zone $B$ are subject to minimum and maximum parking ratios of 0.3 and 0.5 per 1,000 SF respectively. It appears that these lower warehouse parking ratios would most appropriately be applied to the increment of floor area that exceeds $150,000 \mathrm{SF}$, given that the rate for warehouses up to $149,000 \mathrm{SF}$ is 1.6 spaces per $1,000 \mathrm{SF}$.

All minimum parking ratios have been reduced by 20\% per the sensitive lands reduction factor in Section 16.94.020.B. 6 (Reduction in Required Parking Spaces). As discussed in findings elsewhere in this report, to preserve Wetlands A, B, and C and the CWS vegetated corridor, the applicant proposes the creation of tracts that reduce the developable area by approximately 10.9 acres or $24 \%$ of the site area. This land area is sufficient to account for the additional parking spaces that would otherwise have been required for the development.

The buildings do not have specific users at this time but are anticipated to contain a mix of light industrial, manufacturing, and warehouse/distribution tenants. To examine whether sufficient parking is available to accommodate a range of uses, parking calculations have been performed under two scenarios. To be consistent with the Conditional Use Permit request to allow Building C to be a standalone warehouse over 150,000 SF, Scenario 1 analyzes parking demand for Building C as $100 \%$ warehouse and the remaining buildings as $100 \%$ industrial. By contrast, Scenario 2analyzes parking demand assuming $100 \%$ industrial in all buildings, with no standalone warehouse 150,000 SF or larger.

| SCENARIO 1 (BUILDING C AS STANDALONE WAREHOUSE) MINIMUM AND MAXIMUM PARKING REQUIREMENTS |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Use | Building <br> Area | Minimum Required Parking Stalls before sensitive lands reduction | Minimum Required Parking Stalls after 20\% sensitive lands reduction | Maximum <br> Permitted Parking Stalls (Zone B) | Proposed Parking Stalls |
| Industrial (no standalone warehouse over 150,000 SF) | 351,902 SF | 563 | 451 | N/A | 490 |
| Building C as <br> standalone warehouse over $150,000 \mathrm{SF}$ | 183,292 SF | 250 (240 for first 150,000 SF plus 10 for next 33,292 SF) | 200 (192 for first 150,000 SF plus 8 for next 33,292 SF) | N/A (N/A for first 150,000 SF plus 16 for next 33,292 SF) | 181 |
| Total | 535,194 SF | 814 | 651 | N/A | 671 |


| SCENARIO 2 (ALL BUILDINGS AS LIGHT INDUSTRIAL) <br> MINIMUM AND MAXIMUM PARKING REQUIREMENTS |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Use | Minimum <br> Building <br> Area | Minimum <br> Required <br> Pefking Stalls <br> before sensitive <br> lands reduction | Required Parking <br> Stalls after 20\% <br> sensitive lands <br> reduction | Maximum <br> Permitted <br> Parking Stalls <br> (Zone B) | Proposed <br> Parking <br> Stalls |
| Industrial <br> (no <br> standalone <br> warehouse <br> over | $535,194 \mathrm{SF}$ | 858 | 687 | N/A | 671 |
| $150,000 \mathrm{SF}$ ) |  |  |  |  |  |

Under Scenario 1, a minimum of 651 parking spaces is required, while under Scenario 2, a minimum of 687 parking spaces is required. As illustrated on Sheets C3.0 and A0.10 in Attachment 6, the development will provide a total of 671 on-site parking spaces, and no on-street parking is available to serve the development. In Scenario 1, sufficient parking is available to serve a mix of industrial uses that includes a standalone warehouse and distribution use in Building C, if such a user should rent or purchase that building. In Scenario 2, the number of proposed parking spaces is approximately $2 \%$ below the nominal parking requirement for $100 \%$ industrial buildings, which certainly meets the intent of the Development Code to encourage appropriate use of land and promote orderly growth as outlined in Section 16.02.020 (particularly since gross building areas will be refined as the project moves closer to building permits). Moreover, the proposed number of parking spaces equates to a parking ratio of 1.25 spaces per 1,000 SF,
which is on par with market demand for industrial properties in the area. If needed, additional parking could be added during final design.

Under both scenarios, there is no applicable maximum number of parking spaces since industrial uses have no maximum ratio per Table 1, and under Scenario 1, the maximum of 16 spaces for the increment of Building C that exceeds 150,000 SF does not in itself subject the use to a maximum. This standard is met.
B. Dimensional and General Configuration Standards

1. Dimensions For the purpose of this Chapter, a "parking space" means a stall nine (9) feet in width and twenty (20) feet in length. Up to twenty five (25) percent of required parking spaces may have a minimum dimension of eight (8) feet in width and eighteen (18) feet in length so long as they are signed as compact car stalls.
Response: As demonstrated on Sheet(s) C3.0 and A0.10 of Attachment 6, all proposed parking spaces meet minimum stall dimensions for standard parking; no compact parking is proposed. This standard is met.
2. Layout

Parking space configuration, stall and access aisle size shall be of sufficient width for all vehicle turning and maneuvering. Groups of more than four (4) parking spaces shall be served by a driveway so as to minimize backing movements or other maneuvering within a street, other than an alley. All parking areas shall meet the minimum standards shown in the following table and diagram.


| Table 3 - Two-way Driving Aisle (Dimensions in Feet) |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | B | C | D | E | F | G | H | 1 |
| $45^{\circ}$ | 8.0 | 16.5 | 24.0 | 11.3 | 57.0 | 3.0 | 2.5 | 62.0 |
|  | 9.0 | 18.5 | 24.0 | 12.7 | 61.0 | 3.0 | 2.5 | 66.0 |
| $60^{\circ}$ | 8.0 | 17.0 | 24.0 | 9.2 | 58.0 | 3.0 | 2.5 | 63.0 |
|  | 9.0 | 19.5 | 24.0 | 10.4 | 63.0 | 3.0 | 2.5 | 68.0 |
| $75^{\circ}$ | 8.0 | 16.5 | 26.0 | 8.3 | 59.0 | 3.0 | 3.0 | 65.0 |
|  | 9.0 | 19.0 | 24.0 | 9.3 | 62.0 | 3.0 | 3.0 | 68.0 |
| $90^{\circ}$ | 8.0 | 18.0 | 26.0 | 8.0 | 56.0 | 3.0 | 3.0 | 62.0 |
|  | 9.0 | 20.0 | 24.0 | 9.0 | 58.0 | 3.0 | 3.0 | 64.0 |

Response: As demonstrated on Sheet(s) C3.0 and A0.10-A0.12 of Attachment 6, all stall and access aisles will be of sufficient width for all vehicle turning and maneuvering in compliance with the standards for two-way drive aisles. No parking spaces will require backing or other maneuvering within a public street. This standard is met.

## 3. Wheel Stops

a. Parking spaces along the boundaries of a parking lot or adjacent to interior landscaped areas or sidewalks shall be provided with a wheel stop at least four (4) inches high, located three (3) feet back from the front of the parking stall as shown in the above diagram.
b. Wheel stops adjacent to landscaping, bio-swales or water quality facilities shall be designed to allow storm water runoff.
c. The paved portion of the parking stall length may be reduced by three (3) feet if replaced with three (3) feet of low lying landscape or hardscape in lieu of a wheel stop; however, a curb is still required. In other words, the traditional three-foot vehicle overhang from a wheel stop may be low-lying landscaping rather than an impervious surface.
Response: The applicant proposes to provide vertical curb at each parking stall to prevent vehicles from traveling beyond the boundary of the parking areas. All stormwater from the development will be collected and directed to engineered extended dry basins for stormwater quality treatment. This standard is met.

## 4. Service Drives

Service drives shall be clearly and permanently marked and defined through use of rails, fences, walls, or other barriers or markers, and shall have minimum vision clearance area formed by the intersection of the driveway center line, the street right-of-way line, and a straight line joining said lines through points fifteen (15) feet from their intersection.
Response: No service drives are proposed for this project. This standard does not apply.
5. Credit for On-Street Parking
a. On-Street Parking Credit. The amount of off-street parking required shall be reduced by one (1) off-street parking space for every on-street parking space adjacent to the development. On-street parking shall follow the established configuration of existing on-street parking, except that angled parking may be allowed for some streets, where permitted by City standards.
b. The following constitutes an on-street parking space:
(1) Parallel parking, each twenty-four (24) feet of uninterrupted curb; Forty-five (45)/sixty (60) degree diagonal, each with ten (10) feet of curb; Ninety (90) degree (perpendicular) parking, each with eight (8) feet of curb;
(4) Curb space must be connected to the lot which contains the use;
(5) Parking spaces that would not obstruct a required clear vision area, nor any other parking that violates any law or street standard; and;
(6) On-street parking spaces credited for a specific use may not be used exclusively by that use, but shall be available for general public use at all times. No signs or actions limiting general public use of on-street spaces is permitted.
Response: City Engineering staff has informed the applicant that on-street parking will not be permitted on SW Cipole Place. Similarly, on-street parking will not be available on SW TualatinSherwood Road or SW 124th Avenue. This standard does not apply.

## 6. Reduction in Required Parking Spaces

Developments utilizing Engineered storm water bio-swales or those adjacent to environmentally constrained or sensitive areas may reduce the amount of required parking spaces by ten (10) percent when twenty-five (25) through forty-nine (49) parking spaces are required, fifteen (15) percent when fifty (50) and seventy-four (74) parking spaces are required and twenty (20) percent when more than seventy-five (75) parking spaces are required, provided the area that would have been used for parking is maintained as a habitat area or is generally adjacent to an environmentally sensitive or constrained area.
Response: As demonstrated on Sheets C5.0 in Attachment 6, the project site contains five tracts to mitigate environmental impacts on site. Two tracts are for wetland preservation and three tracts are for stormwater management facilities (bioswales or extended dry basins). The site and location of these wetlands constrains the site layout and removes a significant amount of land that could otherwise be used for parking. As all five buildings would be required to provide at least 75 parking stalls, all five buildings qualify for the $20 \%$ reduction. This standard is met.

## 7. Parking Location and Shared Parking

Owners of off-street parking facilities may post a sign indicating that all parking on the site is available only for residents, customers and/or employees, as applicable.
Response: This standard is optional, provides permissive direction, and is noted as a future option by the owner and/or tenants of the project site.
C. Bicycle Parking Facilities

## 1. General Provisions

a. Applicability. Bicycle parking spaces shall be provided for new development, changes of use, and major renovations, defined as construction valued at twentyfive (25) percent or more of the assessed value of the existing structure.
b. Types of Spaces. Bicycle parking facilities shall be provided in terms of short-term bicycle parking and long-term bicycle parking. Short-term bicycle parking is intended to encourage customers and other visitors to use bicycles by providing a convenient and readily accessible place to park bicycles. Long-term bicycle parking provides employees, students, residents, commuters, and others who generally stay at a site for at least several hours a weather-protected place to park bicycles.
c. Minimum Number of Spaces. The required total minimum number of bicycle parking spaces for each use category is shown in Table 4, Minimum Required Bicycle Parking Spaces.
d. Minimum Number of Long-term Spaces. If a development is required to provide eight (8) or more required bicycle parking spaces in Table 4, at least twenty-five (25) percent shall be provided as long-term bicycle with a minimum of one (1) long-term bicycle parking space.
e. $\quad$ Multiple Uses. When there are two or more primary uses on a site, the required bicycle parking for the site is the sum of the required bicycle parking for the individual primary uses.
Response: Per Table 4 of $16.94 .020(C)$, industrial users are required to provide a minimum of 2 bicycle parking spaces, or 1 per 40 parking spaces, whichever is greater. The following table summarizes the required and provided bicycle parking.

| Minimum Bicycle Parking Requirements |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Lot \& Building | Proposed <br> Vehicle <br> Parking <br> Stalls | Minimum <br> Required <br> Bicycle Spaces | Minimum <br> Required Long- <br> term Bicycle <br> Spaces | Proposed <br> Bicycle <br> Parking <br> Spaces |
| Lot 1 / <br> Building A | 152 | 4 | 0 | 4 |
| Lot 2 / <br> Building B | 124 | 4 | 0 | 4 |
| Lot 3 / <br> Building C | 181 | 5 | 0 | 6 |
| Lot 4 / <br> Building D | 127 | 4 | 0 | 4 |
| Lot 5 / <br> Building E | 87 | 3 | 0 | 4 |
| Total | 671 | $\mathbf{2 0}$ | $\mathbf{0}$ | $\mathbf{2 2}$ |

As summarized in the table, the project will provide sufficient bicycle parking as required per Table 4. This standard is met.
2. Location and Design.
a. General Provisions
(1) Each space must be at least two (2) feet by six (6) feet in area, be accessible without moving another bicycle, and provide enough space between the rack and any obstructions to use the space properly.
(2) There must be an aisle at least five (5) feet wide behind all required bicycle parking to allow room for bicycle maneuvering. Where the bicycle parking is adjacent to a sidewalk, the maneuvering area may extend into the right-of-way.
(3) Lighting. Bicycle parking shall be at least as well lit as vehicle parking for security.
(4) Reserved Areas. Areas set aside for bicycle parking shall be clearly marked and reserved for bicycle parking only.
(5) Bicycle parking in the Old Town Overlay District can be located on the sidewalk within the right-of-way. A standard inverted "U shaped" or staple design is appropriate. Alternative, creative designs are strongly encouraged.
(6) Hazards. Bicycle parking shall not impede or create a hazard to pedestrians. Parking areas shall be located so as to not conflict with vision clearance standards.
Response: As illustrated on Sheets A0.11-A0.12 in Attachment 6, all required bicycle parking will be provided as interior spaces within each building to comply with the design standards above. This standard will be met.
b. Short-term Bicycle Parking
(1) Provide lockers or racks that meet the standards of this section.
(2) Locate inside or outside the building within thirty (30) feet of the main entrance to the building or at least as close as the nearest vehicle parking space, whichever is closer.
Response: As illustrated on Sheets A0.11-A0.12 in Attachment 6, all required short term bicycle parking will be provided as interior spaces within each building. This standard will be met.

## c. Long-term Bicycle Parking

(1) Provide racks, storage rooms, or lockers in areas that are secure or monitored (e.g., visible to employees or customers or monitored by security guards).
(2) Locate the outside bicycle parking spaces within one hundred (100) feet of the entrance that will be accessed by the intended users.
(3) All of the spaces shall be covered.

Response: As summarized in the table responding to Criterion 1 above, no long-term bicycle parking spaces are required as no building requires more than 8 bicycle spaces. This standard does not apply.
d. Covered Parking (Weather Protection)
(1) When required, covered bicycle parking shall be provided in one (1) of the following ways: inside buildings, under roof overhangs or awnings, in bicycle lockers, or within or under other structures.
(2) Where required covered bicycle parking is not within a building or locker, the cover must be permanent and designed to protect the bicycle from rainfall and provide seven-foot minimum overhead clearance.
(3) Where required bicycle parking is provided in lockers, the lockers shall be securely anchored.

| Table 4-Minimum Required Bicycle Parking Space (Excerpt) |  |
| :--- | :--- |
| Industrial Categories | Minimum Required Spaces |
| Industrial | 2 or 1 per 40 spaces, whichever is greater |

Response: As summarized in the table responding to Criterion 1 above, no long-term bicycle parking spaces are required as no building requires more than 8 bicycle spaces. Since no long-term parking is required, no covered parking is required. However, the applicant proposes to voluntarily provide bicycle parking within the buildings rather than outdoors. This standard does not apply.

### 16.94.030- Off-Street Loading Standards

## A. Minimum Standards

1. A driveway designed for continuous forward flow of passenger vehicles for the purpose of loading and unloading passengers shall be located on the site of any school, or other public meeting place, which is designed to accommodate more than twenty five (25) persons at one time.
2. The minimum loading area for non-residential uses shall not be less than ten (10) feet in width by twenty-five (25) feet in length and shall have an unobstructed height of fourteen (14) feet.
3. Multiple uses on the same parcel or adjacent parcels may utilize the same loading area if it is shown in the development application that the uses will not have substantially overlapping delivery times.
4. The following additional minimum loading space is required for buildings in excess of twenty thousand $(20,000)$ square feet of gross floor area:
a. Twenty thousand $(20,000)$ to fifty $(50,000)$ sq. ft. - five hundred (500) sq. ft.
b. Fifty $(50,000)$ sq. ft. or more - seven hundred fifty $(750)$ sq. ft.

Response: As demonstrated on the site plan and building elevations contained in Attachment 6, the proposed project does not include a school or other public meeting place. Each proposed building contains multiple loading areas well in excess of the 10 -foot-wide, 25 -foot-length, and 1,000 SF minimum for buildings larger than $50,000 \mathrm{SF}$. This standard is met.

## B. Separation of Areas

Any area to be used for the maneuvering of delivery vehicles and the unloading or loading of materials shall be separated from designated off-street parking areas and designed to prevent the encroachment of delivery vehicles onto off-street parking areas or public streets. Off-street parking areas used to fulfill the requirements of this Chapter shall not be used for loading and unloading operations.
Response: As demonstrated on Sheets C3.0 and A0.11-A0.12 in Attachment 6, the proposed project separates off-street parking and off-street loading areas, and no encroachment will occur on public streets. This standard is met.
C. Exceptions and Adjustments.

The review authority, through Site Plan Review, may approve loading areas within a street right-of-way in the Old Town Overlay District when all of the following conditions are met:

1. Short in duration (i.e., less than one (1) hour);
2. Infrequent (less than three (3) operations occur daily between 5:00 a.m. and 12:00 a.m. or all operations occur between 12:00 a.m. and 5:00 a.m. at a location that is not adjacent to a residential zone);
3. Does not unreasonably obstruct traffic; [or] Does not obstruct traffic during peak traffic hours;
4. Does not obstruct a primary emergency response route; and5.Is acceptable to the applicable roadway authority.
Response: The project is not within the Old Town Overlay District. This adjustment does not apply.

## Chapter 16.96-On-Site Circulation

### 16.96.010 - On-Site Pedestrian and Bicycle Circulation

## B. Maintenance

No building permit or other City permit shall be issued until plans for ingress, egress and circulation have been approved by the City. Any change increasing any ingress, egress or circulation requirements, shall be a violation of this Code unless additional facilities are provided in accordance with this Chapter.
Response: The City of Sherwood will review, as part of issuance of site development and building permits, plans that must demonstrate compliance with standards addressing ingress, egress, and circulation. This standard is met.

## C. Joint Access

Two (2) or more uses, structures, or parcels of land may utilize the same ingress and egress when the combined ingress and egress of all uses, structures, or parcels of land satisfied the other requirements of this Code, provided that satisfactory legal evidence is presented to the City in the form of deeds, easements, leases, or contracts to clearly establish the joint use.
Response: The applicant proposes to record reciprocal access and maintenance agreements for the site that will allow unrestricted use of the parking circulation areas. Compliance with this standard can be ensured through review of materials submitted for issuance of site development and building permits. This standard is met.

## D. Connection to Streets

1. Except for joint access per this Section, all ingress and egress to a use or parcel shall connect directly to a public street, excepting alleyways with paved sidewalk.
2. Required private sidewalks shall extend from the ground floor entrances or the ground floor landing of stairs, ramps or elevators to the public sidewalk or curb of the public street which provides required ingress and egress.
Response: Shared access from SW Cipole Place will enable employees, guests, and customers the ability to efficiently travel to and from the site. A network of private walkways is proposed throughout the site to enable safe and convenient pedestrian travel to each of the buildings from public sidewalks along SW Cipole Place and SW 124th Avenue. The entrance of each building is connected to a public sidewalk by an internal private walkway. This standard is met.

## E. Maintenance of Required Improvements

Required ingress, egress and circulation improvements shall be kept clean and in good repair.
Response: Ongoing maintenance of ingress, egress, and circulation will be the responsibility of the property owner(s), as required by these standards.

## F. Access to Major Roadways

Points of ingress or egress to and from Highway 99W and arterials designated on the Transportation Plan Map, attached as Appendix C of the Community Development Plan, Part II, shall be limited as follows:

1. Single and two-family uses and manufactured homes on individual residential lots developed after the effective date of this Code shall not be granted permanent driveway ingress or egress from Highway 99 W and arterial roadways. If alternative public access is not available at the time of development, provisions shall be made for temporary access which shall be discontinued upon the availability of alternative access.
2. Other private ingress or egress from Highway 99W and arterial roadways shall be minimized. Where alternatives to Highway 99W or arterials exist or are proposed, any new or altered uses developed after the effective date of this Code shall be required to use the alternative ingress and egress.
3. All site plans for new development submitted to the City for approval after the effective date of this Code shall show ingress and egress from existing or planned local or collector streets, consistent with the Transportation Plan Map and Section VI of the Community Development Plan.
Response: The proposed site plan includes one point of access to SW Tualatin-Sherwood Road, forming a new south leg at the existing signalized intersection of SW Tualatin-Sherwood Road and SW Cipole Road that will enable full turn movements. An analysis of the proposed public cul-de-sac, SW Cipole Place, is presented in the Traffic Impact Analysis submitted with the application (Attachment 11). The applicant has obtained a Design Exception from Washington County to access SW Tualatin-Sherwood Road with a roadway that is not an arterial or a collector (Attachment 12). This standard is met.

## G. Service Drives

Service drives shall be provided pursuant to Section 16.94.030.
Response: Service drives are discussed in the response to Section 16.94.020.B.4 (Section 16.94.030 does not have standards for service drives).
16.96.030 - Minimum Non-Residential Standards

Minimum standards for private, on-site circulation improvements in non-residential developments:
A. Driveways
2. Industrial: Improved hard surfaced driveways are required as follows:

| Improved Hard Surface Driveway Requirements |  |  |  |
| :---: | :---: | :---: | :---: |
| Parking Spaces | Required <br> \# Driveways | Minimum Width |  |
|  | One-Way Pair | Two -Way |  |
| $1-249$ | 1 | 15 feet | 24 feet |
| 250 and above | 2 | 15 feet | 24 feet |

Response: As illustrated on Sheets C 3.0 and A0.10, all five proposed buildings will have fewer than 250 parking spaces so each building is required to have at least one driveway. The proposed site layout provides access to two driveways for Buildings A through D and one driveway for Building E. This standard is met.
3. Surface materials are encouraged to be pervious when appropriate considering soils, anticipated vehicle usage and other pertinent factors.
Response: All proposed driveways will be hard-surfaced with concrete and asphalt but pervious paving is neither proposed nor required. This standard does not apply.
B. Sidewalks and Curbs

1. A private pathway/sidewalk system extending throughout the development site shall be required to connect to existing development, to public rights-of-way with or without improvements, to parking and storage areas, and to connect all building entrances to one another. The system shall also connect to transit facilities within five hundred (500) feet of the site, future phases of development, and whenever possible to parks and open spaces.
Response: As shown on Attachment 6, a network of internal walkways is proposed to connect each of the buildings with public sidewalks fronting the site, as well as to provide connectivity between buildings within the site. A transit stop, serving TriMet bus route 97, exists adjacent to the intersection of SW Tualatin-Sherwood Road and SW Cipole Road. The development will provide a pedestrian pathway along the proposed cul-de-sac, SW Cipole Place. An additional connection from the private sidewalks to the public sidewalk network is provided to SW 124th Avenue. This standard is met.
2. Curbs shall also be required at a standard approved by the Hearing Authority. Private pathways/sidewalks shall be connected to public rights-of-way along driveways but may be allowed other than along driveways if approved by the Hearing Authority.
Response: Each of the proposed internal walkways will be vertically separated from abutting vehicular parking and circulation areas by a six-inch-tall curb, except where walkways cross through a parking area. This standard is met.
3. Private Pathway/Sidewalk Design. Private pathway surfaces shall be concrete, asphalt, brick/masonry pavers, or other pervious durable surface. Primary pathways connecting front entrances to the right of way shall be at least 6 feet wide and conform to ADA standards. Secondary pathways between buildings and within parking areas shall be a minimum of four (4) feet wide and/or conform to ADA standards. Where the system crosses a parking area, driveway or street, it shall be clearly marked with contrasting paving materials or raised crosswalk (hump). At a minimum all crosswalks shall include painted striping.
Response: Each of the proposed internal walkways will be constructed of concrete. Each of the proposed walkways, regardless of whether they provide a connection to a public sidewalk, is at least six feet wide, as shown on Attachment 6, Sheet C3.0. This standard is met.
4. Exceptions. Private pathways/sidewalks shall not be required where physical or topographic conditions make a connection impracticable, where buildings or other existing development on adjacent lands physically preclude a connection now or in the future considering the potential for redevelopment; or pathways would violate provisions of leases, restrictions or other agreements.
Response: No pathways/sidewalks are proposed southward from the cul-de-sac bulb to the future Blake Road due to the steep difference in elevation between the roadways and the fact that Blake Road has not yet been dedicated as right-of-way or constructed.

### 16.96.040- On-Site Vehicle Circulation

## A. Maintenance

No building permit or other City permit shall be issued until plans for ingress, egress and circulation have been approved by the City. Any change increasing any ingress, egress or circulation requirements, shall be a violation of this Code unless additional facilities are provided in accordance with this Chapter.

Response: The applicant will be required to include plans as part of materials submitted to the City of Sherwood for issuance of site development and building permits that demonstrate compliance with the standard cited above. This standard is met.

## B. Joint Access [See also Chapter 16.108]

Two (2) or more uses, structures, or parcels of land are strongly encouraged to utilize jointly the same ingress and egress when the combined ingress and egress of all uses, structures, or parcels of land satisfy the other requirements of this Code, provided that satisfactory legal evidence is presented to the City in the form of deeds, easements, leases, or contracts to clearly establish the joint use. In some cases, the City may require a joint access to improve safety, vision clearance, site distance, and comply with access spacing standards for the applicable street classification.
Response: As noted above, the applicant proposes to utilize joint access for the site. Reciprocal access and maintenance agreements will be recorded for relevant portions of the site in order to ensure ongoing shared use. This standard is met.

## C. Connection to Streets

1. Except for joint access per this Section, all ingress and egress to a use or parcel shall connect directly to a public street, excepting alleyways.
2. Required private sidewalks shall extend from the ground floor entrances or the ground floor landing of stairs, ramps or elevators to the public sidewalk or curb of the public street which provides required ingress and egress.
Response: As noted above, the applicant proposes to record a reciprocal access and maintenance agreement to allow unrestricted use of the shared vehicular circulation areas. Shared access from the future SW Cipole Place cul-de-sac will enable employees, guests, and customers the ability to efficiently travel to and from the site. A network of private pathways is proposed to connect the entrance of each building to a public sidewalk. This standard is met.

## D. Maintenance of Required Improvements

Required ingress, egress and circulation improvements shall be kept clean and in good repair.
Response: Ongoing maintenance of ingress, egress, and circulation will be the responsibility of the property owner(s), as required by these standards.

## E. Service Drives

Service drives shall be provided pursuant to Section 16.94.030.
Response: Service drives are discussed in the response to Section 16.94.020.B.4 (Section 16.94.030 does not have standards for service drives).

## Chapter 16.98-On-Site Storage

### 16.98.010 - Recreational Vehicles and Equipment

Recreational vehicles and equipment may be stored only within designated and improved off-street parking areas. Such areas shall meet the screening and landscaping requirements of Section 16.92.030.
Response: No recreational vehicles or equipment is anticipated within the proposed development. This standard does not apply.

### 16.98.020 - Solid Waste and Recycling Storage

All uses shall provide solid waste and recycling storage receptacles which are adequately sized to accommodate all solid waste generated on site. All solid waste and recycling storage areas and receptacles shall be located out of public view. Solid waste and recycling receptacles for multi-family, commercial,
industrial and institutional uses shall be screened by six (6) foot high sight-obscuring fence or masonry wall and shall be easily accessible to collection vehicles.
Response: As shown in Attachment 6 Sheets C3.0 and A0.20, the proposed waste and recycling containers will be located in separate enclosures for each building. The waste and recycling service area are located out of public view and will be screened by concrete enclosures with operable gates. No other service areas, such as outdoor storage or mechanical equipment are proposed. This standard is met.

### 16.98.030-Material Storage

A. Generally. Except as otherwise provided herein, external material storage is prohibited, except in commercial and industrial zones where storage areas are approved by the Review Authority as part of a site plan or per Section 16.98.040.
B. Standards. Except as per Section 16.98.040, all service, repair, storage, and merchandise display activities carried on in connection with any commercial or industrial activity, and not conducted within an enclosed building, shall be screened from the view of all adjacent properties and adjacent streets by a six (6) foot to eight (8) foot high, sight obscuring fence subject to chapter 16.58.020. In addition, unless adjacent parcels to the side and rear of the storage area have existing solid evergreen screening or sight-obscuring fencing in place, new evergreen screening no less than three (3) feet in height shall be planted along side and rear property lines. Where other provisions of this Code require evergreen screening, fencing, or a landscaped berm along side and rear property lines, the additional screening stipulated by this Section shall not be required.
C. Hazardous Materials. Storage of hazardous, corrosive, flammable, or explosive materials, if such storage is otherwise permitted by this Code, shall comply with all local fire codes, and Federal and State regulations.
Response: While specific users are not known at this time, no material storage areas are proposed in conjunction with the T-S Corporate Park development. In the event future corporate park tenants or users require material storage, the necessary approval will be requested, and the provisions of this section will be met. Any hazardous materials storage will be permitted with the City and Fire District as required. This standard is met.

### 16.98.040 - Outdoor Sales and Merchandise Display

A. Sales Permitted

Outdoor sales and merchandise display activities, including sales and merchandise display that is located inside when the business is closed but otherwise located outside, shall be permitted when such activities are deemed by the Commission to be a customary and integral part of a permitted commercial or industrial use.

1. Permanent outdoor sales and merchandise display are in use year round or in excess of four (4) months per year and require the location to be reviewed through a site plan review. They will be reviewed as conditional uses in accordance with Chapter 16.82. Permanent outdoor and merchandise display are subject to the standards outlined in subsection B, below.
2. Temporary outdoor sales and merchandise display are seasonal and are not displayed year round and must meet the requirements of Chapter 16.86 (temporary uses). When the temporary use is not occurring the site shall return to its original state.
3. Food vendors including food carts, ice cream trucks, hotdog stands or similar uses are only permitted as a permanent outdoor sale use as described in A. 1 above.
Response: No outdoor sales areas or activities are proposed as part of this development. Sales and display activities by future tenants will be subject to compliance with these requirements. This standard does not apply.
B. Standards
4. Outdoor sales and merchandise display areas shall be kept free of debris. Merchandise shall be stacked or arranged, or within a display structure. Display structures shall be secured and stable.
5. Outdoor sales and merchandise display shall not be located within required yard, building, or landscape setbacks, except where there is intervening right-of-way of a width equal to or greater than the required setback; and shall not interfere with on-site or off-site pedestrian or vehicular circulation.
6. Outdoor retail sales and merchandise display areas for vehicles, boats, manufactured homes, farm equipment, and other similar uses shall be improved with asphalt surfacing, crushed rock, or other dust-free materials.
7. Additional standards may apply to outdoor sales and merchandise display dependent on specific restrictions in the zone.
Response: No outdoor sales and merchandise display is proposed with this development. Sales and display activities by future tenants will be subject to compliance with these requirements. This standard does not apply at this time.

## Chapter 16.100 - Permanent Signs

### 16.100.010 - Common Regulations

A. Sign Permits

1. Except as otherwise provided in this Section and in Chapter 16.102, a person may not construct, install, structurally alter or relocate any sign without first obtaining an administrative sign permit from the City as required by Chapter 16.72, including payment of the fee required by Section 16.74.010. In addition, all permitted illuminated signs are subject to the provisions of the State Electrical Code and any applicable permit fees.
Response: No signage is proposed as part of this application; all signage will be reviewed under a separate permit. This standard is met.

## Division VI. - Public Infrastructure

## Chapter 16.104-General Provisions

16.104.020 - Future Improvements

The location of future public improvements including water, sanitary sewer, storm water, streets, bicycle and pedestrian paths, and other public facilities and rights-of-way, as depicted in the Transportation System Plan (TSP) Chapters 4, 5, 6 and 7 of the Community Development Plan are intended as general locations only. The precise alignment and location of a public improvement shall be established during the land use process and shall be depicted on public improvement plans submitted and approved pursuant to $\S 16.108$ and other applicable sections of this Code.
Response: The civil plans submitted with this application (Attachment 6) depict the proposed alignment and location of public utilities and streets. This standard is met.
16.104.030 - Improvement Procedures

Except as otherwise provided, all public improvements shall conform to City standards and specifications found in the Engineering Design Manual and installed in accordance with Chapter 16.108. The Council may establish additional specifications to supplement the standards of this Code and other applicable ordinances. Except for public projects constructed consistent with an existing facility plan, a public improvements shall not be undertaken until land use approval has been granted, a public improvement
plan review fee has been paid, all improvement plans have been approved by the City, and an improvement permit has been issued.
Response: The civil plans submitted with this application (Attachment 6) depict the preliminary layout of the proposed public utilities and streets, which have been designed to be compliant with City standards. Subsequent review by the City's Engineering staff during the permit phase of the project will ensure compliance with applicable standards. This standard is met.

## Chapter 16.106-Transportation Facilities

### 16.106.010-Generally

A. Creation

Public streets shall be created in accordance with provisions of this Chapter. Except as otherwise provided, all street improvements and rights-of-way shall conform to standards for the City's functional street classification, as shown on the Transportation System Plan (TSP) Map (Figure 17) and other applicable City standards. The following table depicts the guidelines for the street characteristics.

| City Street Characteristic Guidelines (Excerpts) |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Type of Street | Right <br> of Way <br> Width | Number of Lanes | Minimum <br> Lane <br> Width | On <br> Street <br> Parking <br> Width | Bike <br> Lane <br> Width | Sidewalk Width | Landscape Strip <br> (Exclusive <br> of Curb) | Median <br> Width |
| Arterial | 60-120' | 2-5 | 12' | Limited | $6{ }^{\prime}$ | 6-8' | 5' | $14^{\prime} \text { if }$ required |
| $40^{\prime}$ <br> Commercial/ <br> Industrial Not <br> Exceeding <br> 3000 vehicles <br> per day | $64^{\prime}$ | 2 | 20' | 8' | none | $6{ }^{\prime}$ | 5' | none |

Response: The site abuts SW Tualatin-Sherwood Road and SW 124th Avenue, both of which are classified as five-lane arterial roadways under Washington County jurisdiction. As illustrated on Sheets C3.1-C3.6, the applicant proposes to dedicate additional right-of-way along the south side of Tualatin-Sherwood Road as Washington County has a funded project to improve this section of SW Tualatin-Sherwood Road beginning in 2021, the applicant does not propose to improve the project frontage. The applicant proposes to dedicate right-of-way and improve the west side of SW 124th Avenue to arterial standards (Attachment 6, Sheet C3.2) per Washington County standards. As discussed in the Traffic Impact Analysis (Attachment 11), the project is anticipated to generate approximately 1,844 vehicle trips per day. As a result, the proposed cul-de-sac (SW Cipole Place) is proposed to have a 64 -foot right of way with a 40 -foot paved section with two paved lanes (see Attachment 6, Sheet C3.3). These improvements have been designed consistent with the functional classification of each street that is stipulated in the City of Sherwood Transportation System Plan. This standard is met.
B. Street Naming

1. All streets created by subdivision or partition will be named prior to submission of the final plat.
2. Any street created by a public dedication shall be named prior to or upon acceptance of the deed of dedication.
3. An action to name an unnamed street in the City may be initiated by the Council or by a person filing a petition as described in this Section.
4. All streets named shall conform to the general requirements as outlined in this Section.
5. At the request of the owner(s), the City may approve a private street name and address. Private streets are subject to the same street name standards as are public streets. All private street signs will be provided at the owner(s) expense.
Response: The applicant proposes the name Cipole Place for the new cul-de-sac aligned with Cipole Road, consistent with the naming convention identified in criterion C below. This name will be specified on the final plat following review and approval by the City. This standard is met.

## C. Street Name Standards

1. All streets named or renamed shall comply with the following criteria:
a. Major streets and highways shall maintain a common name or number for the entire alignment.
b. Whenever practicable, names as specified in this Section shall be utilized or retained.
c. Hyphenated or exceptionally long names shall be avoided.
d. Similar names such as Farview and Fairview or Salzman and Saltzman shall be avoided.
e. Consideration shall be given to the continuation of the name of a street in another jurisdiction when it is extended into the City.
2. The following classifications (suffixes) shall be utilized in the assignment of all street names:
a. Boulevards: North/south arterials providing through traffic movement across the community.
b. Roads: East/west arterials providing through traffic movement across the community.
c. Avenues: Continuous, north/south collectors or extensions thereof.
d. Streets: Continuous, east-west collectors or extensions thereof.
e. Drives: Curvilinear collectors (less than 180 degrees) at least 1,000 feet in length or more.
f. Lanes: Short east/west local streets under 1,000 feet in length.
g. Terraces: short north/south local streets under 1,000 feet in length.
h. Court: All east/west cul-de-sacs.
i. Place: All north/south cul-de-sacs.
j. Ways: All looped local streets (exceeding 180 degrees).
k. Parkway: A broad landscaped collector or arterial.
3. Except as provided for by this section, no street shall be given a name that is the same as, similar to, or pronounced the same as any other street in the City unless that street is an extension of an already-named street.
4. All proposed street names shall be approved, prior to use, by the City.

Response: Per the preliminary subdivision plan and site plan (Attachment 6 Sheet C8.0) the proposed cul-de-sac will be named SW Cipole Place as it extends southward from existing SW Cipole Road to the north of the site. This standard is met.

## D. Preferred Street Names

Whenever practicable, historical names will be considered in the naming or renaming of public roads. Historical factors to be considered shall include, but not be limited to the following:

1. Original holders of Donation Land Claims in Sherwood.
2. Early homesteaders or settlers of Sherwood.
3. Heirs of original settlers or long-time (50 or more years) residents of Sherwood.
4. Explorers of or having to do with Sherwood.
5. Indian tribes of Washington County.
6. Early leaders and pioneers of eminence.
7. Names related to Sherwood's flora and fauna.
8. Names associated with the Robin Hood legend.

Response: The proposed cul-de-sac will be named SW Cipole Place as the southward extension of existing SW Cipole Road to the north of the site. No new names are proposed. This standard does not apply.

### 16.106.020-Required Improvements

A. Generally

Except as otherwise provided, all developments containing or abutting an existing or proposed street, that is either unimproved or substandard in right-of-way width or improvement, shall dedicate the necessary right-of-way prior to the issuance of building permits and/or complete acceptable improvements prior to issuance of occupancy permits. Right-of-way requirements are based on functional classification of the street network as established in the Transportation System Plan, Figure 17.
Response: As illustrated on Sheet C3.1-C3.6, the applicant proposes to dedicate additional right-of-way along the south side of Tualatin-Sherwood Road to meet the required arterial standard of a minimum 102foot right-of-way ( 51 feet from centerline) for SW Tualatin-Sherwood Road. As Washington County has a funded project to improve Tualatin-Sherwood Road beginning in 2021, the applicant does not propose to improve the project frontage. The applicant proposes to dedicate right-of-way along the west side of 124th Avenue to meet the required arterial standard of a minimum 98 -foot right-of-way ( 49 feet from centerline) for SW 124th Avenue, and to widen the street and add public sidewalk. The applicant proposes to dedicate 64 feet of right-of-way and improve Cipole Place to local street standards for industrial development. These widths fully comply with the street standards identified in the Transportation System Plan. No dedication or improvements are proposed along Blake Road south of the site since the alignment of Blake Road is entirely within the parcel to the south (Parcel 2 of Partition Plat 2019-029). This standard is met.

## B. Existing Streets

Except as otherwise provided, when a development abuts an existing street, the improvements requirement shall apply to that portion of the street right-of-way located between the centerline of the right-of-way and the property line of the lot proposed for development. In no event shall a required street improvement for an existing street exceed a pavement width of thirty (30) feet.
Response: The applicant proposes to dedicate but not improve the south side of Tualatin-Sherwood Road since Washington County has a funded project to improve this section of Tualatin-Sherwood Road beginning in 2021. The applicant proposes to widen 124th Avenue and add public sidewalk. This standard is met.
C. Proposed Streets

1. Except as otherwise provided, when a development includes or abuts a proposed street, in no event shall the required street improvement exceed a pavement width of forty (40) feet.
2. Half Streets: When a half street is created, a minimum of 22 feet of driving surface shall be provided by the developer.
Response: As illustrated on Sheet C3.3, the applicant proposes to dedicate and improve the full width of the Cipole Place roadway, including the cul-de-sac bulb. This standard is met.

## D. Extent of Improvements

1. Streets required pursuant to this Chapter shall be dedicated and improved consistent with Chapter 6 of the Community Development Plan, the TSP and applicable City specifications included in the City of Sherwood Construction Standards. Streets shall include curbs, sidewalks, catch basins, street lights, and street trees. Improvements shall also include any bikeways designated on the Transportation System Plan map. Applicant may be required to dedicate land for required public improvements only when the exaction is directly related to and roughly proportional to the impact of the development, pursuant to Section 16.106.090.

Response: As described above, the applicant proposes to dedicate right-of-way along SW Tualatin-Sherwood Road and SW 124th Avenue to arterial standards per the Transportation System Plan Figure 11 (Motor Vehicle Projects). The applicant does not propose to improve Tualatin-Sherwood Road since Washington County is scheduled to improve the road beginning in 2021. The applicant proposes to improve SW 124th Avenue by widening the roadway to accommodate a bike lane (per TSP Figure 13) and adding a public sidewalk to the west side. These improvements have been designed in accordance with the standards referenced above and will be eligible for System Development Charge credits consistent with City of Sherwood provisions and eligible for Washington County Transportation Development Tax credit consistent with County provisions. This standard is met.
2. If the applicant is required to provide street improvements, the City Engineer may accept a future improvements guarantee in lieu of street improvements if one or more of the following conditions exist, as determined by the City:
a. A partial improvement is not feasible due to the inability to achieve proper design standards;
b. A partial improvement may create a potential safety hazard to motorists or pedestrians.
c. Due to the nature of existing development on adjacent properties it is unlikely that street improvements would be extended in the foreseeable future and the improvement associated with the project under review does not, by itself, provide a significant improvement to street safety or capacity;
d. The improvement would be in conflict with an adopted capital improvement plan;
e. The improvement is associated with an approved land partition on property zoned residential use and the proposed land partition does not create any new streets; or
f. Additional planning work is required to define the appropriate design standards for the street and the application is for a project that would contribute only a minor portion of the anticipated future traffic on the street.
Response: Washington County is scheduled to improve SW Tualatin-Sherwood Road. The applicant has not requested to defer required improvements along SW 124th Avenue or SW Cipole Place. This standard does not apply.
E. Transportation Facilities Modifications

1. A modification to a standard contained within this Chapter and Section 16.58 .010 and the standard cross sections contained in Chapter 8 of the adopted TSP may be granted in accordance with the procedures and criteria set out in this section.
2. A modification request concerns a deviation from the general design standards for public facilities, in this Chapter, Section 16.58.010, or Chapter 8 in the adopted Transportation System Plan. The standards that may be modified include but are not limited to:
a. Reduced sight distances.
b. Vertical alignment.
c. Horizontal alignment.
d. Geometric design (length, width, bulb radius, etc.).
e. Design speed.
f. Crossroads.
g. Access policy.
h. A proposed alternative design which provides a plan superior to these standards.
i. Low impact development.
j. Access Management Plans
3. Modification Procedure
a. A modification shall be proposed with the application for land use approval.
b. A modification is processed as a Type II application. Modification requests shall be processed in conjunction with the underlying development proposal.
c. When a modification is requested to provide a green street element that is not included in the Engineering Design Manual, the modification process will apply, but the modification fee will be waived.
4. Criteria for Modification: Modifications may be granted when criterion 4a and any one of criteria $4 b$ through $4 e$ are met:
a. Consideration shall be given to public safety, durability, cost of maintenance, function, appearance, and other appropriate factors to advance the goals of the adopted Sherwood Comprehensive Plan and Transportation System Plan as a whole. Any modification shall be the minimum necessary to alleviate the hardship or disproportional impact.
b. Topography, right-of-way, existing construction or physical conditions, or other geographic conditions impose an unusual hardship on the applicant, and an equivalent alternative which can accomplish the same design purpose is available.
c. A minor change to a specification or standard is required to address a specific design or construction problem which, if not enacted, will result in an unusual hardship. Self- imposed hardships shall not be used as a reason to grant a modification request.
d. An alternative design is proposed which will provide a plan equal to or superior to the existing street standards.
e. Application of the standards of this chapter to the development would be grossly disproportional to the impacts created.
Response: The applicant has not requested a transportation facility modification (see separate variance request for Cipole Place). This standard does not apply.
16.106.030-Location
A. Generally

The location, width and grade of streets shall be considered in their relation to existing and planned streets, topographical conditions, and proposed land uses. The proposed street system shall provide adequate, convenient and safe traffic and pedestrian circulation, and intersection angles, grades, tangents, and curves shall be adequate for expected traffic volumes. Street alignments shall be consistent with solar access requirements as per Chapter 16.156, and topographical considerations.
Response: Proposed street improvements along the site boundaries will supplement existing streets in order to conform to the corresponding arterial functional classification, as specified in the City of Sherwood Transportation System Plan. Washington County's design of the Tualatin-Sherwood Road
improvements and the applicant's design of the SW 124th Avenue improvements will comply with the criterion cited above to the extent that the corresponding standards achieve the stated characteristics listed above. The proposed site access (SW Cipole Place) has been proposed at a location with an existing traffic signal to minimize access points on Tualatin-Sherwood Road, and the cul-de-sac width has been designed to local commercial/industrial street standards. This standard is met.

## B. Street Connectivity and Future Street Systems

1. Future Street Systems. The arrangement of public streets shall provide for the continuation and establishment of future street systems as shown on the Local Street Connectivity Map contained in the adopted Transportation System Plan (Figure 16).
Response: The Local Street Connectivity Map (Figure 18 of the Transportation System Plan) does not show the extension of any new local streets through the site. Rather, this diagram depicts an arrow for a conceptual street connection, which the TEA Implementation Plan describes by noting that "...we are assuming an internal drive will be located here instead" of an extension of Cipole Road south of Tualatin-Sherwood Road. This standard is met.
2. Connectivity Map Required. New residential, commercial, and mixed-use development involving the construction of new streets shall be submitted with a site plan that implements, responds to and expands on the Local Street Connectivity map contained in the TSP.
a. A project is deemed to be consistent with the Local Street Connectivity map when it provides a street connection in the general vicinity of the connection(s) shown on the map, or where such connection is not practicable due to topography or other physical constraints; it shall provide an alternate connection approved by the decision-maker.
b. Where a developer does not control all of the land that is necessary to complete a planned street connection, the development shall provide for as much of the designated connection as practicable and not prevent the street from continuing in the future.
c. Where a development is disproportionately impacted by a required street connection, or it provides more than its proportionate share of street improvements along property line (i.e., by building more than $3 / 4$ width street), the developer shall be entitled to System Development charge credits, as determined by the City Engineer.
d. Driveways that are more than 24 feet in width shall align with existing streets or planned streets as shown in the Local Street Connectivity Map in the adopted Transportation System Plan (Figure 17), except where prevented by topography, rail lines, freeways, pre-existing development, or leases, easements, or covenants.
Response: No residential, commercial, or mixed uses development is proposed at this time. If these uses are proposed in the future, such a request would be subject to this requirement. This standard does not apply at this time.
3. Block Length. For new streets except arterials, block length shall not exceed 530 feet. The length of blocks adjacent to arterials shall not exceed 1,800 feet.
Response: The block lengths along the site's street frontage have previously been established by the existing street network and by the future Blake Road alignment approved by Partition Plat 2019-029. The block length from SW Tualatin-Sherwood Road to SW 124th Avenue is approximately 1,100 feet; the block length from SW 124th Avenue to SW Cipole Road is approximately 825 feet; and the block length from SW Cipole Road to SW Oregon Street is approximately 1,800 feet. Since both SW Tualatin-Sherwood Road and SW 124th Avenue are
arterials, these lengths are acceptable. The applicant has also submitted an associated Engineering Design Modification request for block length on the future Blake Road (Attachment 22). This standard is met.
4. Where streets must cross water features identified in Title 3 of the Urban Growth Management Functional Plan (UGMFP), provide crossings at an average spacing of 800 to 1,200 feet, unless habitat quality or length of crossing prevents a full street connection.
Response: No Title 3-designated water features are contained within the subject site. This standard does not apply.
5. Where full street connections over water features identified in Title 3 of the UGMFP cannot be constructed in centers, main streets and station communities (including direct connections from adjacent neighborhoods), or spacing of full street crossings exceeds 1,200 feet, provide bicycle and pedestrian crossings at an average spacing of 530 feet, unless exceptional habitat quality or length of crossing prevents a connection.
Response: No Title 3-designated water features are contained within the subject site. This standard does not apply.
6. Pedestrian and Bicycle Connectivity. Paved bike and pedestrian accessways consistent with cross section standards in Figure 8-6 of the TSP shall be provided on public easements or right- of-way when full street connections are not possible, with spacing between connections of no more than 300 feet. Multi-use paths shall be built according to the Pedestrian and Bike Master Plans in the adopted TSP.
Response: The extension of a new street through the site is not required for consistency with the City of Sherwood Transportation System Plan Figure 17 (Street Functional Classification) or Figure 18 (Local Street Connectivity). Figures 12 (Pedestrian Projects) and 13 (Biking Projects) of the Transportation System Plan do not identify any pedestrian or bicycle connectivity projects that affect the site. As discussed in the response to the variance approval criteria in Chapter 16.84, the applicant is requesting a variance to waive the standard for a paved bicycle and pedestrian path south of the cul-de-sac. With the approval of the variance request, this standard is met.
7. Exceptions. Streets, bike, and pedestrian connections need not be constructed when any of the following conditions exists:
a. Physical or topographic conditions make a street or accessway connection impracticable. Such conditions include but are not limited to freeways, railroads, steep slopes, wetlands or other bodies of water where a connection could not reasonably be provided.
b. Buildings or other existing development on adjacent lands physically preclude a connection now or in the future considering the potential for redevelopment; or
c. Where streets or accessways would violate provisions of leases, easements, covenants, restrictions or other agreements existing as of May 1, 1995, which preclude a required street or accessway connection.
Response: As discussed in the response to Chapter 16.84, the applicant is requesting variance approval to construct a cul-de-sac approximately 550 feet long to serve the site. The variance findings detail the reasoning behind the request and identify the physical and topographic conditions (steep slopes) that make it impracticable to extend the roadway further south to the future Blake Road alignment. The site's topography would require steeply sloped pedestrian/bike connections that would be impractical, costly, and potentially dangerous due to the combination of steep slopes and retaining walls needed to configure the site for vehicular access and circulation for industrial use. With the approval of the variance request, this standard is met.
C. Underground Utilities

All public and private underground utilities, including sanitary sewers and storm water drains, shall be constructed prior to the surfacing of streets. Stubs for service connections shall be long enough to avoid disturbing the street improvements when service connections are made.
Response: The applicant understands the need to construct underground utilities in the proper sequence. City Engineering staff will verify this sequence during the permitting phase. This standard is met.

## D. Additional Setbacks

Generally additional setbacks apply when the width of a street right-of-way abutting a development is less than the standard width under the functional classifications in Section VI of the Community Development Plan. Additional setbacks are intended to provide unobstructed area for future street right-of-way dedication and improvements, in conformance with Section VI. Additional setbacks shall be measured at right angles from the centerline of the street.
Response: The applicant proposes to dedicate right-of-way along abutting streets in accordance with arterial standards and has measured setbacks based on the resulting lot lines. This standard is met.

### 16.106.040-Design

Standard cross sections showing street design and pavement dimensions are located in the City of Sherwood's Engineering Design Manual.

## A. Reserve Strips

Reserve strips or street plugs controlling access or extensions to streets are not allowed unless necessary for the protection of the public welfare or of substantial property rights. All reserve strips shall be dedicated to the appropriate jurisdiction that maintains the street.
Response: Reserve strips or street plugs controlling access or extensions to streets are not proposed as part of this development. This standard does not apply.

## B. Alignment

All proposed streets shall, as far as practicable, be in alignment with existing streets. In no case shall the staggering of streets create a "T" intersection or a dangerous condition. Street offsets of less than one hundred (100) feet are not allowed.
Response: The development proposes the 550 -foot cul-de-sac SW Cipole Place as the primary access into the development. Cipole Place will be an extension of the existing street SW Cipole Road north of the proposed development and no staggering of streets is proposed. This standard is met.

## C. Future Extension

Where necessary to access or permit future subdivision or development of adjoining land, streets must extend to the boundary of the proposed development and provide the required roadway width. Dead-end streets less than 100' in length must comply with the Engineering Design Manual. A durable sign must be installed at the applicant's expense. The sign is required to notify the public of the intent to construct future streets. The sign must read as follows: "This road will be extended with future development. For more information contact the City of Sherwood Engineering Department."
Response: A new cul-de-sac is proposed as part of the T-S Corporate Park as the primary access to the development. The proposed street will extend approximately 550 feet into the property serving as access for all five buildings. No further extension is necessary to provide access to parcels to the south as they can take access from the future Blake Road. As a result, no signage regarding road extension is needed. This standard does not apply.

## D. Intersection Angles

Streets shall intersect as near to ninety (90) degree angles as practical, except where topography requires a lesser angle. In all cases, the applicant shall comply with the Engineering Design Manual.
Response: The new cul-de-sac proposed extends southward from the existing SW Cipole Road at a 90degree angle from SW Tualatin-Sherwood Road. This standard is met.

## E. Cul-de-sacs

1. All cul-de-sacs shall be used only when exceptional topographical constraints, existing development patterns, or compliance with other standards in this code preclude a street extension and circulation. A cul-de-sac shall not be more than two hundred (200) feet in length and shall not provide access to more than 25 dwelling units.
2. All cul-de-sacs shall terminate with a turnaround in accordance with the specifications in the Engineering Design Manual. The radius of circular turnarounds may be larger when they contain a landscaped island, parking bay in their center, Tualatin Valley Fire and Rescue submits a written request, or an industrial use requires a larger turnaround for truck access.
3. Public easements, tracts, or right-of-way shall provide paved pedestrian and bicycle access ways at least 6 feet wide where a cul-de-sac or dead-end street is planned, to connect the ends of the streets together, connect to other streets, or connect to other existing or planned developments in accordance with the standards of this Chapter, the TSP, the Engineering Design Manual or other provisions identified in this Code for the preservation of trees.
Response: Cipole Place is proposed as a public cul-de-sac street solely to allow the division of the proposed industrial campus into five lots in a one-building-per-lot configuration. Making a public through street connection to (future) SW Blake Road is impractical primarily due to the site topography. If the applicant chooses not to proceed with the final plat (i.e., to keep the site as one parcel), then the cul-de-sac would be a private roadway with public utility easements. The applicant is requesting a variance to the standard in Subparagraph 1 to allow the proposed cul-de-sac length (approximately 550 feet) to exceed the 200 -foot standard (Attachment 6, Sheet C3.3). The proposed cul-de-sac terminates in a bulb with a paved radius of 54 feet to allow for fire truck turnarounds, consistent with Subparagraph 2. The applicant has also submitted an associated Engineering Design Modification request for cul-de-sac radius (Attachment 21).The applicant is providing private access and is also requesting a variance to the standard in Subparagraph 3 because the site's topography would require steeply sloped pedestrian/bike connections that would be impractical, costly, and potentially dangerous (though pedestrian access is provided from Buildings D and E to 124th Avenue). Justification for the variance request is found in the response to Chapter 16.84. With the approval of the variance request, this standard is met.

## F. Grades and Curves

Grades shall be evaluated by the City Engineer and comply with the Engineering Design Manual.
Response: The applicant proposes to match existing grades on abutting portions of SW Tualatin-Sherwood Road and SW 124th Avenue. The proposed 3\% grade for Cipole Place is depicted on Sheet C3.3 and falls within the ranges specified in the Engineering Design Manual (which limits grades to no more than 15\%). This standard is met.

[^3]consideration shall be given at cross streets for the minimum distance required for future grade separations and to provide sufficient depth to allow screening of the railroad.
Response: This site is not adjacent to a railroad right-of-way. This standard does not apply.

## H. Buffering of Major Streets

Where a development abuts Highway 99W, or an existing or proposed principal arterial, arterial or collector street, or neighborhood route, adequate protection for residential properties must be provided, through and local traffic be separated, and traffic conflicts minimized. In addition, visual corridors pursuant to Section 16.142.040, and all applicable access provisions of Chapter 16.96, are to be met. Buffering may be achieved by: parallel access streets, lots of extra depth abutting the major street with frontage along another street, or other treatment suitable to meet the objectives of this Code.
Response: The site abuts two arterial roadways (SW Tualatin-Sherwood Road and SW 124th Avenue) and a proposed collector (the future Blake Road). However, the proposed development is industrial, not residential, so no residential protection measures are required within the site. Compliance with Section 16.142 .040 and Chapter 16.96 is addressed elsewhere in this narrative. This standard does not apply.

## I. Median Islands

As illustrated in the adopted Transportation System Plan, Chapter 8, median islands may be required on arterial or collector streets for the purpose of controlling access, providing pedestrian safety or for aesthetic purposes.
Response: The site abuts two arterial roadways (SW Tualatin-Sherwood Road and SW 124th Avenue) under Washington County jurisdiction. County staff has not identified the need for median islands in either roadway (medians depicted in the TSP Figure 16A only apply in certain locations). The site also abuts a proposed collector, the future Blake Road that will be under City of Sherwood jurisdiction. Since this will be a two-lane section, no median is required per TSP Figure 16C. This standard does not apply.

## J. Transit Facilities

Development along an existing or proposed transit route, as illustrated in Figure 7-2 in the TSP, is required to provide areas and facilities for bus turnouts, shelters, and other transit-related facilities to Tri-Met specifications. Transit facilities shall also meet the following requirements:

1. Locate buildings within 20 feet of or provide a pedestrian plaza at major transit stops.
2. Provide reasonably direct pedestrian connections between the transit stop and building entrances on the site.
3. Provide a transit passenger landing pad accessible to disabled persons (if not already existing to transit agency standards).
4. Provide an easement or dedication for a passenger shelter and underground utility connection from the new development to the transit amenity if requested by the public transit provider.
5. Provide lighting at a transit stop (if not already existing to transit agency standards).

Response: A transit stop, serving TriMet bus route 97 exists adjacent to the intersection of SW TualatinSherwood Road and SW Cipole Road. Washington County is scheduled to improve SW Tualatin-Sherwood Road beginning in 2021, which will include any transit facilities as needed to satisfy TriMet standards. This standard is met.

## K. Traffic Controls

1. Pursuant to Section 16.106.080, or as otherwise required by the City Engineer, an application must include a traffic impact analysis to determine the number and types of traffic controls necessary to accommodate anticipated traffic flow.
2. For all other proposed developments including commercial, industrial or institutional uses with over an estimated 400 ADT, or as otherwise required by the City Engineer, the application must include a traffic impact analysis to determine the number and types of traffic controls necessary to accommodate anticipated traffic flow.
Response: The required traffic impact analysis (TIA) is included as Attachment 11. The TIA identified the need to modify the existing signal at the intersection of SW Tualatin-Sherwood Road and Cipole Road to accommodate the addition of the proposed south leg, and the County is requiring further coordination with County staff regarding signal design (Attachment 12). This standard is met.

## L. Traffic Calming

1. The following roadway design features, including internal circulation drives, may be required by the City in new construction in areas where traffic calming needs are anticipated:
a. Curb extensions (bulb-outs).
b. Traffic diverters/circles.
c. Alternative paving and painting patterns.
d. Raised crosswalks, speed humps, and pedestrian refuges.
e. Other methods demonstrated as effective through peer reviewed Engineering studies.
2. With approval of the City Engineer, traffic calming measures such as speed humps and additional stop signs can be applied to mitigate traffic operations and/or safety problems on existing streets. They should not be applied with new street construction unless approved by the City Engineer and Tualatin Valley Fire \& Rescue.
Response: As the site is not in a residential neighborhood and the cul-de-sac will preclude opportunities for cut-through traffic, no traffic calming measures are necessary.
M. Vehicular Access Management

All developments shall have legal access to a public road. Access onto public streets shall be permitted upon demonstration of compliance with the provisions of adopted street standards in the Engineering Design Manual.

1. Measurement: See the following access diagram where $R / W=$ Right-of-Way; and P.I. $=$ Point-of-Intersection where P.I. shall be located based upon a 90 degree angle of intersection between ultimate right-of-way lines.
a. Minimum right-of-way radius at intersections shall conform to City standards.
b. All minimum distances stated in the following sections shall be governed by sight distance requirements according to the Engineering Design Manual.
c. All minimum distances stated in the following sections shall be measured to the nearest easement line of the access or edge of travel lane of the access on both sides of the road.
d. All minimum distances between accesses shall be measured from existing or approved accesses on both sides of the road.
e. Minimum spacing between driveways shall be measured from Point " C " to Point " $C$ " as shown below:


Response: SW Cipole Place is proposed as the single connection to SW Tualatin-Sherwood Road, as reviewed and approved by Washington County pursuant to the Design Exception in Attachment 12. All building sites are proposed to have frontage along abutting streets, with all driveways providing access to SW Cipole Place. Measurements have been performed as illustrated in the diagram. This standard is met.

## 2. Roadway Access

No use will be permitted to have direct access to a street or road except as specified below. Access spacing shall be measured from existing or approved accesses on either side of a street or road. The lowest functional classification street available to the legal lot, including alleys within a public easement, shall take precedence for new access points.
a. Local Streets:

Minimum right-of-way radius is fifteen (15) feet. Access will not be permitted within ten (10) feet of Point "B," if no radius exists, access will not be permitted within twenty-five (25) feet of Point "A." Access points near an intersection with a Neighborhood Route, Collector or Arterial shall be located beyond the influence of standing queues of the intersection in accordance with AASHTO standards. This requirement may result in access spacing greater than ten (10) feet.
Response: SW Cipole Place is proposed as a local street. No driveway access points are proposed within 10 feet of intersection radii (Attachment 6, Sheet C3.3) or within the queuing areas identified in the TIA (Attachment 11). This standard is met.

## b. Neighborhood Routes:

Minimum spacing between driveways (Point " $C$ " to Point " $C$ ") shall be fifty (50) feet with the exception of single family residential lots in a recorded subdivision. Such lots shall not be subject to a minimum spacing requirement between driveways (Point " $C$ " to Point " $C$ "). In all instances, access points near an intersection with a Neighborhood Route, Collector or Arterial shall be located beyond the influence of standing queues of the intersection in accordance with AASHTO standards. This requirement may result in access spacing greater than fifty (50) feet.
Response: No access is proposed to a neighborhood route. This standard does not apply.
c. Collectors:

All commercial, industrial and institutional uses with one-hundred-fifty (150) feet or more of frontage will be permitted direct access to a Collector. Uses with less than one-hundred-fifty (150) feet of frontage shall not be permitted direct access to Collectors unless no other alternative exists.

Where joint access is available it shall be used, provided that such use is consistent with Section 16.96.040, Joint Access. No use will be permitted direct access to a Collector within one- hundred (100) feet of any present Point "A." Minimum spacing between driveways (Point "C" to Point "C") shall be one-hundred (100) feet. In all instances, access points near an intersection with a Collector or Arterial shall be located beyond the influence of standing queues of the intersection in accordance with AASHTO standards. This requirement may result in access spacing greater than one hundred (100) feet.
Response: No access is proposed to a collector. This standard does not apply.
d. Arterials and Highway 99W - Points of ingress or egress to and from Highway 99W and arterials designated on the Transportation Plan Map, attached as Figure 1 of the Community Development Plan, Part II, shall be limited as follows:
(1) Single and two-family uses and manufactured homes on individual residential lots developed after the effective date of this Code shall not be granted permanent driveway ingress or egress from Highway 99W or arterials. If alternative public access is not available at the time of development, provisions shall be made for temporary access which shall be discontinued upon the availability of alternative access.
(2) Other private ingress or egress from Highway 99W and arterial roadways shall be minimized. Where alternatives to Highway 99W or arterials exist or are proposed, any new or altered uses developed after the effective date of this Code shall be required to use the alternative ingress and egress. Alternatives include shared or crossover access agreement between properties, consolidated access points, or frontage or backage roads. When alternatives do not exist, access shall comply with the following standards:
(a) Access to Highway 99W shall be consistent with ODOT standards and policies per OAR 734, Division 51, as follows: Direct access to an arterial or principal arterial will be permitted provided that Point 'A' of such access is more than six hundred (600) feet from any intersection Point 'A' or other access to that arterial (Point 'C').
(b) The access to Highway 99 W will be considered temporary until an alternative access to public right-of-ways is created. When the alternative access is available the temporary access to Highway 99W shall be closed.
(3) All site plans for new development submitted to the City for approval after the effective date of this Code shall show ingress and egress from existing or planned local, neighborhood route or collector streets, including frontage or backage roads, consistent with the Transportation Plan Map and Chapter 6 of the Community Development Plan.
Response: No access is proposed to SW 124th Avenue, and access to SW TualatinSherwood Road has been coordinated with Washington County, the roadway jurisdiction. A single access is proposed at an existing signalized intersection (SW Cipole Road). Washington County has approved a Design Exception to allow a local street (SW Cipole Place) to access an arterial (SW Tualatin-Sherwood Road). This standard is met.

## 3. Exceptions to Access Criteria for City-Owned Streets

a. Alternate points of access may be allowed if an access management plan which maintains the classified function and integrity of the applicable facility is submitted to and approved by the City Engineer as the access management plan must be included as part of the land use submittal or an application for modification as described in § 16.106.020 E. (Transportation Facilities Modifications).
b. Access in the Old Town (OT) Overlay Zone

Access points in the OT Overlay Zone shown in an adopted plan such as the Transportation System Plan, are not subject to the access spacing standards and do not need a variance. However, the applicant shall submit a partial access management plan for approval by the City Engineer. The approved plan shall be implemented as a condition of development approval.
Response: The applicant is not proposing an access management plan and the site is not in the Old Town Overlay Zone. This standard does not apply.
N. Private Streets

1. The construction of a private street serving a single-family residential development is prohibited unless it provides principal access to two or fewer residential lots or parcels (i.e. flag lots).
2. Provisions shall be made to assure private responsibility for future access and maintenance through recorded easements. Unless otherwise specifically authorized, a private street shall comply with the same standards as a public street identified in the Community Development Code and the Transportation System Plan.
3. A private street shall be distinguished from public streets and reservations or restrictions relating to the private street shall be described in land division documents and deed records.
4. A private street shall also be signed differently from public streets and include the words "Private Street".
Response: No private streets are anticipated as part of the T-S Corporate Park, as SW Cipole Place is proposed to be a public roadway. If the applicant opts to lease all the buildings rather than subdividing and selling individual lots, then the applicant may not record the final subdivision plat. In that case, a private driveway would be proposed in lieu of a public street but no maintenance agreement would be needed since the driveway would be on a single lot. This standard does not apply.

### 16.106.060 - Sidewalks

A. Required Improvements

1. Except as otherwise provided, sidewalks shall be installed on both sides of a public street and in any special pedestrian way within new development.
2. For Highway 99W, arterials, or in special industrial districts, the City Manager or designee may approve a development without sidewalks if alternative pedestrian routes are available.
3. In the case of approved cul-de-sacs serving less than fifteen (15) dwelling units, sidewalks on one side only may be approved by the City Manager or designee.
Response: The site currently has approximately 500 feet of developed frontage along the SW TualatinSherwood Road. Sidewalks appear to be five feet wide. As Washington County has a funded project to improve this section of Tualatin-Sherwood Road beginning in 2021, the applicant does not propose to improve the project frontage. The proposed development will be responsible for sidewalk improvements along the remaining frontage of SW 124th Avenue. The applicant proposes sidewalks on SW Cipole Place . This standard is met.
B. Design Standards
4. Arterial and Collector Streets

Arterial and collector streets shall have minimum six (6) or eight (8) foot wide sidewalks/multi-use paths, located as required by this Code. Residential areas shall have a minimum of a six (6) foot wide sidewalk and commercial industrial areas shall have a minimum of an eight (8) foot wide sidewalk.
2. Local Streets

Local streets shall have minimum five (5) foot wide sidewalks, located as required by this Code.
3. Handicapped Ramps

Sidewalk handicapped ramps shall be provided at all intersections.
Response: Washington County is designing the proposed improvements to SW Tualatin-Sherwood Road. As illustrated on Attachment 6 Sheet C3.6, a six-foot sidewalk is proposed on SW 124th Avenue (an arterial) and a six-foot sidewalk is proposed on SW Cipole Place (a local street). To minimize impacts on wetlands east of the cul-de-sac, the applicant proposes a sidewalk on the west side of SW Cipole Place but not on the east side north of the cul-de-sac bulb. Handicapped ramps are proposed at the intersection of SW Tualatin-Sherwood Road and SW Cipole Place (Attachment 6 Sheet C3.3). This standard is met.
C. Pedestrian and Bicycle Paths

Provide bike and pedestrian connections on public easements or right-of-way when full street connections are not possible, with spacing between connections of no more than 330 feet except where prevented by topography, barriers such as railroads or highways, or environmental constraints such as rivers and streams.
Response: The applicant has requested a variance to the corresponding standard in 16.106.040.E.3 because the site's topography would require steeply sloped pedestrian/bike connections that would be impractical, costly, and potentially dangerous due to the combination of steep slopes and retaining walls needed to configure the site for vehicular access and circulation for industrial use. Justification for the variance request is found in the response to Chapter 16.84 . With the approval of the variance request, this standard is met.

### 16.106.070 - Bike Lanes

If shown in Figure 13 of the Transportation System Plan, bicycle lanes shall be installed in public rights-ofway, in accordance with City specifications. Bike lanes shall be installed on both sides of designated roads, should be separated from the road by a twelve-inch stripe or other means approved by Engineering Staff, and should be a minimum of five (5) feet wide.
Response: Figure 13 (Biking Projects) of the TSP illustrates the existing bike lane on SW Tualatin-Sherwood Road and the proposed bike lane on SW 124th Avenue. Washington County will install the bike lane in SW Tualatin-Sherwood Road as part of their construction project beginning in 2021, and the applicant proposes a bike lane on SW 124th Avenue (Attachment 6 Sheets C3.2 and C3.6). This standard is met.
16.106.080 - Traffic Impact Analysis (TIA)
B. Applicability

A traffic impact analysis (TIA) shall be required to be submitted to the City with a land use application at the request of the City Engineer or if the proposal is expected to involve one (1) or more of the following:

1. An amendment to the Sherwood Comprehensive Plan or zoning map.
2. A new direct property approach road to Highway $99 W$ is proposed.
3. The proposed development generates fifty (50) or more PM peak-hour trips on Highway 99W, or one hundred (100) PM peak-hour trips on the local transportation system.
4. An increase in use of any adjacent street or direct property approach road to Highway 99 W by ten (10) vehicles or more per day that exceed the twenty thousand-pound gross vehicle weight.
5. The location of an existing or proposed access driveway does not meet minimum spacing or sight distance requirements, or is located where vehicles entering or leaving the property are restricted, or such vehicles are likely to queue or hesitate at an approach or access connection, thereby creating a safety hazard.6.A change in internal traffic patterns that may cause safety problems, such as back up onto the highway or traffic crashes in the approach area.
Response: The project is anticipated to generate on the order of 1,844 vehicle trips per day. A Traffic Impact Analysis has been provided as Attachment 11. This standard is met.

## C. Requirements

The following are typical requirements that may be modified in coordination with Engineering Staff based on the specific application.

1. Pre-application Conference. The applicant shall meet with the City Engineer prior to submitting an application that requires a TIA. This meeting will be coordinated with Washington County and ODOT when an approach road to a County road or Highway 99W serves the property, so that the TIA will meet the requirements of all relevant agencies.
2. Preparation. The TIA shall be prepared by an Oregon Registered Professional Engineer qualified to perform traffic Engineering analysis and will be paid for by the applicant.
3. Typical Average Daily Trips and Peak Hour Trips. The latest edition of the Trip Generation Manual, published by the Institute of Transportation Engineers (ITE), shall be used to gauge PM peak hour vehicle trips, unless a specific trip generation study that is approved by the City Engineer indicates an alternative trip generation rate is appropriate.
4. Intersection-level Analysis. Intersection-level analysis shall occur at every intersection where the analysis shows that fifty (50) or more peak hour vehicle trips can be expected to result from the development.
5. Transportation Planning Rule Compliance. The requirements of OAR 660-012-0060 shall apply to those land use actions that significantly affect the transportation system, as defined by the Transportation Planning Rule.
Response: The applicant's transportation consultant has coordinated with both City Engineering staff and Washington County staff to identify the applicable requirements for the TIA and has provided the appropriate data and analysis in the TIA. The TIA scoping memo is Appendix A in Attachment 11. This standard is met.
D. Study Area

The following facilities shall be included in the study area for all TIAs:

1. All site-access points and intersections (signalized and unsignalized) adjacent to the proposed development site. If the site fronts an arterial or collector street, the analysis shall address all intersections and driveways along the site frontage and within the access spacing distances extending out from the boundary of the site frontage.
2. Roads and streets through and adjacent to the site.
3. All intersections needed for signal progression analysis.
4. In addition to these requirements, the City Engineer may require analysis of any additional intersections or roadway links that may be adversely affected as a result of the proposed development.

Response: The applicant's transportation consultant has coordinated with both City Engineering staff and Washington County staff to identify the appropriate study area and has evaluated the operations of the affected intersections in the TIA (Attachment 11). This standard is met.

## E. Analysis Periods

To adequately assess the impacts of a proposed land use action, the following study periods, or horizon years, should be addressed in the transportation impact analysis where applicable:

1. Existing Year.
2. Background Conditions in Project Completion Year. The conditions in the year in which the proposed land use action will be completed and occupied, but without the expected traffic from the proposed land use action. This analysis should account for all City-approved developments that are expected to be fully built out in the proposed land use action horizon year, as well as all planned transportation system improvements.
3. Full Buildout Conditions in Project Completion Year. The background condition plus traffic from the proposed land use action assuming full build-out and occupancy.
4. Phased Years of Completion. If the project involves construction or occupancy in phases, the applicant shall assess the expected roadway and intersection conditions resulting from major development phases. Phased years of analysis will be determined in coordination with City staff.
5. Twenty-Year or TSP Horizon Year. For planned unit developments, comprehensive plan amendments or zoning map amendments, the applicant shall assess the expected future roadway, intersection, and land use conditions as compared to approved comprehensive planning documents.
Response: The TIA analyzes existing traffic operations and forecasts operations in 2021 (prior to the County's improvements to SW Tualatin-Sherwood Road) and in 2025 (following the County's improvements to SW Tualatin-Sherwood Road). No phasing is proposed as part of the development, and no planned unit development, comprehensive plan amendment, or zoning map amendment is proposed. This standard is met.

## F. Approval Criteria

When a TIA is required, a proposal is subject to the following criteria, in addition to all criteria otherwise applicable to the underlying land use proposal:

1. The analysis complies with the requirements of 16.106.080.C;
2. The analysis demonstrates that adequate transportation facilities exist to serve the proposed development or identifies mitigation measures that resolve identified traffic safety problems in a manner that is satisfactory to the City Engineer and, when County or State highway facilities are affected, to Washington County and ODOT;
3. For affected non-highway facilities, the TIA demonstrates that mobility and other applicable performance standards established in the adopted City TSP have been met; and
4. Proposed public improvements are designed and will be constructed to the street standards specified in Section 16.106.010 and the Engineering Design Manual, and to the access standards in Section 16.106.040.
5. Proposed public improvements and mitigation measures will provide safe connections across adjacent right-of-way (e.g., protected crossings) when pedestrian or bicycle facilities are present or planned on the far side of the right-of-way.
Response: Kittelson \& Associates transportation engineers projected site trip generation (Attachment 11) based on Land Use Code 130 - Industrial Park within the Institute of Transportation Engineers' (ITE) Trip Generation Manual, 10th edition. The report analyzed traffic operations in the vicinity in the years 2021 and 2025, both with and without the proposed development:

- In 2021, the SW Oregon Street/SW Tualatin-Sherwood Road intersection is expected to exceed mobility standards in the PM peak hour with or without the proposed development.
- In 2021, the proposed development would cause the SW Oregon Street/SW Tonquin Road intersection to exceed mobility standards in the PM peak hour.
- In 2021, all other intersections in the study area are anticipated to meet mobility standards in both the AM and PM peak hours.
- In 2025 (following Washington County's planned improvements to SW Tualatin-Sherwood Road), the SW Oregon Street/SW Tonquin Road intersection is expected to exceed mobility standards in the PM peak hour with or without the proposed development.
- In 2025, all other intersections in the study area are anticipated to meet mobility standards in both the AM and PM peak hours.

The TIA also analyzed traffic operations depending on whether Cipole Road is extended to Blake Road or not, concluding that "there appears to be no significant system-wide benefit to extending SW Cipole Road through the site to connect with the future Blake Road" and points out potential roadway conflicts if Cipole Road were extended south to Blake Road.

## G. Conditions of Approval

The City may deny, approve, or approve a development proposal with conditions needed to meet operations and safety standards and provide the necessary right-of-way and improvements to ensure consistency with the future planned transportation system. Improvements required as a condition of development approval, when not voluntarily provided by the applicant, shall be roughly proportional to the impact of the development on transportation facilities, pursuant to Section 16.106.090. Findings in the development approval shall indicate how the required improvements are directly related to and are roughly proportional to the impact of development.
Response: The TIA recommends providing a proportionate cost share allocation towards the future conversion of the SW Oregon Street/SW Tonquin Road intersection either to a roundabout or signalized intersection. A condition of approval to that effect would be appropriate to mitigate for the traffic impacts of the proposed development.

### 16.106.090 - Rough Proportionality

A. Purpose

The purpose of this section is to ensure that required transportation facility improvements are roughly proportional to the potential impacts of the proposed development. The rough proportionality requirements of this section apply to both frontage and non-frontage improvements. A proportionality analysis will be conducted by the City Engineer for any proposed development that triggers transportation facility improvements pursuant to this chapter. The City Engineer will take into consideration any benefits that are estimated to accrue to the development property as a result of any required transportation facility improvements. A proportionality determination can be appealed pursuant to Chapter 16.76. The following general provisions apply whenever a proportionality analysis is conducted.
B. Mitigation of impacts due to increased demand for transportation facilities associated with the proposed development shall be provided in rough proportion to the transportation impacts of the proposed development. When applicable, anticipated impacts will be determined by the TIA in accordance with Section 16.106.080. When no TIA is required, anticipated impacts will be determined by the City Engineer.
C. The following shall be considered when determining proportional improvements:

1. Condition and capacity of existing facilities within the impact area in relation to City standards. The impact area is generally defined as the area within a one-half-mile radius of the proposed development. If a TIA is required, the impact area is the TIA study area.
2. Existing vehicle, bicycle, pedestrian, and transit use within the impact area.
3. The effect of increased demand on transportation facilities and other approved, but not yet constructed, development projects within the impact area that is associated with the proposed development.
4. Applicable TSP goals, policies, and plans.
5. Whether any route affected by increased transportation demand within the impact area is listed in any City program including school trip safety; neighborhood traffic management; capital improvement; system development improvement, or others.
6. Accident history within the impact area.
7. Potential increased safety risks to transportation facility users, including pedestrians and cyclists.
8. Potential benefit the development property will receive as a result of the construction of any required transportation facility improvements.
9. Other considerations as may be identified in the review process pursuant to Chapter 16.72. Response: To ensure rough proportionality, the TIA recommends providing a proportionate cost share allocation towards the future conversion of the SW Oregon Street/SW Tonquin Road intersection either to a roundabout or signalized intersection and includes the proportionate share percentage computations based on traffic volumes. This standard is met.

## Chapter 16.110-Sanitary Sewers

### 16.110.010-Required Improvements

Sanitary sewers shall be installed to serve all new developments and shall connect to existing sanitary sewer mains. Provided, however, that when impractical to immediately connect to a trunk sewer system, the use of septic tanks may be approved, if sealed sewer laterals are installed for future connection and the temporary system meets all other applicable City, Clean Water Services, Washington County and State sewage disposal standards.
Response: As depicted in Attachment 6 Sheets C6.3-C6.6, the proposed development will construct a public sanitary sewer line in Tualatin-Sherwood Road from Oregon Street eastward to the Cipole Road intersection, and then southward in Cipole Place to serve the development. Private sanitary sewer laterals will be constructed from the buildings to the new public line in Cipole Place. Sheets C6.0-C6.2 show all proposed connections. South of Cipole Place, sewer service will be extended southward to the future location of Blake Road to provide sewer service to the future water treatment plant south of Blake Road; this line would be in a public utility easement. This standard is met.
16.110.020 - Design Standards
A. Capacity

Sanitary sewers shall be constructed, located, sized, and installed at standards consistent with this Code, the Sanitary Sewer Service Plan Map in the Sanitary Sewer Master Plan, and other applicable Clean Water Services and City standards, in order to adequately serve the proposed development and allow for future extensions.
Response: Compliance with the standards of this code is demonstrated in this narrative and in sheets C6.0-C6.3 of Attachment 6. The sanitary sewer plan was designed in accordance with the Sanitary Sewer Service Plan Map in the Sanitary Sewer Master Plan and has been reviewed at a conceptual level with the City Engineer. Further demonstration of compliance with applicable standards will take place during the permitting phase of the project. This standard is met.
B. Over-Sizing

1. When sewer facilities will, without further construction, directly serve property outside a proposed development, gradual reimbursement may be used to equitably distribute the cost of that over-sized system.
2. Reimbursement shall be in an amount estimated by the City to be a proportionate share of the cost for each connection made to the sewer by property owners outside of the development, for a period of ten (10) years from the time of installation of the sewers. The boundary of the reimbursement area and the method of determining proportionate shares shall be determined by the City. Reimbursement shall only be made as additional connections are made and shall be collected as a surcharge in addition to normal connection charges.
Response: As illustrated in Attachment 6 Sheets C6.0-C6.6, the proposed development will construct a public sanitary sewer line in Tualatin-Sherwood Road from Oregon Street eastward to the Cipole Road intersection, and then southward in Cipole Place to serve the development. South of Cipole Place, sewer service will be extended southward to the future location of Blake Road. This line has been over-sized to accommodate anticipated discharge from the future water treatment plant south of Blake Road. If the applicant chooses to seek reimbursement for oversizing the lines, a formal request will be filed with the City. This standard is met.

### 16.110.030 - Service Availability

Approval of construction plans for new facilities pursuant to Chapter 16.106, and the issuance of building permits for new development to be served by existing sewer systems shall include certification by the City that existing or proposed sewer facilities are adequate to serve the development.
Response: Issuance of a service availability certification by the City shall occur through review and approval of plans for public improvements, which will be submitted to the City for issuance of the required permits subsequent to receiving necessary land use approvals. This standard is met.

## Chapter 16.112 - Water Supply

### 16.112.010-Required Improvements

Water lines and fire hydrants conforming to City and Fire District standards shall be installed to serve all building sites in a proposed development. All waterlines shall be connected to existing water mains or shall construct new mains appropriately sized and located in accordance with the Water System Master Plan.
Response: As shown in the sanitary and water utilities plans (Sheets C6.0-C6.2 in Attachment 6), a new 16 -inch diameter public water line will be extended into the site from an existing 12 -inch line located in SW Cipole Road (to be constructed by the Willamette Water Supply Program). The applicant also proposes private water loops through the west and east sides of the site for fire-fighting. The water line loops through the site will be in 10 -foot-wide public utility easements. There will be separate water line taps to each of the buildings for a fire water vault and a domestic water service.

All the proposed buildings will be provided with separate water meters and private service lines. Fire hydrants and water lines were designed in conformance with City and Fire District standards. This standard is met.

### 16.112.020 - Design Standards

A. Capacity

Water lines providing potable water supply shall be sized, constructed, located and installed at standards consistent with this Code, the Water System Master Plan, the City's Design and Construction Manual, and with other applicable City standards and specifications, in order to adequately serve the proposed development and allow for future extensions.

Response: The sanitary and water utilities plans (Sheets C6.0-C6.2 in Attachment 6) were designed to be consistent with the City of Sherwood Code, the Water System Master Plan, the City's Design and Construction Manual, and with other applicable City standards. Further demonstration of compliance with applicable standards will take place during the permitting phase of the project. This standard is met.

## B. Fire Protection

All new development shall comply with the fire protection requirements of Chapter 16.116, the applicable portions of Chapter 7 of the Community Development Plan, and the Fire District.
Response: The proposed development has been designed to comply with requirements of Chapter 16.116, the applicable portions of Chapter 7 of the Community Development Plan, and Fire District standards. New fire hydrants are proposed internal to the site and spaced to provide necessary coverage for fire apparatus response. All new buildings constructed at the site will include automatic fire suppression systems. This standard is met.
C. Over-Sizing

1. When water mains will, without further construction, directly serve property outside a proposed development, gradual reimbursement may be used to equitably distribute the cost of that over-sized system.
2. Reimbursement shall be in an amount estimated by the City to be the proportionate share of the cost of each connection made to the water mains by property owners outside the development, for a period of ten (10) years from the time of installation of the mains. The boundary of the reimbursement area and the method of determining proportionate shares shall be determined by the City. Reimbursement shall only be made as additional connections are made and shall be collected as a surcharge in addition to normal connection charges.
3. When over-sizing is required in accordance with the Water System Master Plan, it shall be installed per the Water System Master Plan. Compensation for over-sizing may be provided through direct reimbursement, from the City, after mainlines have been accepted. Reimbursement of this nature would be utilized when the cost of over-sizing is for system wide improvements.
Response: As illustrated in Attachment 6 Sheets C6.0-C6.2, the Willamette Water Supply Program will construct two public water mains extending southward from Tualatin-Sherwood Road to the future location of Blake Road. These lines have been over-sized to accommodate anticipated water usage for the future water treatment plant south of Blake Road. If the Willamette Water Supply Program chooses to seek reimbursement for oversizing the lines, a formal request will be filed with the City. This standard is met.

### 16.112.030 - Service Availability

Approval of construction plans for new water facilities pursuant to Chapter 16.106, and the issuance of building permits for new development to be served by existing water systems shall include certification by the City that existing or proposed water systems are adequate to serve the development.
Response: Issuance of a service availability certification by the City shall occur through review and approval of plans for public improvements, which will be submitted to the City for issuance of the required permits subsequent to receiving necessary land use approvals. This standard is met.

## Chapter 16.114-Storm Water

Storm water facilities, including appropriate source control and conveyance facilities, shall be installed in new developments and shall connect to the existing downstream drainage systems consistent with the Comprehensive Plan and the requirements of the Clean Water Services water quality regulations contained in their Design and Construction Standards R\&O 04-9, or its replacement.

Response: The stormwater plans (Sheets C5.0-C5.2 in Attachment 6) show how the proposed development will manage stormwater from the site. New water quality and detention facilities in stormwater tracts are proposed to manage run-off in a manner consistent with applicable Clean Water Services design standards. This criterion is met.

### 16.114.020 - Design Standards

A. Capacity

Storm water drainage systems shall be sized, constructed, located, and installed at standards consistent with this Code, the Storm Drainage Master Plan Map, attached as Exhibit E, Chapter 7 of the Community Development Plan, other applicable City standards, the Clean Water Services Design and Construction standards R\&O 04-9 or its replacement, and hydrologic data and improvement plans submitted by the developer.
B. On-Site Source Control

Storm water detention and groundwater recharge improvements, including but not limited to such facilities as dry wells, detention ponds, and roof top ponds shall be constructed according to Clean Water Services Design and Construction Standards.
C. Conveyance System

The size, capacity and location of storm water sewers and other storm water conveyance improvements shall be adequate to serve the development and accommodate upstream and downstream flow. If an upstream area discharges through the property proposed for development, the drainage system shall provide capacity to the receive storm water discharge from the upstream area. If downstream drainage systems are not sufficient to receive an increase in storm water caused by new development, provisions shall be made by the developer to increase the downstream capacity or to provide detention such that the new development will not increase the storm water caused by the new development.
Response: New water quality and detention facilities are proposed to manage run-off from the site in a manner consistent with applicable Clean Water Services standards. No upstream discharges flow through the site, and no off-site downstream facilities are proposed to be used to manage runoff from the site. The preliminary storm report (Attachment 16) demonstrates feasibility for the proposed stormwater management system. This standard is met.

### 16.114.030 - Service Availability

Approval of construction plans for new storm water drainage facilities pursuant to Chapter 16.106, and the issuance of building permits for new development to be served by existing storm water drainage systems shall include certification by the City that existing or proposed drainage facilities are adequate to serve the development.
Response: Issuance of a service availability certification by the City shall occur through review and approval of plans for public improvements, which will be submitted to the City for issuance of the required permits subsequent to receiving necessary land use approvals. This standard is met.

## Chapter 16.116 - Fire Protection

### 16.116.010-Required Improvements

When land is developed so that any commercial or industrial structure is further than two hundred and fifty (250) feet or any residential structure is further than five hundred (500) feet from an adequate water supply for fire protection, as determined by the Fire District, the developer shall provide fire protection facilities necessary to provide adequate water supply and fire safety.
Response: The Willamette Water Supply Program proposes to construct a 16 -inch public water main in Cipole Place and additional private water line loops through the site will provide an adequate supply for
the proposed fire protection hydrants (Sheets C6.0-C6.2 in Attachment 6). Each water line is within 250 feet of the proposed buildings. This standard is met.

### 16.116.020 - Standards

A. Capacity

All fire protection facilities shall be approved by and meet the specifications of the Fire District, and shall be sized, constructed, located, and installed consistent with this Code, Chapter 7 of the Community Development Plan, and other applicable City standards, in order to adequately protect life and property in the proposed development.
Response: Multiple new fire hydrants are proposed to serve the development. All fire protection facilities were designed in compliance with the City of Sherwood Development Code, Chapter 7 of the Community Development Plan, and other applicable City standards. Compliance with these standards is demonstrated on Sheets C6.0-C6.2 of Attachment 6. Further demonstration of compliance with applicable standards will take place during the permitting phase of the project. This standard is met.

## B. Fire Flow

Standards published by the Insurance Services Office, entitled "Guide for Determination of Required Fire Flows" shall determine the capacity of facilities required to furnish an adequate fire flow. Fire protection facilities shall be adequate to convey quantities of water, as determined by ISO standards, to any outlet in the system, at no less than twenty (20) pounds per square inch residual pressure. Water supply for fire protection purposes shall be restricted to that available from the City water system. The location of hydrants shall be taken into account in determining whether an adequate water supply exists.
Response: Fire flow tests and hydraulic modeling will be performed during the permitting phase of the project to demonstrate compliance with this standard. The future water mains illustrated on Sheets $6.0-$ C6.2 of Attachment 6 are anticipated to provide more than adequate fire flow. This standard is met.

## C. Access to Facilities

Whenever any hydrant or other appurtenance for use by the Fire District is required by this Chapter, adequate ingress and egress shall be provided. Access shall be in the form of an improved, permanently maintained roadway or open paved area, or any combination thereof, designed, constructed, and at all times maintained, to be clear and unobstructed. Widths, height clearances, ingress and egress shall be adequate for District firefighting equipment. The Fire District, may further prohibit vehicular parking along private accessways in order to keep them clear and unobstructed, and cause notice to that effect to be posted.
Response: All new fire hydrants on site will be easily accessible by District firefighting equipment. The sanitary and water utilities plans (Sheets C6.0-C6.2 in Attachment 6) show the location of and access routes for all new fire hydrants. As shown in the plans, all hydrants will be located adjacent to paved roads or drive aisles, which will remain unobstructed to provide adequate width, height clearance, and ingress and egress to allow for the maneuvering of District firefighting equipment. Vehicle parking areas on site will not obstruct the movement of firefighting equipment. This standard is met.

## D. Hydrants

Hydrants located along private, accessways shall either have curbs painted yellow or otherwise marked prohibiting parking for a distance of at least fifteen (15) feet in either direction, or where curbs do not exist, markings shall be painted on the pavement, or signs erected, or both, given notice that parking is prohibited for at least fifteen (15) feet in either direction.
Response: There are multiple proposed hydrants internal to the site in and around buildings and parking areas. However, as the hydrants are not located on private drive aisles no curb markings or signage is merited. This standard does not apply.

### 16.116.030 - Miscellaneous Requirements

A. Timing of Installation

When fire protection facilities are required, such facilities shall be installed and made serviceable prior to or at the time any combustible construction begins on the land unless, in the opinion of the Fire District, the nature or circumstances of said construction makes immediate installation impractical.
B. Maintenance of Facilities

All on-site fire protection facilities, shall be maintained in good working order. The Fire District may conduct periodic tests and inspection of fire protection and may order the necessary repairs or changes be made within ten (10) days.
C. Modification of Facilities

On-site fire protection facilities, may be altered or repaired with the consent of the Fire District; provided that such alteration or repairs shall be carried out in conformity with the provisions of this Chapter.
Response: These standards are understood and will be the responsibility of the applicant to uphold. These standards are met.

## Chapter 16.118 - Public and Private Utilities

### 16.118.020-Standard

A. Installation of utilities shall be provided in public utility easements and shall be sized, constructed, located and installed consistent with this Code, and applicable utility company and City standards.
Response: This proposed development requires public utility easements for the public storm lines and for the utilities extending southward from Cipole Place to the future Blake Road. These easements are shown in the utility plans (Sheets C6.0-C6.2 in Attachment 6) and the preliminary plat Sheet C8.0 in Attachment 6. The easements were designed in compliance with the City of Sherwood Development Code, Chapter 7 of the Community Development Code, and applicable utility company and City standards. Further demonstration of compliance with applicable standards will take place during the permitting phase of the project. This standard is met.
B. Public utility easements shall be a minimum of eight (8) feet in width unless a reduced width is specifically exempted by the City Engineer. An eight-foot wide public utility easement (PUE) shall be provided on private property along all public street frontages. This standard does not apply to developments within the Old Town Overlay.
Response: This proposed development requires public utility easements for the public storm lines and for the utilities extending southward from Cipole Place to the future Blake Road. These easements are shown in the utility plans (Sheets C6.0-C6.2 in Attachment 6) and the preliminary plat Sheet C8.0 in Attachment 6. Additionally, an eight-foot public utility easement will be provided along the west side of Cipole Place. The easements were designed in compliance with the City of Sherwood Development Code, Chapter 7 of the Community Development Code, and applicable utility company and City standards. Further demonstration of compliance with applicable standards will take place during the permitting phase of the project. This standard is met.
C. Where necessary, in the judgment of the City Manager or his designee, to provide for orderly development of adjacent properties, public and franchise utilities shall be extended through the site to the edge of adjacent property(ies).
Response: The applicant proposes to provide a utility corridor south from Cipole Place to the future Blake Road to accommodate development to the south. This standard is met.
D. Franchise utility conduits shall be installed per the utility design and specification standards of the utility agency.
Response: The applicant will provide any needed conduits for franchise utilities during construction, as will be further verified during the permitting phase of the project. This standard is met.
E. Public Telecommunication conduits and appurtenances shall be installed per the City of Sherwood telecommunication design standards.
Response: The applicant will provide any needed conduits for franchise utilities during construction, as will be further verified during the permitting phase of the project. This standard is met.
F. Exceptions: Installation shall not be required if the development does not require any other street improvements. In those instances, the developer shall pay a fee in lieu that will finance installation when street or utility improvements in that location occur.
Response: All applicable public and private utilities requirements will be met through this proposal. No exceptions to this section are requested. This standard does not apply.

### 16.118.030-Underground Facilities

Except as otherwise provided, all utility facilities, including but not limited to, electric power, telephone, natural gas, lighting, cable television, and telecommunication cable, shall be placed underground, unless specifically authorized for above ground installation, because the points of connection to existing utilities make underground installation impractical, or for other reasons deemed acceptable by the City.
Response: All proposed utilities will be constructed underground as required. Further demonstration of compliance with applicable standards will take place during the permitting phase of the project. This standard is met.

### 16.118.040-Exceptions

Surface-mounted transformers, surface-mounted connection boxes and meter cabinets, temporary utility service facilities during construction, high capacity electric and communication feeder lines, and utility transmission lines operating at fifty thousand $(50,000)$ volts or more may be located above ground. The City reserves the right to approve location of all surface-mounted transformers.
Response: It is anticipated that the development will require some or all of these above-ground utility facilities, as will be further coordinated with Engineering staff during the permitting phase of the project. This standard is met.

### 16.118.050 - Private Streets

The construction of new private streets, serving single-family residential developments shall be prohibited unless it provides principal access to two or fewer residential lots or parcels i.e. flag lots. Provisions shall be made to assure private responsibility for future access and maintenance through recorded easements. Unless otherwise specifically authorized, a private street shall comply with the same standards as a public street identified in the Community Development Code and the Transportation System Plan. A private street shall be distinguished from public streets and reservations or restrictions relating to the private street shall be described in land division documents and deed records. A private street shall also be signed differently from public streets and include the words "Private Street".
Response: No private streets are anticipated as part of the T-S Corporate Park, as SW Cipole Place is proposed to be a public roadway. If the applicant opts to lease all the buildings rather than subdividing and selling individual lots, then the applicant may not record the final subdivision plat. In that case, a private driveway would be proposed in lieu of a public street. This standard does not apply.

## Division VII. - Land Divisions, Subdivisions, Partitions, Lot Line Adjustments and Modifications

## Chapter 16.120-Subdivisions

### 16.120.020 - General Subdivision Provisions

A. Approval of a subdivision occurs through a two-step process: the preliminary plat and the final plat.

1. The preliminary plat shall be approved by the Approval Authority before the final plat can be submitted for approval consideration; and
2. The final plat shall reflect all conditions of approval of the preliminary plat.

Response: The applicant is requesting approval of the preliminary plat (Sheet C8.0) as part of this application. Following approval of the preliminary plat and construction of required infrastructure, the applicant will submit a separate application for the final plat. This standard is met.
B. All subdivision proposals shall conform to all state regulations set forth in ORS Chapter 92, Subdivisions and Partitions.
Response: The preliminary subdivision plat is included as Sheet C8.0, and the final plat will be prepared by a licensed Oregon surveyor in accordance with the requirements of ORS 92 and Washington County Surveyor standards. This standard is met.

## C. Future re-division

When subdividing tracts into large lots, the Approval Authority shall require that the lots be of such size and shape as to facilitate future re-division in accordance with the requirements of the zoning district and this Division.
Response: This provision is more applicable to residential subdivisions than to large-lot industrial developments. The proposed lot sizes range from 3.71 to 9.0 acres and have not been over-sized in anticipation of future re-division. This standard does not apply.

## D. Future Partitioning

When subdividing tracts into large lots which may be resubdivided, the City shall require that the lots be of a size and shape, and apply additional building site restrictions, to allow for the subsequent division of any parcel into lots of smaller size and the creation and extension of future streets.
Response: This provision is more applicable to residential subdivisions than to large-lot industrial developments. The proposed lot sizes range from 3.71 to 9.0 acres and have not been over-sized in anticipation of future re-division. This standard does not apply.

## E. Lot averaging

Lot size may be averaged to allow lots less than the minimum lot size allowed in the underlying zoning district subject to the following regulations:

1. The average lot area for all lots is not less than allowed by the underlying zoning district.
2. No lot created under this provision shall be less than $90 \%$ of the minimum lot size allowed in the underlying zoning district.
3. The maximum lot size cannot be greater than $10 \%$ of the minimum lot size.

Response: No lot averaging is proposed. This standard does not apply.
F. Required Setbacks

All required building setback lines as established by this Code, shall be shown in the preliminary subdivision plat.
Response: Proposed building setbacks are illustrated on Sheets A0.11 and A0.12. This standard is met.

## G. Property Sales

No property shall be disposed of, transferred, or sold until required subdivision approvals are obtained, pursuant to this Code.
Response: The development is speculative and there are no specific users to whom the proposed lots would be sold prior to recording of the final plat. This standard is met.

### 16.120.030 - Approval Procedure-Preliminary Plat

A. Approval Authority

1. The approving authority for preliminary and final plats of subdivisions shall be in accordance with Section 16.72.010 of this Code.
A. A subdivision application for 4-10 lots will follow a Type II review process.
b. A subdivision application for 11-50 lots will follow a Type III review process.
c. A subdivision application for over 50 lots will follow a Type IV review process.
2. Approval of subdivisions is required in accordance with this Code before a plat for any such subdivision may be filed or recorded with County. Appeals to a decision may be filed pursuant to Chapter 16.76.
Response: The applicant is requesting subdivision approval for five lots. Therefore, the request is subject to the Type II review process. This standard is met.
B. Phased Development
3. The Approval Authority may approve a time schedule for developing a subdivision in phases, but in no case shall the actual construction time period for any phase be greater than two years without reapplying for a preliminary plat.
4. The criteria for approving a phased subdivision review proposal are:
a. The public facilities shall be scheduled to be constructed in conjunction with or prior to each phase to ensure provision of public facilities prior to building occupancy;
b. The development and occupancy of any phase shall not be dependent on the use of temporary public facilities:
(1) For purposes of this subsection, a temporary public facility is an interim facility not constructed to the applicable City or district standard; and
(2) The phased development shall not result in requiring the City or other property owners to construct public facilities that were required as a part of the approval of the preliminary plat.
5. The application for phased development approval shall be reviewed concurrently with the preliminary plat application and the decision may be appealed in the same manner as the preliminary plat.
Response: No phased subdivision is proposed, all lots will be created on a single final plat. This standard does not apply.
16.120.040 - Approval Criteria: Preliminary Plat

No preliminary plat shall be approved unless:
A. Streets and roads conform to plats approved for adjoining properties as to widths, alignments, grades, and other standards, unless the City determines that the public interest is served by modifying streets or road patterns.
Response: Additional right-of-way dedication along the north and east site boundaries will be provided as part of this development to accommodate turn lanes on SW Tualatin-Sherwood Road and SW 124th Avenue. The creation of the Cipole Place cul-de-sac utilizes the intersection location and alignment where Cipole Road currently intersects Tualatin-Sherwood Road. The partition plat which created this parcel
(Partition Plat 2019-029) established the location and alignment for the future Blake Road, which will be dedicated and constructed when the property to the south develops. This standard is met.
B. Streets and roads held for private use are clearly indicated on the plat and all reservations or restrictions relating to such private roads and streets are set forth thereon.
Response: No private roads are proposed as part of the subdivision (though if the applicant does not proceed with the final plat, Cipole Place would be a shared private driveway). This standard does not apply.
C. The plat complies with applicable zoning district standards and design standards in Division II, and all provisions of Divisions IV, VI, VIII and IX. The subdivision complies with Chapter 16.128 (Land Division Design Standards).
Response: Findings that demonstrate compliance with the applicable development standards from Divisions IV, VI, and VIII are presented herein and Division IX does not apply as there are no historic resources on site. This standard is met.
D. Adequate water, sanitary sewer, and other public facilities exist to support the use of land proposed in the plat.
Response: As illustrated in Attachment 6 Sheets C5.0-C6.6, public water and sanitary sewer lines will be available to serve the site and the applicant will construct stormwater management facilities to serve the site. Further detail is provided in the responses to Division VI. This standard is met.
E. Development of additional, contiguous property under the same ownership can be accomplished in accordance with this Code.
Response: The Willamette Water Supply System Commission owns both parcels within Partition Plat 2019-029, including the subject site (Parcel 1 of the plat) and the property to the south (Parcel 2 of the plat) which is planned for a water treatment facility. Parcel 2 can develop independently in the future, taking access from the future Blake Road and connecting to utilities to be constructed through this proposed development. A public utility easement is proposed from the southern terminus of Cipole Place to the future alignment of Blake Road. This standard is met.
F. Adjoining land can either be developed independently or is provided access that will allow development in accordance with this Code.
Response: The Willamette Water Supply System Commission owns both parcels within Partition Plat 2019-029, including the subject site (Parcel 1 of the plat) and the property to the south (Parcel 2 of the plat) which is planned for a water treatment facility. Parcel 2 can develop independently in the future, taking access from the future Blake Road and connecting to utilities to be constructed through this proposed development. Properties to the west, north, and east have access to existing roadways (Dahlke Lane to the west, Tualatin-Sherwood Road to the north, and 124th Avenue to the east). This standard is met.
G. $\quad$ Tree and woodland inventories have been submitted and approved as per Section 16.142.060.

Response: Existing conditions (including trees and woodlands) are depicted on Attachment 6 Sheet C2.0, with further tree detail provided on Sheets C2.1 and C2.2. The tree canopy limits illustrated in the civil plans show the location of woodlands. The arborist report (Attachment 18) provides a tree inventory in areas where trees are proposed to be preserved and along the site boundaries. This standard is met.
H. The plat clearly shows the proposed lot numbers, setbacks, dedications and easements.

Response: Attachment 6 Sheet C8.0 is the preliminary plat which includes the proposed lot numbers, dedications, and easements, while setbacks are depicted on Sheet A0.11-A0.12. This standard is met.
I. A minimum of five percent (5\%) open space has been provided per Section 16.44.010.B.8 (Townhome-Standards) or Section 16.142.030 (Parks, Open Spaces and Trees-Single-Family Residential Subdivisions), if applicable.
Response: The proposed development is wholly industrial and not a residential development. This standard does not apply.

### 16.120.050 - Final Subdivision Plat

A. Procedure

1. Unless otherwise noted below, final subdivision approval includes meeting all conditions from the land use approval, review and approval by County, and the signature of the City's designee on the mylar.
2. The subdivider shall submit the final plat, and all supplementary information required by the Planning Department or pursuant to this Code.
3. Upon approval of the final plat drawing, the applicant may submit the mylar for final signature.
4. All requirements for signature of the mylar shall be completed within two (2) years of approval of the final plat.
B. Extensions

If the final plat is not approved within two (2) years, the preliminary plat approval shall expire and a new plat must be submitted. However, the City may, upon written request by the applicant, grant a single extension up to one (1) year upon a written finding that the facts upon which approval was based have not changed to an extent sufficient to warrant refiling of the preliminary plat and that no other development approval would be affected. For preliminary plat approvals granted between January 1, 2007 and December 31, 2009, the approval shall be extended until December 31, 2013.
C. Approval Criteria: Final Plat

By means of a Type I procedure, the City shall review the final plat based on findings regarding compliance with the following criteria:

1. The final plat is consistent in design (e.g., number and dimensions of lots, easements, tracts, right-of-way) with the approved preliminary plat, and all conditions of approval have been satisfied;
2. All public improvements required by the preliminary plat have been installed and approved by the City Engineer or appropriate service provider ( e.g., road authority). Alternatively, the developer has provided a performance guarantee in accordance with § 16.120.070.
3. The streets and roads for public use are dedicated without reservation or restriction other than reversionary rights upon vacation of any such street or road and easements for public utilities;
4. The plat and deed contain a dedication to the public of all public improvements, including but not limited to streets, public pathways and trails, access reserve strips, parks, sewage disposal, storm drainage and water supply systems;
5. The applicant has provided copies of all recorded homeowners association Covenants, Conditions and Restrictions (CC\&R's); deed restrictions; private easements and agreements (e.g., for access, common areas, parking, etc.); and other recorded documents pertaining to common improvements recorded and referenced on the plat;
6. The plat complies with the applicable Sections of this code (i.e., there have been no changes in land use or development resulting in a code violation since preliminary plat approval);
7. Certification by the City or service district, as applicable, that water and sanitary sewer service is available to every lot depicted on the plat; or bond, contract or other assurance
has been provided by the subdivider/partitioner to the City that such services will be installed in accordance Division VI of this Code, and the bond requirements of 16.120.070. The amount of the bond, contract or other assurance by the subdivider/partitioner shall be determined by a registered professional engineer, subject to review and approval by the City;
8. The plat contains an affidavit by the surveyor who surveyed the land, represented on the plat to the effect the land was correctly surveyed and marked with proper monuments as provided by ORS Chapter 92, indicating the initial point of the survey, and giving the dimensions and kind of such monument and its reference to some corner established by the U.S. Geological Survey, or giving two or more permanent objects for identifying its location.
Response: The current application is for preliminary subdivision approval. The final plat submittal will occur via separate application. This standard does not apply at this stage in the development process.

### 16.120.060 - Improvement Agreement

A. Subdivision Agreement

The subdivider shall either install required improvements and repair existing streets and other public facilities damaged in the development of the subdivision pursuant to the Division VI, or execute and file with the City an agreement specifying the period within which all required improvements and repairs shall be completed, and providing that if such work is not completed within the period specified, the City may complete the same and recover the full cost and expense thereof from the subdivider. Such agreement may also provide for the construction of the improvements in stages.
B. Performance Security

The subdivider is required to provide monetary assurance of full and faithful performance in the form of a bond, cash, or other security acceptable to the City in an amount equal to one hundred twenty-five percent (125\%) of the estimated cost of the improvements.
Response: The applicant will either complete all required public improvements and repairs prior to recording the final plat or will sign an agreement with the City outlining the proposed construction schedule. The applicant will provide financial security as required. This standard is met.

### 16.120.070 - Bond

A. Performance guarantee required. As required by Section 16.120.060, the subdivider shall file with the agreement an assurance of performance supported by one of the following:

1. A surety bond executed by a surety company authorized to transact business in the state of Oregon which remains in force until the surety company is notified by the City in writing that it may be terminated or cash.
2. Determination of sum. The assurance of performance shall be for a sum determined by the City Engineer as required to cover the cost of the improvements and repairs, including related engineering and incidental expenses.
3. Itemized improvement estimate. The subdivider shall furnish to the City Engineer an itemized improvement estimate, certified by a registered civil engineer, to assist the City Engineer in calculating the amount of the performance assurance.
4. When subdivider fails to perform. In the event the subdivider fails to carry out all provisions of the agreement and the City has un-reimbursed costs or expenses resulting from such failure, the City shall call on the bond, cash deposit for reimbursement.
5. Termination of performance guarantee. The subdivider shall not cause termination of nor allow expiration of said guarantee without having first secured written authorization from the City.

Response: The applicant will provide the required performance bond in a form acceptable to the City, with the amount to be determined based on the cost associated with constructing public improvements. This standard is met.

### 16.120.080 - Filing and Recording of Final Subdivision Plat

A. County Review

When the City determines that the plat conforms to all requirements, the plat shall be authorized for review by the County.
B. Recording the Plat

After approval, the City shall authorize the transmittal of the final map, tracing, and other data to the County, to determine that there has been compliance with all provisions of State and local statutes. Approval of the final plat shall be null and void if the plat is not recorded within sixty (60) days after the date of the last required approving signatures have been obtained.
C. Effective Date

Subdivision approval shall become final upon the recording with the County of the approved subdivision plat or partition map together with any required documents. Development permits may be issued only after final approval, except for activities at the preliminary plat phase, specifically authorized by this Code.
Response: The current application is only for preliminary subdivision approval. The filing and recording of the final subdivision plat will occur via separate application. This standard does not apply at this time.

## Chapter 16.128-Land Division Design Standards

### 16.128.010-Blocks

A. Connectivity

1. Block Size

The length, width, and shape of blocks shall be designed to provide adequate building sites for the uses proposed, and for convenient access, circulation, traffic control and safety.
2. Block Length

Block length standards shall be in accordance with Section 16.108.040. Generally, blocks shall not exceed five-hundred thirty (530) feet in length, except blocks adjacent to principal arterial, which shall not exceed one thousand eight hundred $(1,800)$ feet. The extension of streets and the formation of blocks shall conform to the Local Street Network map contained in the Transportation System Plan.
3. Pedestrian and Bicycle Connectivity. Paved bike and pedestrian accessways shall be provided on public easements or right-of-way consistent with Figure 7.401.
B. Utilities Easements for sewers, drainage, water mains, electric lines, or other utilities shall be dedicated or provided for by deed. Easements shall be a minimum of ten (10) feet in width and centered on rear or side lot lines; except for tie-back easements, which shall be six (6) feet wide by twenty (20) feet long on side lot lines at the change of direction.
C. Drainages

Where a subdivision is traversed by a watercourse, drainage way, channel or street, drainage easements or rights-of-way shall be provided conforming substantially to the alignment and size of the drainage.
Response: The enclosed site plans illustrate that the site is of satisfactory dimensions to allow development of industrial uses. The block lengths along the site's street frontage have previously been established by the existing street network and by the future Blake Road alignment approved by Partition Plat 2019-029. The block length from SW Tualatin-Sherwood Road to SW 124th Avenue is approximately 1,100 feet; the block length from SW 124th Avenue to SW Cipole Road is approximately 825 feet; and the block length from SW Cipole Road to SW Oregon Street is approximately 1,800 feet. Since both SW

Tualatin-Sherwood Road and SW 124th Avenue are arterials, these lengths are acceptable. The applicant has also submitted an associated Engineering Design Modification request for block length on the future Blake Road (Attachment 22). Proposed public utility easements are depicted on Attachment 6 Sheet C8.0. There are no watercourses that need to be accommodated on site. This standard is met.

### 16.128.020-Pedestrian and Bicycle Ways

Pedestrian or bicycle ways may be required to connect cul-de-sacs, divide through an unusually long or oddly shaped block, or to otherwise provide adequate circulation.
Response: The applicant proposes on-site private pedestrian connections between building entrances and the public right-of-way. The extension of a new street through the site is not required for consistency with the City of Sherwood Transportation System Plan Figure 17 (Street Functional Classification) or Figure 18 (Local Street Connectivity). Figures 12 (Pedestrian Projects) and 13 (Biking Projects) of the Transportation System Plan do not identify any pedestrian or bicycle connectivity projects that affect the site. As discussed in the response to the variance approval criteria in Chapter 16.84, the applicant is requesting a variance to waive the standard for a paved bicycle and pedestrian path south of the cul-de-sac. With the approval of the variance request, this standard is met.

### 16.128.030-Lots

A. Size and Shape

Lot size, width, shape, and orientation shall be appropriate for the location and topography of the subdivision or partition, and shall comply with applicable zoning district requirements, with the following exception:

1. Lots in areas not served by public sewer or water supply shall conform to any special County Health Department standards.
Response: As previously outlined, the proposed subdivision will meet the dimensional standards of section 16.31.030 - Development Standards within the Employment Industrial zone. Public sewer and water supply currently exist and will be extended at the applicant's expense. Utility connections from the proposed industrial buildings will be developed throughout the property and connect to the City's utility infrastructure. This standard is met.
B. Access

All lots in a subdivision shall abut a public street, except as allowed for infill development under Chapter 16.68.
Response: The industrial development includes the subdivision of the property into five lots. With the development of SW Cipole Place, all lots except Lot 1 will have frontage along the proposed cul-de-sac. Lot 1 is positioned along SW Tualatin-Sherwood Road while Lots 4 and 5 will have additional frontage along SW 124th Avenue. This standard is met.

## C. Double Frontage

Double frontage and reversed frontage lots are prohibited except where essential to provide separation of residential development from railroads, traffic arteries, adjacent nonresidential uses, or to overcome specific topographical or orientation problems. A five (5) foot wide or greater easement for planting and screening may be required.
Response: No double frontage lots are proposed. While Lots 4 and 5 each have frontage on SW 124th Avenue and SW Cipole Place, the frontage on Cipole Place is limited (short) due to its location on the cul-de-sac bulb. This standard is met.
D. Side Lot Lines

Side lot lines shall, as far as practicable, run at right angles to the street upon which the lots face, except that on curved streets side lot lines shall be radial to the curve of the street.

Response: As illustrated on Attachment 6 Sheet C8.0, the proposed side lot lines are as perpendicular to the street as can be accommodated by the site geometry. This standard is met.

## E. Grading

Grading of building sites shall conform to the following standards, except when topography of physical conditions warrants special exceptions:

1. Cut slopes shall not exceed one (1) and one-half (1 1/2) feet horizontally to one (1) foot vertically.
2. Fill slopes shall not exceed two (2) feet horizontally to one (1) foot vertically.

Response: As detailed in the geotechnical reports (part of Attachment 16), portions of the site are underlain by bedrock, some of which will be exposed as part of the site grading process. Cut slopes in rock will be approximately $0.5: 1$, as depicted on Attachment 6 Sheet C4.0. In areas with no rock cuts, cut slopes will be at 3:1 and fill slopes are proposed at 3:1 (Sheets C4.1-C4.2). This standard is met.

## Division VIII. - Environmental Resources

## Chapter 16.136 - Procedures

### 16.136.010-Applicability

The standards of this Chapter, and applicable portions of Chapter 5 of the Community Development Plan, shall apply to any new uses or changes to existing uses in commercial, industrial and institutional zones, except as per Section 16.136.050.
Response: The applicant requests that the City Manager waive the standards of this chapter per the Exceptions in Section 16.136.050. As the buildings are speculative, determination of compliance would be more appropriately deferred to the time of tenant improvements when specific users are known.

### 16.136.020 - Conformance

Conformance with the standards of this Chapter shall, at a minimum, be certified in writing by a professional engineer and submitted with the application for site plan review required by Chapter 16.90, except as per Section 16.136.050. The written certification shall include:
A. Statement certifying that the proposed commercial, industrial or institutional use, if properly managed and operated, will comply with City environmental performance standards, and citing evidence supporting the certification.
B. Copies of any applicable State permits or recent test results, if available, which would indicate compliance with City environmental performance standards.
Response: The applicant requests that the City Manager waive the standards of this chapter per the Exceptions in Section 16.136.050. As the buildings are speculative, determination of compliance would be more appropriately deferred to the time of tenant improvements when specific users are known.

### 16.136.030-Additional Information

A. Prior to accepting any land use application to which this Chapter applies, the City Manager or his or her designee, may determine that additional expertise in evaluating the application, due to the complexity of its impact on environmental resources, is warranted. Under such circumstances, the City may contract with a professional engineer or other qualified consultant to evaluate and make recommendations on specific application elements relative to City environmental resource standards.
B. Upon the City's determination that additional expertise is needed, the applicant shall deposit a sum equal to the estimated cost, as determined by the City, of such professional services. If the actual cost of such services is more than estimated, the applicant shall be responsible for the
difference, provided however, that the applicant's financial responsibilities will not exceed ten percent $(10 \%)$ of the estimate without prior written authorization. If the cost of such services is less than the estimate, the balance of the deposit shall be returned to the applicant upon final action on their land use application.
Response: The applicant requests that the City Manager waive the standards of this chapter per the Exceptions in Section 16.136.050. As the buildings are speculative, determination of compliance would be more appropriately deferred to the time of tenant improvements when specific users are known.

### 16.136.040 - Referenced Statutes and Rules

The Federal, State or regional statutes and rules cited in this Chapter are made part of this Code by reference. The statutes and rules cited are as current at the time of adoption of this Code. If a referenced statute or rule is amended by Federal, State or regional agencies, this Code must be amended for the new statute or rule to take precedence.
Response: The applicant acknowledges the requirement with applicable environmental standards promulgated by agencies other than the City. The applicant requests that the City Manager waive the standards of this chapter per the Exceptions in Section 16.136.050. As the buildings are speculative, determination of compliance would be more appropriately deferred to the time of tenant improvements when specific users are known.

### 16.136.050-Exceptions

The City shall make an initial determination whether a proposed development is subject to any of the standards of this Chapter, or whether the development is exempt. The City Manager or his or her designee is authorized to waive all or some of these standards when a proposed development clearly does not represent a substantial impact on the City's environmental resource standards as per this Chapter. The findings of the City Manager or his or her designee shall be made in writing, and copies shall be forwarded to the applicant and the Commission. The action of the City Manager or his or her designee may be appealed as per Chapter 16.76.
Response: The applicant requests that the City Manager waive the standards of this chapter per the Exceptions in Section 16.136.050. As the buildings are speculative, determination of compliance would be more appropriately deferred to the time of tenant improvements when specific users are known.

## Chapter 16.142 - Parks, Trees and Open Spaces

### 16.142.040 - Visual Corridors

A. Corridors Required New developments located outside of the Old Town Overlay with frontage on Highway 99W, or arterial or collector streets designated on Figure 8-1 of the Transportation System Plan shall be required to establish a landscaped visual corridor according to the following standards:

| Landscape Visual Corridor Standards |  |  |
| :--- | :---: | :--- |
|  | Category | Width |
| 1. | Highway 99W | 25 Feet |
| 2. | Arterial | 15 Feet |
| 3. | Collector | 10 Feet |

In residential developments where fences are typically desired adjoining the above described major street the corridor may be placed in the road right-of-way between the property line and
the sidewalk. In all other developments, the visual corridor shall be on private property adjacent to the right-of-way.
Response: The proposed landscape design will provide a 15 -foot-wide landscaped Visual Corridor along SW Tualatin-Sherwood Road and SW 124th Avenue (Attachment 6 Sheets L1.11 and L1.15). Both buffers are entirely located within the boundaries of the site. This criterion is met.

## B. Landscape Materials

The required visual corridor areas shall be planted as specified by the review authority to provide a continuous visual and/or acoustical buffer between major streets and developed uses. Except as provided for above, fences and walls shall not be substituted for landscaping within the visual corridor. Uniformly planted, drought resistant street trees and ground cover, as specified in Section 16.142.060, shall be planted in the corridor by the developer. The improvements shall be included in the compliance agreement. In no case shall trees be removed from the required visual corridor.
Response: Tree, shrub, and groundcover species proposed within the Visual Corridor buffers have been selected and placed to comply with the standard cited above, as shown on Attachment 6, Sheets L1.11 and L1.15. No fences are proposed within the visual corridors, and the only proposed walls are retaining walls stemming from the site topography. This standard is met.

## C. Establishment and Maintenance

Designated visual corridors shall be established as a portion of landscaping requirements pursuant to Chapter 16.92. To assure continuous maintenance of the visual corridors, the review authority may require that the development rights to the corridor areas be dedicated to the City or that restrictive covenants be recorded prior to the issuance of a building permit.
Response: The applicant acknowledges this standard. As of the date of this application, the City has not requested dedication of the Visual Corridors as public property. Ongoing maintenance of the Visual Corridors will be the responsibility of the property owners and building tenants. This standard is met.
D. Required Yard

Visual corridors may be established in required yards, except that where the required visual corridor width exceeds the required yard width, the visual corridor requirement shall take precedence. In no case shall buildings be sited within the required visual corridor, with the exception of front porches on townhomes, as permitted in Section 16.44.010(E)(4)(c).
Response: The required Visual Corridor widths along SW Tualatin-Sherwood Road and SW 124th Avenue ( 15 feet) are smaller than the corresponding minimum setbacks of the El zone (20 feet). No proposed buildings are located within either Visual Corridor. This standard is met.
E. Pacific Highway 99W Visual Corridor

1. Provide a landscape plan for the highway median paralleling the subject frontage. In order to assure continuity, appropriate plant materials and spacing, the plan shall be coordinated with the City Planning Department and ODOT.
2. Provide a visual corridor landscape plan with a variety of trees and shrubs. Fifty percent (50\%) of the visual corridor plant materials shall consist of groupings of at least five (5) native evergreen trees a minimum of ten (10) feet in height each, spaced no less than fifty (50) feet apart, if feasible. Deciduous trees shall be a minimum of four (4) inches DBH and twelve (12) feet high, spaced no less than twenty-five (25) feet apart, if feasible.
Response: The proposed development is not located along Pacific Highway 99W. This standard does not apply.
16.142.050 - Park Reservation

Areas designated on the Natural Resources and Recreation Plan Map, in Chapter 5 of the Community Development Plan, which have not been dedicated pursuant to Section 16.142.030 or 16.134.020, may be required to be reserved upon the recommendation of the City Parks Board, for purchase by the City within a period of time not to exceed three (3) years.
Response: The Comprehensive Plan's Natural Resources and Recreation Map does not include the subject site as the map pre-dates inclusion of the site within the urban growth boundary or city limits. More recent Metro data illustrates limited upland habitat along the southern portion of the property. However, as this site has previously been identified for industrial development, it would be inconsistent with the Comprehensive Plan for the City to purchase that portion of the site for park or recreation purposes. This standard is not applicable.

### 16.142.060 - Street Trees

A. Installation of Street Trees on New or Redeveloped Property.

Trees are required to be planted to the following specifications along public streets abutting or within any new development or re-development. Planting of such trees shall be a condition of development approval. The City shall be subject to the same standards for any developments involving City-owned property, or when constructing or reconstructing City streets. After installing street trees, the property owner shall be responsible for maintaining the street trees on the owner's property or within the right-of-way adjacent to the owner's property.

1. Location: Trees shall be planted within the planter strip along a newly created or improved streets. In the event that a planter strip is not required or available, the trees shall be planted on private property within the front yard setback area or within public street right-of-way between front property lines and street curb lines or as required by the City.
Response: As shown on Attachment 6, new street trees are proposed along the SW TualatinSherwood Road and SW 124th Avenue frontages of the subject site, and along Cipole Place. Installation will occur either within new planter strips or behind the public sidewalk and within the front setback area. This standard is met.
2. Size: Trees shall have a minimum trunk diameter of two (2) caliper inches, which is measured six inches above the soil line, and a minimum height of six (6) feet when planted.
3. Types: Developments shall include a variety of street trees. The trees planted shall be chosen from those listed in 16.142.080 of this Code.
Response: Selected species of street trees are consistent with the adopted listed contained in Section 16.142.090, as shown on Attachment 6. Two-inch caliper street trees will be installed in conjunction with site development. These standards are met.
4. Required Street Trees and Spacing:
a. The minimum spacing is based on the maximum canopy spread identified in the recommended street tree list in section 16.142.080 with the intent of providing a continuous canopy without openings between the trees. For example, if a tree has a canopy of forty (40) feet, the spacing between trees is forty (40) feet. If the tree is not on the list, the mature canopy width must be provided to the planning department by a certified arborist.
Response: Selected street tree species have been spaced consistent with the specifications contained in Section 16.142.090, as shown on Attachment 6. This standard is met.
b. All new developments shall provide adequate tree planting along all public streets. The number and spacing of trees shall be determined based on the type of tree and the spacing standards described in a. above and considering driveways, street light locations and utility connections. Unless exempt per c. below, trees shall not be spaced more than forty (40) feet apart in any development.
c. A new development may exceed the forty-foot spacing requirement under section b. above, under the following circumstances:
(1) Installing the tree would interfere with existing utility lines and no substitute tree is appropriate for the site; or
(2) There is not adequate space in which to plant a street tree due to driveway or street light locations, vision clearance or utility connections, provided the driveways, street light or utilities could not be reasonably located elsewhere so as to accommodate adequate room for street trees; and
(3) The street trees are spaced as close as possible given the site limitations in (1) and (2) above.
(4) The location of street trees in an ODOT or Washington County right-ofway may require approval, respectively, by ODOT or Washington County and are subject to the relevant state or county standards.
(5) For arterial and collector streets, the City may require planted medians in lieu of paved twelve-foot wide center turning lanes, planted with trees to the specifications of this subsection.
Response: Proposed tree spacing along SW Tualatin-Sherwood Road, SW 124th Avenue, and future cul-de-sac SW Cipole Place is shown on Attachment 6 Sheets L1.10 and L1.13 at less than 40 feet on center. The applicant does not request any exceptions from that maximum spacing distance. Coordination with Washington County will determine the ultimate location and spacing of trees to be installed along SW Tualatin-Sherwood Road. Communications to date with City of Sherwood and Washington County staff have not indicated that planted medians are necessary or required along the site's SW Tualatin-Sherwood Road and SW 124th Avenue frontages. These standards are met.

## B. Removal and Replacement of Street Trees.

Response: While some existing trees in areas to be dedicated as Tualatin-Sherwood Road right-of-way will be removed, there are no existing street trees proposed for removal with this application. Based on direction from City staff, the applicant understands that a separate street tree removal permit is not required because the removal of existing trees is being reviewed as part of the overall development. This standard does not apply.
C. Homeowner's Association Authorization.

The Planning Commission may approve a program for the adoption, administration and enforcement by a homeowners' association (HOA) of regulations for the removal and replacement of street trees within the geographic boundaries of the association.

1. An HOA that seeks to adopt and administer a street tree program must submit an application to the City. The application must contain substantially the following information:
a. The HOA must be current and active. The HOA should meet at least quarterly and the application should include the minutes from official HOA Board meetings for
a period not less than eighteen (18) months (six (6) quarters) prior to the date of the application.
b. $\quad$ The application must include proposed spacing standards for street trees that are substantially similar to the spacing standards set forth in 16.142.060.A above.
c. The application must include proposed street tree removal and replacement standards that are substantially similar to the standards set forth in 16.142.060.B above.
d. The application should include a copy of the HOA bylaws as amended to allow the HOA to exercise authority over street tree removal and replacement, or demonstrate that such an amendment is likely within ninety (90) days of a decision to approve the application.
e. The application should include the signatures of not less than seventy-five (75) percent of the homeowners in the HOA in support of the application.
2. An application for approval of a tree removal and replacement program under this section shall be reviewed by the City through the Type IV land use process. In order to approve the program, the City must determine:
a. The HOA is current and active.
b. $\quad$ The proposed street tree removal and replacement standards are substantially similar to the standards set forth in 16.142.060.B above.
c. The proposed street tree spacing standards are substantially similar to the standards set forth in 16.142.060.A above.
d. The HOA has authority under its bylaws to adopt, administer and enforce the program.
e. The signatures of not less than seventy-five (75) percent of the homeowners in the HOA in support of the application.
3. A decision to approve an application under this section shall include at least the following conditions:
a. Beginning on the first January 1 following approval and on January 1 every two (2) years thereafter, the HOA shall make a report to the city planning department that provides a summary and description of action taken by the HOA under the approved program. Failure to timely submit the report that is not cured within sixty (60) days shall result in the immediate termination of the program.
b. The HOA shall comply with the requirements of Section 12.20 of the Sherwood Municipal Code.
4. The City retains the right to cancel the approved program at any time for failure to substantially comply with the approved standards or otherwise comply with the conditions of approval.
a. If an HOA tree removal program is canceled, future tree removals shall be subject to the provisions of section 16.142.060.
b. A decision by the City to terminate an approved street tree program shall not affect the validity of any decisions made by the HOA under the approved program that become final prior to the date the program is terminated.
c. If the city amends the spacing standards or the removal and replacement standards in this section (SZCDC 16.142.060) the City may require that the HOA amend the corresponding standards in the approved street tree program.
5. An approved HOA tree removal and replacement program shall be valid for five (5) years; however the authorization may be extended as approved by the City, through a Type II Land Use Review.

Response: The applicant is not seeking to implement a Homeowners Association for the proposed corporate park. This standard does not apply.
D. Exemption from Replacing Street Trees.

A street tree that was planted in compliance with the Code in effect on the date planted and no longer required by spacing standards of section A.4. above may be removed without replacement provided:

1. Exemption is granted at the time of street tree removal permit or authorized homeowner's association removal per Section 16.142.060.C. above.
2. The property owner provides a letter from a certified arborist stating that the tree must be removed due to a reason identified in the tree removal criteria listed in Section 16.142.060.B.1. above, and
3. The letter describes why the tree cannot be replaced without causing continued or additional damage to public or private utilities that could not be prevented through reasonable maintenance.
Response: While the applicant is proposing to remove some trees from the abutting road rights-of-way, these trees are not intentionally-planted street trees. Therefore, the applicant is not proposing to remove street trees, so no replacement is required. These standards are not applicable.
E. Notwithstanding any other provision in this section, the city manager or the manager's designee may authorize the removal of a street tree in an emergency situation without a tree removal permit when the tree poses an immediate threat to life, property or utilities. A decision to remove a street tree under this section is subject to review only as provided in ORS 34.100.
Response: No intentionally-planted street trees currently exist within the public right-of-way, therefore the application will not seek the removal of street trees. . This standard does not apply.

## F. Trees on Private Property Causing Damage.

Any tree, woodland or any other vegetation located on private property, regardless of species or size, that interferes with or damages public streets or utilities, or causes an unwarranted increase in the maintenance costs of same, may be ordered removed or cut by the City Manager or his or her designee. Any order for the removal or cutting of such trees, woodlands or other vegetation, shall be made and reviewed under the applicable City nuisance abatement ordinances.
Response: The proposed development will not seek the authorization for tree removal under this provision. This standard does not apply.

## G. Penalties.

The abuse, destruction, defacing, cutting, removal, mutilation or other misuse of any tree planted on public property or along a public street as per this Section, shall be subject to the penalties defined by Section 16.02.040, and other penalties defined by applicable ordinances and statutes, provided that each tree so abused shall be deemed a separate offense.
Response: Per the arborist report (Attachment 18) and Sensitivity Plans (Attachment 6 Sheets C7.0-C7.5), several trees have been documented within the public right-of-way. However, these trees were not intentionally planted to be street trees. As the applicant is seeking City authorization prior to removal, no penalties are appropriate. This standard does not apply.

### 16.142.070 - Trees on Property Subject to Certain Land Use Applications

A. Generally

The purpose of this Section is to establish processes and standards which will minimize cutting or destruction of trees and woodlands within the City. This Section is intended to help protect the scenic beauty of the City; to retain a livable environment through the beneficial effect of trees on
air pollution, heat and glare, sound, water quality, and surface water and erosion control; to encourage the retention and planting of tree species native to the Willamette Valley and Western Oregon; to provide an attractive visual contrast to the urban environment, and to sustain a wide variety and distribution of viable trees and woodlands in the community over time.
B. Applicability

All applications including a Type II - IV land use review, shall be required to preserve trees or woodlands, as defined by this Section to the maximum extent feasible within the context of the proposed land use plan and relative to other codes, policies, and standards of the City Comprehensive Plan.
Response: The Site Plan Review and Conditional Use requests presented through this application are subject to the standards addressed below.
C. Inventory

1. To assist the City in making its determinations on the retention of trees and woodlands, land use applications including Type II - IV development shall include a tree and woodland inventory and report. The report shall be prepared by a qualified professional and must contain the following information:
a. Tree size (in DBH and canopy area)
b. Tree species
c. The condition of the tree with notes as applicable explaining the assessment
d. The location of the tree on the site
e. The location of the tree relative to the planned improvements
$f$. Assessment of whether the tree must be removed to accommodate the development
g. Recommendations on measures that must be taken to preserve trees during the construction that are not proposed to be removed.
2. In addition to the general requirements of this Section, the tree and woodland inventory's mapping and report shall also include, but is not limited to, the specific information outlined in the appropriate land use application materials packet.
3. Definitions for the inventory purposes of this Section
a. A tree is a living woody plant having a trunk diameter as specified below at Diameter at Breast Height (DBH). Trees planted for commercial agricultural purposes, and/or those subject to farm forest deferral, such as nut and fruit orchards and Christmas tree farms, are excluded from this definition and from regulation under this Section, as are any living woody plants under six (6) inches at DBH. All trees six (6) inches or greater shall be inventoried.
b. A woodland is a biological community dominated by trees covering a land area of 20,000 square feet or greater at a density of at least fifty (50) trees per every 20,000 square feet with at least fifty percent (50\%) of those trees of any species having a six (6) inches or greater at DBH. Woodlands planted for commercial agricultural purposes and/or subject to farm forest deferral, such as nut and fruit orchards and Christmas tree farms, are excluded from this definition, and from regulation under this Section.
c. A large stature tree is over 20 feet tall and wide with a minimum trunk diameter of 30 inches at DBH.
Response: The site has considerable tree coverage, as illustrated on Sheets C2.0 and C7.0-C7.6 in Attachment 6. The project surveyor and arborist identified the extents of the tree canopy and inventoried the majority of the trees. The submitted tree inventory and arborist report (Attachment 18) provide information on the location, species, size, canopy, and condition of existing trees located within the
boundaries of the site, as well as trees located along the site's SW Tualatin-Sherwood Road and SW 124th Avenue frontages (Attachment 6). Some of the trees within the interior of the site were not individually inventoried, as they are located in areas where the buildings, parking areas, and truck courts are proposed so the trees will be removed to accommodate the proposed industrial development. The intent of this standard is met.

## D. Retention requirements

1. Trees may be considered for removal to accommodate the development including buildings, parking, walkways, grading etc., provided the development satisfies of D. 2 or D.3, below.

Response: The applicant proposes to remove those trees in areas where the buildings, parking areas, and truck courts are proposed and trees where grading along the site perimeter is proposed. However, as shown on Attachment 6, 502 new trees are proposed for installation throughout the site, and 505 trees are proposed to be preserved. Findings in response to items "D.2" and "D.3" are presented below. This standard is met.
2. Required Tree Canopy - Residential Developments (Single Family Attached, Single Family Detached and Two - Family)
Each net development site shall provide a variety of trees to achieve a minimum total tree canopy of 40 percent. The canopy percentage is based on the expected mature canopy of each tree by using the equation $\pi r 2$ to calculate the expected square footage of canopy for each tree. The expected mature canopy is counted for each tree regardless of an overlap of multiple tree canopies.
The canopy requirement can be achieved by retaining existing trees or planting new trees. Required street trees can be used toward the total on site canopy required to meet this standard. The expected mature canopy spread of the new trees will be counted toward the needed canopy cover. A certified arborist or other qualified professional shall provide the estimated tree canopy of the proposed trees to the planning department for review.
Response: The subject proposal does not include residential development. This standard is not applicable.
3. Required Tree Canopy - Non-Residential and Multi-family Developments Each net development site shall provide a variety of trees to achieve a minimum total tree canopy of 30 percent. The canopy percentage is based on the expected mature canopy of each tree by using the equation $\pi r 2$ to calculate the expected square footage of each tree. The expected mature canopy is counted for each tree even if there is an overlap of multiple tree canopies.
The canopy requirement can be achieved by retaining existing trees or planting new trees. Required landscaping trees can be used toward the total on site canopy required to meet this standard. The expected mature canopy spread of the new trees will be counted toward the required canopy cover. A certified arborist or other qualified professional shall provide an estimated tree canopy for all proposed trees to the planning department for review as a part of the land use review process.
16.142.070 - Required Tree Canopy

Residential (single family \& two-family developments)

Old Town \& Infill developments

Commercial, Industrial, Institutional Public and Multi-family


Response: As shown on Attachment 6, landscaping plans proposed for the T-S Corporate Park and portions of the site to be developed with warehouse, distribution, and light industrial uses will achieve a tree canopy coverage of 23 percent of the net site area through installation of 502 new deciduous and evergreen trees. These percentages are based on the calculated mature canopy of each selected tree species, as determined through use of the equation stipulated above. These coverages comply with Section 16.142.070.D. 3 and, by rule, will effectively mitigate the site's
existing tree canopy. Furthermore, as evidenced in the arborist report (Attachment 18), the 505 existing trees being retained will provide mature canopy of approximately 60.8 percent of the net site area. Combined, the existing and proposed trees will provide a canopy of 83.8 percent, which far exceeds the $30 \%$ requirement. This standard is met.
4. The City may determine that, regardless of D. 1 through D.3, that certain trees or woodlands may be required to be retained. The basis for such a decision shall include; specific findings that retention of said trees or woodlands furthers the purposes and goals of this Section, is feasible and practical both within the context of the proposed land use plan and relative to other policies and standards of the City Comprehensive Plan, and are: a. Within a Significant Natural Area, 100-year floodplain, City greenway, jurisdictional wetland or other existing or future public park or natural area designated by the City Comprehensive Plan, or
b. A landscape or natural feature as per applicable policies of the City Comprehensive Plan, or are necessary to keep other identified trees or woodlands on or near the site from being damaged or destroyed due to windfall, erosion, disease or other natural processes, or
c. Necessary for soil stability and the control of erosion, for managing and preserving surface or groundwater quantities or quality, or for the maintenance of a natural drainageway, as per Clean Water Services stormwater management plans and standards of the City Comprehensive Plan, or
d. Necessary in required buffers between otherwise incompatible land uses, or from natural areas, wetlands and greenways, or
e. Otherwise merit retention because of unusual size, size of the tree stand, historic association or species type, habitat or wildlife preservation considerations, or some combination thereof, as determined by the City.
Response: Additional tree preservation standards beyond D. 1 through D. 3 are not merited as they would hinder the development of the site in contravention with the City's economic development objectives. Furthermore, since the proposed mature canopy of $60.8 \%$ far exceeds the $30 \%$ minimum standard for industrial development, the applicant is already performing significant tree retention and planting for an industrial development. The trees proposed for removal from the site are not located within a 100-year floodplain, City greenway, jurisdictional wetland, or existing or planned public park. ${ }^{5}$ The arborist report (Attachment 18) provides specific recommendations on how to best maintain the quality and prevent damage to the 505 existing trees that will remain on site. New deciduous and evergreen trees will provide buffering and screening of the site from nearby areas. The applicant is not aware of any unique species, historic, or habitat considerations that would merit preservation of trees proposed for removal. This standard does not apply.
5. Tree retention requirements for properties located within the Old Town Overlay or projects subject to the infill standards of Chapter 16.68 are only subject to retention requirements identified in D.4. above.
Response: The subject site is not located within the Old Town Overlay. This standard is not applicable.
6. The Notice of Decision issued for the land use applications subject to this Section shall indicate which trees and woodlands will be retained as per subsection D of this Section,

[^4]which may be removed or shall be retained as per subsection $D$ of this Section and any limitations or conditions attached thereto.
Response: The applicant requests that the Notice of Decision enumerate tree preservation and removal as proposed in the attached drawings and arborist report. This standard is met.
7. All trees, woodlands, and vegetation located on any private property accepted for dedication to the City for public parks and open space, greenways, Significant Natural Areas, wetlands, floodplains, or for storm water management or for other purposes, as a condition of a land use approval, shall be retained outright, irrespective of size, species, condition or other factors. Removal of any such trees, woodlands, and vegetation prior to actual dedication of the property to the City shall be cause for reconsideration of the land use plan approval.
Response: The applicant proposes to retain some of the trees in the proposed tracts depicted in Attachment 6, Sheet C8.0. If any of these tracts are dedicated to the City, then trees will be retained to the extent that they do not interfere with utility installation. This standard is met.
E. Tree Preservation Incentive

Retention of existing native trees on site which are in good health can be used to achieve the required mature canopy requirement of the development. The expected mature canopy can be calculated twice for existing trees. For example, if one existing tree with an expected mature canopy of 10 feet ( 78.5 square feet) is retained it will count as twice the existing canopy (157 square feet).
Response: As detailed in the arborist report (Attachment 18), the expected mature canopy has been calculated for existing trees proposed to remain on site. This standard is met.

## F. Additional Preservation Incentives

1. General Provisions. To assist in the preservation of trees, the City may apply one or more of the following flexible standards as part of the land use review approval. To the extent that the standards in this section conflict with the standards in other sections of this Title, the standards in this section shall apply except in cases where the City determines there would be an unreasonable risk to public health, safety, or welfare. Flexibility shall be requested by the applicant with justification provided within the tree preservation and protection report as part of the land use review process and is only applicable to trees that are eligible for credit towards the effective tree canopy cover of the site. A separate adjustment application as outlined in Section 16.84.030.A is not required.
2. Flexible Development Standards. The following flexible standards are available to applicants in order to preserve trees on a development site. These standards cannot be combined with any other reductions authorized by this code.
a. Lot size averaging. To preserve existing trees in the development plan for any Land Division under Division VII, lot size may be averaged to allow lots less than the minimum lot size required in the underlying zone as long as the average lot area is not less than that allowed by the underlying zone. No lot area shall be less than 80 percent of the minimum lot size allowed in the zone;
b. Setbacks. The following setback reductions will be allowed for lots preserving existing trees using the criteria in subsection (1) below. The following reductions shall be limited to the minimum reduction necessary to protect the tree.
(1) Reductions allowed:
(a.) Front yard - up to a 25 percent reduction of the dimensional standard for a front yard setback required in the base zone. Setback of garages may not be reduced by this provision.
(b.) Interior setbacks - up to a 40 percent reduction of the dimensional standards for an interior side and/or rear yard setback required in the base zone.
(c.) Perimeter side and rear yard setbacks shall not be reduced through this provision.
c. Approval criteria:
(1.) A demonstration that the reduction requested is the least required to preserve trees; and
(2.) The reduction will result in the preservation of tree canopy on the lot with the modified setbacks; and
(3.) The reduction will not impede adequate emergency access to the site and structure.
Response: The applicant is not requesting to rely on any of the incentives described above. These standards are not applicable.
3. Sidewalks. Location of a public sidewalk may be flexible in order to preserve existing trees or to plant new large stature street trees. This flexibility may be accomplished through a curb-tight sidewalk or a meandering public sidewalk easement recorded over private property and shall be reviewed on a case by case basis in accordance with the provisions of the Engineering Design Manual, Street and Utility Improvement Standards. For preservation, this flexibility shall be the minimum required to achieve the desired effect. For planting, preference shall be given to retaining the planter strip and separation between the curb and sidewalk wherever practicable. If a preserved tree is to be utilized as a street tree, it must meet the criteria found in the Street Tree section, 16.142.060.
Response: The applicant is not seeking flexibility from tree preservation and planting requirements in order to construct new sidewalks. This standard is not applicable.
4. Adjustments to Commercial and Industrial development Standards. Adjustments to Commercial or Industrial Development standards of up to 20 feet additional building height are permitted provided;
a. At least $50 \%$ of a Significant Tree stand's of canopy within a development site (and not also within the sensitive lands or areas that areas dedicated to the City) is preserved;
b. The project arborist or qualified professional certifies the preservation is such that the connectivity and viability of the remaining significant tree stand is maximized;
c. Applicable buffering and screening requirements are met;
d. Any height adjustments comply with state building codes;
e. $\quad$ Significant tree stands are protected through an instrument or action subject to approval by the City Manager or the City manager's designee that demonstrates it will be permanently preserved and managed as such;
(1.) A conservation easement;
(2.) An open space tract;
(3.) A deed restriction; or
(4.) Through dedication and acceptance by the City.

Response: The applicant will not be requesting any adjustments to the El zone development standards in order to preserve additional existing trees. These standards are not applicable.

## G. Tree Protection During Development

The applicant shall prepare and submit a final Tree and Woodland Plan prior to issuance of any construction permits, illustrating how identified trees and woodlands will be retained, removed or
protected as per the Notice of Decision. Such plan shall specify how trees and woodlands will be protected from damage or destruction by construction activities, including protective fencing, selective pruning and root treatments, excavation techniques, temporary drainage systems, and like methods. At a minimum, trees to be protected shall have the area within the drip line of the tree protected from grading, stockpiling, and all other construction related activity unless specifically reviewed and recommended by a certified arborist or other qualified professional. Any work within the dripline of the tree shall be supervised by the project arborist or other qualified professional onsite during construction.
Response: The trees to be retained on site are located within the wetland mitigation areas, tract A and D and along the west and south site perimeter. Tree protection within this area is shown on Sheets C7.0C7.2 in Attachment 6. This standard is met.

## H. Penalties

Violations of this Section shall be subject to the penalties defined by Section 16.02.040, provided that each designated tree or woodland unlawfully removed or cut shall be deemed a separate offense.
Response: The applicant is proposing tree removal and retention in accordance with the approval criteria in this section. This standard does not apply.

### 16.142.090-Recommended Street Trees

## A. Recommended Street Trees:

| 16.142 .090 - Recommended Street Trees |  |  |
| :--- | :--- | :--- |
| Common Name | Botanical Name | Canopy Spread (feet) |
| Acer - Maple | Acer platanoides cavalier |  |
| Cavalier Norway Maple | p. Cleveland | 30 |
| Cleveland Norway Maple | p. Cleveland | 25 |
| Cleveland II Norway Maple | p. columnare | 15 |
| Columnar Norway Maple | p. fairway | 40 |
| Fairway Sugar Maple (sugar <br> maple) | p. olmsted | $20-25$ |
| Olmsted Norway Maple | Acer triflorum | 20 |
| Roughbark Maple | Acer buergeranum | 20 |
| Trident Maple | Acer grandidentatum 'Schmidt' | 15 |
| Rocky Mountain Glow Maple | 20 |  |
| David's Maple | Acer davidii | 25 |
| Metro Gold Hedge Maple | Acer campestre 'Panacek' |  |
| Red Sunset Maple (Old Town) | Acer rubrum red sunset - Red <br> Sunset Maple (Old Town) <br> (Provided that a root barrier is <br> installed) | $25-40$ |
| Royal Red Maple | r. royal red | $20-25$ |
| Gerling Red Maple | r. gerling | $25-35$ |
| Tilford Red Maple | r. tilford |  |

### 16.142.090 - Recommended Street Trees

| Common Name | Botanical Name | Canopy Spread (feet) |
| :---: | :---: | :---: |
| Carpinus - Hornbeam |  |  |
| Pyramidal European Hornbeam | Carpinus betulus pyramidalis | 30-40 |
| Pyramidal European Hornbeam | b. columnaris | 15 |
| Pyramidal European Hornbeam | b. fastigiata | 15-20 |
| Pyramidal European Hornbeam | b. fastigiata | 15-20 |
| Eastern Redbud | Cercic, canadenis - Canadian Red Bud | 10-20 |
| Fraxinus - Ash |  |  |
| Dr. Pirone Ash | augustifolia dr. pirone | 35-50 |
| Raywood Ash | raywoodi | 20 |
| Oregon Ash | latifolia | 25-40 |
| Ginkgo |  |  |
| Autumn Gold | biloba | 25-35 |
| Fairmount | biloba | 15-25 |
| Gleditsia |  |  |
| Honey Locust | triacanthos sunburst | 20-30 |
| Liquidamber |  |  |
| American Sweetgum | styraciflua | 40 |
| Liriodenrod |  | 30-50 |
| Magnolia |  |  |
| Evergreen Magnolia | grandiflora vars |  |
| Southern Magnolia | grandiflora | 40 |
| Dr. Merrill Magnolia | kobus dr. merrill | 15-20 |
| Edith Bogue Magnolia |  | 15 |
| Purnus - Cherry - Plum |  |  |
| Double Flowering Cherry | avium plena | 30-40 |
| Scanlon Globe Cherry | avium scanlon | 30-40 |
| Japanese Cherry | serrulata vars (nonweeping) | 15-30 |
| Okame Cherry | okame | 20-30 |
| Blireana Plum | blireana | 20 |
| Pissardi Plum | pissardi | 10 |
| Krauter's Vesuvius Plum | Vesuvius | 15 |
| Amur Chokecherry | maacki | 25-30 |
| Redbark Cherry | serrula | 20-30 |
| European Birdcherry | padus | 35 |
| Bigflowered Birdcherry | grandiflora | 10-20 |
| Rancho Birdcherry | berg | 15-20 |
| Purpleleaf Birdcherry | purpurea | 10-20 |
| Prairifire Crabapple | Malus 'Prairifire' | 20 |
| Quercus |  |  |
| Crimson Spire Oak | Quercus alba x $Q$. robur 'Crimschmidt' | 15 |

16.142.090 - Recommended Street Trees

| Common Name |  |  |
| :--- | :--- | :--- |
| Botanical Name |  | Canopy Spread (feet) |
| Pin Oak | americana | $35-40$ |
| Tilia - Linden | cordata | $35-40$ |
| American Linden | euchlora | 40 |
| Little Leaf Linden | tomentosa | $20-30$ |
| Crimean Linden | bicentennial | 40 |
| Silver Linden | greenspire | 30 |
| Bicentennial Linden | salem | 20 |
| Greenspire Linden | Tiliacordata 'Chancole' | $20-30$ |
| Salem Linden | 20 |  |
| Chancellor Linden |  |  |

B. Recommended Street Trees under Power Lines:

Acer ginnala - Amur Maple 20' spread
Acer campestre - Hedge Maple 30' spread
Acer palmatum - Japanese Maple 25' spread
Acer griseum - Paperbark Maple 20' spread
Acer circinatum - Vine Maple 25' spread
Amelanchier x grandiflora - Apple Serviceberry 20' spread
Amelanchier Canadensis - Shadblow Serviceberry 20' spread
Cercis Canadensis - Eastern Redbud 25-30' spread
Clerodendrum trichotomum - Glorybower Tree 20' spread
Cornus florida - Flowering Dogwood 20-25' spread
Cornus kousa - Japanese Dogwood 25' spread
Crataegus phaenopyrum - Washington Hawthorn 25' spread
Crataegus x lavellei - Lavelle Hawthorn 20' spread
Fraxinus excelsior globosum - Globe-Headed European Ash 12-15' spread
Fraxinus ornus - Flowering Ash 20-30' spread
Fraxinus oxycarpa aureopolia - Golden Desert Ash 18' spread
Koelreuteria paniculata - Goldenrain Tree 10-20' spread
Laburnum x waterii - Golden Chain Tree 15' spread
Malus - Flowering Crabapple 20-25' spread
Prunus - Flowering Cherry 20-25' spread
Pyrus calleryana - Flowering Pear "Cleveland Select" 20' spread
Styrax japonica - Japanese Snowbell 25' spread
Syringa reticulata - Japanese Tree Lilac 20-25' spread
C. Prohibited Street Trees:

Acer, Silver Maple
Acer, Boxelder
Ailanthus, gladulosa - Tree-of-heaven
Betula; common varieties of Birch
Ulmus; common varieties of Elm
Morus; common varieties of Mulberry
Salix; common varieties of willow
Coniferous Evergreen (Fir, Pine, Cedar, etc.)
Populus; common varieties of poplar, cottonwood and aspen
Female Ginkgo
D. Alternative Street Trees: Trees that are similar to those on the recommended street tree list can be proposed provided that they are non-fruit bearing, non-invasive and not listed on the prohibited street tree list. A letter from a certified arborist must be submitted, explaining why the tree is an equivalent or better street tree than the recommended street trees that are identified in this section.
Response: The landscape plans (Attachment 6, Sheets L0.02 and L1.10 -L1.13) propose street trees in compliance with the recommended trees noted above. This standard is met.

## Chapter 16.144 - Wetland, Habitat and Natural Areas

### 16.144.010-Generally

Unless otherwise permitted, residential, commercial, industrial, and institutional uses in the City shall comply with the following wetland, habitat and natural area standards if applicable to the site as identified on the City's Wetland Inventory, the Comprehensive Plan Natural Resource Inventory, the Regionally Significant Fish and Wildlife Habitat Area map adopted by Metro, and by reference into this Code and the Comprehensive Plan. Where the applicability of a standard overlaps, the more stringent regulation shall apply.
Response: The Comprehensive Plan's Natural Resources and Recreation Map does not include the subject site as the map pre-dates inclusion of the site within the urban growth boundary or city limits. Three wetland areas, totaling approximately 3.66 acres, have been identified within the boundaries of the site and documented in the Wetland Delineation Report (Attachment 14) and the Natural Resource Assessment Report (Attachment 15). Per Metro's Regionally Significant Fish and Wildlife Habitat Area GIS data, Upland Class B habitat has been identified on the southern portion of the site (see Figure 2). However, based on Metro's Habitat Conservation Areas Map, the site property does not contain any land areas designated by Metro as Habitat Conservation Areas. See Figure 3.


Figure 2: Metro Regionally Significant Fish and Wildlife Habitat Area


Figure 3: Metro Habitat Conservation Areas
The proposed development has been designed to reduce the impact of the delineated wetlands and protect a significant portion of the upland habitat, preserving the ecological integrity of the area, as illustrated in the code responses hereafter.

### 16.144.020 - Standards

A. The applicant shall identify and describe the significance and functional value of wetlands on the site and protect those wetlands from adverse effects of the development. A facility complies with this standard if it complies with the criteria of subsections A.1.a and A.1.b, below:

1. The facility will not reduce the area of wetlands on the site, and development will be separated from such wetlands by an area determined by the Clean Water Services Design and Construction Standards R\&O 00-7 or its replacement provided Section 16.140.090 does not require more than the requested setback.
a. A natural condition such as topography, soil, vegetation or other feature isolates the area of development from the wetland.
b. Impact mitigation measures will be designed, implemented, and monitored to provide effective protection against harm to the wetland from sedimentation, erosion, loss of surface or ground water supply, or physical trespass.
c. A lesser setback complies with federal and state permits, or standards that will apply to state and federal permits, if required.
2. If existing wetlands are proposed to be eliminated by the facility, the applicant shall demonstrate that the project can, and will develop or enhance an area of wetland on the site or in the same drainage basin that is at least equal to the area and functional value of wetlands eliminated.

Response: Three wetlands have been identified within the boundaries of the site. Per the Pacific Habitat Services Natural Resource Assessment Report (Attachment 15), Wetland A, approximately 2.34 acres, is a broad seasonal swale that extends the northward to SW Tualatin-Sherwood Road in the northeastern portion of the site, while Wetland $B$ is a small concave wetland on a gentle slope in the now fallow field of Wetland A. Slopes are generally quite gentle across the north end of the site but increase to the south. Wetland C is an approximate 1.29 -acre depressional feature at the south end of the site that extends outside property boundaries with relatively steep slopes on the west and east sides. The northern edge is comparatively low in elevation, but topography rises several feet just to the north. In compliance with Clean Water Services Design and Construction Standards R\&O 00-7 provisions, Pacific Habitat Services identified vegetated corridors (VCs) based on wetland size and the slopes adjacent to the sensitive areas, as summarized in the following table.

| Summary of Vegetated Corridor Widths |  |  |
| :---: | :---: | :---: |
| Sensitive Area | VC Width | Justification |
| Wetland A | 50 feet | - >0.5 acres <br> - Slopes |
| Wetland B | 25 feet | - $\leq 0.5$ acres and isolated <br> - Slopes $<25 \%$ |
| Wetland C | 50 feet or greater | - >0.5 acres <br> - Slopes variable; > and $25 \%$ |

No impacts or alterations are proposed to the wetlands on site. Approximately 10,699 SF of permanent VC encroachment will result from site development (Attachment 15, Figures 4-4C); to facilitate site development (e.g., roadway construction and grading). Individual encroachments are associated with Cipole Place, the site driveway to Building E , steep bank slopes southwest of Building D , and a driveway behind (south of) Building C.

The total area of permanent encroachment also includes 100 SF associated with each of three separate rip rap stilling basins related to the site's stormwater outfalls. As each is a minor encroachment associated with utility infrastructure, and not more than 100 SF in size, replacement mitigation is not necessary (per current CWS D\&C Standards, Chapter 3, Section 3.05.5c and d).

Temporary encroachments will be limited to a trio of stormwater outfall lines that lead to riprap stilling basins; one each to the west and east sides of Wetland A, just south of Tualatin-Sherwood Road, and one at the south end of Wetland A. The alignments of associated pipelines have been sited to facilitate proper drainage. The installation of these pipelines will require a combined area of temporary encroachment of $4,917 \mathrm{SF}$. The footprint of temporary encroachment is defined by a 20 -foot wide construction corridor centered roughly along the proposed pipe alignments. Each of the three rip rap pads for the storm outfalls will require permanent encroachment of 100 SF , as described above.

VC encroachment of 10,699 SF for site development will be mitigated through the expansion of an equivalent area of VC east of Wetland $A$, north of Wetland B, and north of Wetland C (Attachment 15, Figures $4-4 \mathrm{C}$ ). Though mitigation is not required for the 300 SF of rip rap stilling basins, mitigation will nonetheless be provided. In total, 35,654 SF of VC mitigation will be provided outside of the wetland and
required VC. This includes a 1 to 1 replacement for proposed encroachments, as well as an additional $24,955 \mathrm{SF}$ of mitigation; proposed as a water quality benefit to the project. VC expansion will occur within five individual areas. The largest is located east of Wetland A, with smaller areas west of Wetland A, north and south of stormwater Tract C. Two additional areas will expand existing VC north of Wetland C closer to the south end of the development footprint. Proposed expansions will widen existing VC by up to 85 feet. As enhancements will be required throughout the first 50 feet of existing VC, strengthening of the proposed mitigation and water quality benefit expansion areas will occur concurrently with other invasive species control and plant installation improvements. Pacific Habitat Services has submitted the Natural Resource Assessment report (Attachment 15) to Clean Water Services for its review. The applicant has revised the site design per Clean Water Services request before CWS would issue its service provider letter, enclosed as Attachment 19. With the concurrence of Clean Water Services, this standard is met.
B. The applicant shall provide appropriate plans and text that identify and describe the significance and functional value of natural features on the site (if identified in the Community Development Plan, Part 2) and protect those features from impacts of the development or mitigate adverse effects that will occur. A facility complies with this standard if:

1. The site does not contain an endangered or threatened plant or animal species or a critical habitat for such species identified by Federal or State government (and does not contain significant natural features identified in the Community Development Plan, Part 2, Natural Resources and Recreation Plan).
2. The facility will comply with applicable requirements of the zone.
3. The applicant will excavate and store topsoil separate from subsurface soil, and shall replace the topsoil over disturbed areas of the site not covered by buildings or pavement or provide other appropriate medium for re-vegetation of those areas, such as yard debris compost.
4. The applicant will retain significant vegetation in areas that will not be covered by buildings or pavement or disturbed by excavation for the facility; will replant areas disturbed by the development and not covered by buildings or pavement with native species vegetation unless other vegetation is needed to buffer the facility; will protect disturbed areas and adjoining habitat from potential erosion until replanted vegetation is established; and will provide a plan or plans identifying each area and its proposed use.
5. Development associated with the facility will be set back from the edge of a significant natural area by an area determined by the Clean Water Services Design and Construction standards R\&O 00-7 or its replacement, provided Section 16.140.090A does not require more than the requested setback. Lack of adverse effect can be demonstrated by showing the same sort of evidence as in subsection A. 1 above.
Response: The applicant is unaware of any endangered or threatened plant or animal species or critical habitat within the development site, and the site does not contain notable natural features as illustrated in the Community Development Plan, Part 2, Natural Resource and Recreation Plan. The site was recently annexed into the City of Sherwood from Washington County and was not accounted for within the Community Development Plan Natural Resource and Recreation Map (see Figure 2, above). Due to the existing conditions of the site a Wetland Delineation Report (Attachment 14) and Natural Resource Assessment (Attachment 15) were prepared as part of this application. The proposed five-building facility has been designed to comply with applicable zoning standards and erosion and sedimentation control measures promulgated by the City, Clean Water Services, and the Oregon Department of Environmental Quality. The applicant proposes to minimize impact to the delineated wetlands by completely avoiding encroachments into the wetlands and by utilizing vegetated corridor and replanting mitigation as described in section 16.144.020.A and approved by Clean Water Services (Attachment 19). This standard is met.
C. When the Regionally Significant Fish and Wildlife Habitat map indicates there are resources on the site or within 50 feet of the site, the applicant shall provide plans that show the location of resources on the property. If resources are determined to be located on the property, the plans shall show the value of environmentally sensitive areas using the methodologies described in Sections 1 and 2 below.
The Metro Regionally Significant Fish and Wildlife Habitat map shall be the basis for determining the location and value of environmentally sensitive habitat areas. In order to specify the exact locations on site, the following methodology shall be used to determine the appropriate boundaries and habitat values:
6. Verifying boundaries of inventoried riparian habitat. Locating habitat and determining its riparian habitat class is a four-step process:
a. Located the Water Feature that is the basis for identifying riparian habitat.
7. Locate the top of bank of all streams, rivers, and open water within 200 feet of the property.
8. Locate all flood areas within 100 feet of the property.
9. Locate all wetlands within 150 feet of the property based on the Local Wetland Inventory map and on the Metro 2002 Wetland Inventory map (available from the Metro Data Resource Center, 600 NE Grand Ave., Portland, OR 97232). Identified wetlands shall be further delineated consistent with methods currently accepted by the Oregon Division of State Lands and the US Army Corps of Engineers.
Response: Riparian habitat or wetlands are not identified on-site per Metro's Regional Land Information GIS Map, the current documentation of Regionally Significant Fish and Wildlife Habitat information (see Figure 2, above). Riparian habitat exists approximately 750 feet south of the site, well beyond the identification thresholds listed above. However, identified wetlands were further delineated by Pacific Habitat Services in accordance with the methods currently accepted by the Oregon Division of State Lands and the US Army Corps of Engineers (Attachment 14). Based on Metro's Habitat Conservation Areas Map (Figure 3, above), the site property does not contain any land areas designated by Metro as Habitat Conservation Areas. This standard is met.
b. Identify the vegetative cover status of all areas on the property that are within 200 feet of the top of bank of streams, rivers, and open water, are wetlands or are within 150 feet of wetlands, and are flood areas or are within 100 feet of flood areas. Vegetative cover status shall be as identified on the Metro Vegetative Cover map. In the event of a discrepancy between the Metro Vegetative Cover map and the existing site conditions, document the actual vegetative cover based on the following definitions along with a 2002 aerial photograph of the property;
10. Low structure vegetation or open soils - Areas that are part of a contiguous area one acre or larger of grass, meadow, crop-lands, or areas of open soils located within 300 feet of a surface stream (low structure vegetation areas may include areas of shrub vegetation less than one acre in size if they are contiguous with areas of grass, meadow, crop-lands, orchards, Christmas tree farms, holly farms, or areas of open soils located within 300 feet of a surface stream and together form an area of one acre in size or larger).
11. Woody vegetation - Areas that are part of a contiguous area one acre or larger of shrub or open or scattered forest canopy (less than 60\% crown-closure) located within 300 feet of a surface stream.
12. Forest canopy - Areas that are part of a contiguous grove of trees of one acre or larger in area with approximately $60 \%$ or greater crown closure, irrespective of whether the entire grove is within 200 feet of the relevant water feature.
Response: Figure 4below illustrates the documented vegetation types throughout the development area, per Metro's GIS Vegetation data. Per the Pacific Habitat Services Natural Resource Assessment (Attachment 15), a summary of plant communities adjacent to the associated delineated wetlands has been prepared.


Figure 4: Metro Vegetative Cover

| Summary of Plant Communities |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Corridor Conditions |  | Plant Communities |  |  |
|  |  | A | B | C |
| Good | $>80 \%$ cover of native plants, and $>50 \%$ tree canopy |  | $82 \%$ native plants $83 \%$ tree canopy | 52\% tree canopy |
| Marginal | 50\% - 80\% cover of native plants, and 26-50\% tree canopy |  |  |  |


| Degraded | < $50 \%$ cover of native plants, and $\leq 25 \%$ tree canopy | 3\% native plants 6\% tree canopy |  | $27 \%$ native plants |
| :---: | :---: | :---: | :---: | :---: |

The condition of VC is defined by the percentages of native species and canopy cover. Plant Community A is in degraded corridor condition, as the community lacks adequate tree canopy and is overwhelmingly dominated by non-native herbaceous species. Plant Community B has both a good native tree canopy and a high overall coverage of native species. As such, this community is in good corridor condition. Plant Community C is comprised of only 27 percent native species but has a variable tree canopy. As a result of this variability, the tree canopy is 53 percent, just enough to fall within the lower range of good condition. The variability of tree canopy relative to the lower percent cover of plants justifies a corridor condition of marginal for Community C. This standard has been met.
c. Determine whether the degree that the land slopes upward from all streams, rivers, and open water within 200 feet of the property is greater than or less than 25\% (using the Clean Water Services Vegetated Corridor methodology); and
Response: Per Table One of the Pacific Habitat Natural Resource Assessment (Attachment 15) and the applicant's response to Section 16.144.020, slopes upward from the delineated wetlands have been documented. Slopes adjoining Wetland C are steeper, but generally still less than 25 percent. There is one narrow point where slopes exceed 25 percent over the first 50 feet but are less than over the next 25 feet. At this location, a break in slope has been identified and the full setback of 35 feet from the break has been identified in accordance with Clean Water Services Vegetated Corridor methodology. This standard is met.
d. Identify the riparian habitat classes applicable to all areas on the property using Table 8-1 below:

| Table 8-1 - Riparian Habitat Classes |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Distance in | Development/Vegetation Status |  |  |  |
| Water <br> Feature | Developed areas not providing vegetative cover | Low structure vegetation or open soils | Woody vegetation <br> (shrub and scattered forest canopy) | Forest Canopy (closed to open forest canopy) |
| Surface Streams |  |  |  |  |
| 0-50 | Class II | Class 1 | Class 1 | Class 1 |
| 50-100 |  | Class II | Class I | Class 1 |
| 100-150 |  | Class II if slope > $25 \%$ | $\begin{gathered} \hline \text { Class II if slope } \\ >25 \% \\ \hline \end{gathered}$ | Class II |
| 150-200 |  | Class II if slope > 25\% | $\begin{gathered} \hline \text { Class II if slope } \\ >25 \% \end{gathered}$ | Class II if slope > 25\% |
| Wetlands (Wetland Feature itself is a Class I Riparian area) |  |  |  |  |
| 0-100 |  |  | Class I | Class I |
| 100-150 |  |  |  | Class II |
| Flood Areas (Undeveloped portion of a flood area is a Class I Riparian area) |  |  |  |  |
| 0-100 |  |  | Class II | Class II |

Response: The site does not contain and is not adjacent to surface streams or flood areas, but does contain wetlands as identified in Attachments 14 and 15. Per the table, the wetlands themselves are Class I Riparian Areas. Based on the wetland locations and the vegetated Metro Vegetative Cover data above, the scrub and forested areas within the first 100 feet of all three wetland boundaries would be classified as Class I Riparian Habitat. Areas within 100-150 feet of Wetlands A and B would not qualify as Riparian Habitat, and the forested areas (but not scrub areas) within 100-150 feet of Wetland C would qualify as Class II Riparian Habitat. However, based on Metro's Habitat Conservation Areas Map (Figure 3, above), the site property does not contain any land areas designated by Metro as Habitat Conservation Areas.
2. Verifying boundaries of inventoried upland habitat. Upland habitat was identified based on the existence of contiguous patches of forest canopy, with limited canopy openings. The "forest canopy" designation is made based on analysis of aerial photographs, as part of determining the vegetative cover status of land within the region. Upland habitat shall be as identified on the HCA map. The perimeter of an area delineated as "forest canopy" on the Metro Vegetative Cover map may be adjusted to more precisely indicate the drip line of the trees within the canopied area.
Response: As identified on Metro's Regional Land Information System data, Class B upland habitat exists on the southern portion of the site, upslope of the former agricultural fields. As described in the Pacific Habitat Services Wetland Delineation Report (Attachment 14) and Natural Resource Assessment (Attachment 15) the inventoried upland habitat encompasses a relatively young to mature overstory of Douglas fir, bigleaf maple, and Oregon White Oak. The existing conditions plan (Attachment 6, Sheet C2.0) illustrate the location of the tree canopy on site and identify
individual trees which have been assessed by the project arborist (Attachment 18). This standard is met.

### 16.144.030 - Exceptions to Standards

In order to protect environmentally sensitive areas that are not also governed by floodplain, wetland and Clean Water Services vegetated corridor regulations, the City allows flexibility of the specific standards in exchange for the specified amount of protection inventoried environmentally sensitive areas as defined in this code.

## A. Process

The flexibility of standards is only applicable when reviewed and approved as part of a land use application and shall require no additional fee or permit provided criteria is addressed. In the absence of a land use application, review may be processed as a Type 1 administrative interpretation.
Response: The on-site wetlands are regulated by the Oregon Department of State Lands and the U.S. Army Corps of Engineers, and the vegetated corridor is regulated by Clean Water Services. The applicant proposes to comply with applicable standards and is seeking flexibility on the parking standard per standard B. 4 below. This standard is met.
B. Standards modified

1. Lot size - Not withstanding density transfers permitted through Chapter 16.40, when a development contains inventoried regionally significant fish and wildlife habitats as defined in Section 16.144 .020 above, lot sizes may be reduced up to ten percent (10\%) below the minimum lot size of the zone when an equal amount of inventoried resource above and beyond that already required to be protected is held in a public or private open space tract or otherwise protected from further development.
Response: No lot size reduction is requested by the applicant. This standard does not apply.
2. Setbacks - For residential zones, the setback may be reduced up to thirty percent (30\%) for all setbacks except the garage setback provided the following criteria are satisfied:
a. The setback reduction must result in an equal or greater amount of significant fish and/or wildlife habitat protection. Protection shall be guaranteed with deed restrictions or public or private tracts.
b. In no case shall the setback reduction supersede building code and/or Tualatin Valley Fire and Rescue separation requirements.
c. In no case shall the setback be reduced to less than five feet unless otherwise provided for by the underlying zone.
Response: The site is not located within a residential zone; therefore, this standard does not apply.
3. Density - per Section 16.10.020 (Net Buildable Acre definition), properties with environmentally sensitive areas on site may opt to exclude the environmentally sensitive areas from the minimum density requirements provided the sensitive areas are protected via tract or restrictive easement. A proposal to remove said area from the density calculation must include: a delineation of the resource in accordance with Section 16.144.020C, the acreage being protected, and the net reduction below the normally required minimum for accurate reporting to Metro.
Response: The site is not located within a residential zone; this standard does not apply.
4. Parking - Per Section 16.94.020.B.6, 10-25\% of the required parking spaces may be reduced in order to protect inventoried regionally significant fish and wildlife habitat areas, provided these resources are protected via deed restrictions or held in public or private tracts.
Response: While the applicant is seeking a $20 \%$ reduction to required minimum parking due to the presence of wetlands (per Section 16.94.020.B.6), these wetlands have not been designated as regionally significant fish and wildlife habitat areas. This standard does not apply.
5. Landscaping - Per Section 16.92.030.B.6, exceptions may be granted to the landscaping standards in certain circumstances as outlined in that section.
Response: The applicant is not seeking the option of relief from the landscaping standards per the provisions of Section 16.92.030.B.7. This standard does not apply.

## Chapter 16.146-Noise

### 16.146.010-Generally

All otherwise permitted commercial, industrial, and institutional uses in the City shall comply with the noise standards contained in OAR 340-35-035. The City may require proof of compliance with OAR 340-35-035 in the form of copies of all applicable State permits or certification by a professional acoustical engineer that the proposed uses will not cause noise in excess of State standards.
Response: The applicant is aware of the statewide noise standards in OAR 340-35-035 and fully intends to comply as required by law. While specific users are not known at this time, the proposed buildings are likely to emit sounds at similar levels to other light industrial users in the area. The concrete construction type will assist in attenuation of indoor sounds, and no outdoor work activities other than vehicle circulation are proposed. This standard is met.

### 16.146.020 - Noise Sensitive Uses

When proposed commercial and industrial uses do not adjoin land exclusively in commercial or industrial zones, or when said uses adjoin special care, institutional, or parks and recreational facilities, or other uses that are, in the City's determination, sensitive to noise impacts, then:
A. The applicant shall submit to the City a noise level study prepared by a professional acoustical engineer. Said study shall define noise levels at the boundaries of the site in all directions.
B. The applicant shall show that the use will not exceed the noise standards contained in OAR 340-35-035, based on accepted noise modeling procedures and worst case assumptions when all noise sources on the site are operating simultaneously.
C. If the use exceeds applicable noise standards as per subsection B of this Section, then the applicant shall submit a noise mitigation program prepared by a professional acoustical engineer that shows how and when the use will come into compliance with said standards.
Response: Adjoining zones are industrial to the north, east, and south, and Washington County Future Development, 20-acre (FD-20) to the south and west (which will be zoned Employment Industrial upon annexation to City of Sherwood). The site does not abut special care, institutional, parks and recreational facilities, or other sensitive users. Furthermore, while specific users are not known at this time, the proposed buildings are likely to emit sounds at similar levels to other light industrial users in the area. This standard does not apply.

### 16.146.030-Exceptions

This Chapter does not apply to noise making devices which are maintained and utilized solely as warning or emergency signals, or to noise caused by automobiles, trucks, trains, aircraft, and other similar vehicles when said vehicles are properly maintained and operated and are using properly designated rights-of-way, travel ways, flight paths or other routes. This Chapter also does not apply to noise produced by humans or
animals. Nothing in this Chapter shall preclude the City from abating any noise problem as per applicable City nuisance and public safety ordinances.
Response: The applicant is aware that the development is subject to the City's nuisance ordinance. This standard is met.

## Chapter 16.148 - Vibrations

### 16.148.010-Generally

All otherwise permitted commercial, industrial, and institutional uses shall not cause discernible vibrations that exceed a peak of 0.002 gravity at the property line of the originating use, except for vibrations that last five (5) minutes or less per day, based on a certification by a professional engineer.
Response: While specific users are not known at this time, the proposed industrial uses are not anticipated to generate detectable vibration at the property line based on light industrial, manufacturing, and warehouse/distribution uses typical of the Tualatin-Sherwood Road corridor. This standard is met.

### 16.148.020-Exceptions

This Chapter does not apply to vibration caused by construction activities including vehicles accessing construction sites, or to vibrations caused by automobiles, trucks, trains, aircraft, and other similar vehicles when said vehicles are properly maintained and operated and are using properly designated rights-of-way, travelways, flight paths or other routes. Nothing in this Chapter shall preclude the City from abating any vibration problem as per applicable City nuisance and public safety ordinances.
Response: Construction activities are anticipated to cause vibration due to blasting of existing rock to create building industrial sites. The applicant's contractor will seek appropriate permits from the City and Fire District prior to commencing blasting operations. As construction activities are exempt from this chapter, this standard does not apply.

## Chapter 16.150-Air Quality

### 16.150.010-Generally

All otherwise permitted commercial, industrial, and institutional uses shall comply with applicable State air quality rules and statutes:
A. All such uses shall comply with standards for dust emissions as per OAR 340-21-060.
B. Incinerators, if otherwise permitted by Section 16.140.020, shall comply with the standards set forth in OAR 340-25-850 through 340-25-905.
C. Uses for which a State Air Contaminant Discharge Permit is required as per OAR 340-20-140 through 340-20-160 shall comply with the standards of OAR 340-220 through 340-20-276.
Response: While specific users are not known at this time, the applicant intends to comply with applicable air quality standards as required by law. No incinerators are proposed. This standard is met.

### 16.150.020 - Proof of Compliance

Proof of compliance with air quality standards as per Section 16.150 .010 shall be in the form of copies of all applicable State permits, or if permits have not been issued, submission by the applicant, and acceptance by the City, of a report certified by a professional engineer indicating that the proposed use will comply with State air quality standards. Depending on the nature and size of the use proposed, the applicant may, in the City's determination, be required to submit to the City a report or reports substantially identical to that required for issuance of State Air Contaminant Discharge Permits.
Response: Since specific users are not known at this time, it would be more appropriate for the City to request documentation at the time of reviewing and inspecting building permit applications for tenant improvements, rather than at the time of site plan review. This standard does not apply.
16.150.030-Exceptions

Nothing in this Chapter shall preclude the City from abating any air quality problem as per applicable City nuisance and public safety ordinances.
Response: The applicant is aware that the development is subject to the City's nuisance and public safety ordinances. This standard is met.

## Chapter 16.152-Odors

### 16.152.010-Generally

All otherwise permitted commercial, industrial, and institutional uses shall incorporate the best practicable design and operating measures so that odors produced by the use are not discernible at any point beyond the boundaries of the development site.
Response: While specific users are not known at this time, it is not anticipated that the proposed light industrial operations will produce noxious odors discernable at the property line since all operations would occur indoors and any odor-producing activities would be mitigated by appropriate air quality measures. Each facility will have a trash enclosure to contain any odors from waste. This standard is met.

### 16.152.020-Standards

The applicant shall submit a narrative explanation of the source, type and frequency of the odorous emissions produced by the proposed commercial, industrial, or institutional use. In evaluating the potential for adverse impacts from odors, the City shall consider the density and characteristics of surrounding populations and uses, the duration of any odorous emissions, and other relevant factors.
Response: Since specific users are not known at this time, it would be more appropriate for the City to request documentation at the time of reviewing and inspecting building permit applications for tenant improvements, rather than at the time of site plan review. This standard does not apply.

### 16.152.030-Exceptions

Nothing in this Chapter shall preclude the City from abating any odor problem as per applicable City nuisance and public safety ordinances.
Response: The applicant is aware that the development is subject to the City's nuisance and public safety ordinances. This standard is met.

## Chapter 16.154-Heat and Glare

### 16.154.010-Generally

Except for exterior lighting, all otherwise permitted commercial, industrial, and institutional uses shall conduct any operations producing excessive heat or glare entirely within enclosed buildings. Exterior lighting shall be directed away from adjoining properties, and the use shall not cause such glare or lights to shine off site in excess of one-half (0.5) foot candle when adjoining properties are zoned for residential uses.
Response: All operations will be completed indoors and thus will not create heat or visible glare from high temperature processes. No abutting properties are zoned for residential use. This standard is met.

### 16.154.020-Exceptions

Nothing in this Chapter shall preclude the City from abating any heat and glare problem as per applicable City nuisance and public safety ordinances.
Response: The applicant is aware that the development is subject to the City's nuisance and public safety ordinances. This standard is met.

## Chapter 16.156-Energy Conservation

### 16.156.020 - Standards

A. Building Orientation - The maximum number of buildings feasible shall receive sunlight sufficient for using solar energy systems for space, water or industrial process heating or cooling. Buildings and vegetation shall be sited with respect to each other and the topography of the site so that unobstructed sunlight reaches the south wall of the greatest possible number of buildings between the hours of 9:00 AM and 3:00 PM, Pacific Standard Time on December 21st.
Response: All buildings are of suitable size to accommodate solar energy systems, should the owner or tenant choose to implement such as system. Adequate clearance is provided among buildings so that buildings will not cast shade on adjoining structures. Buildings A through D are oriented on an east-west axis which would allow for south-facing solar panels, while Building E is oriented on a north-south axis which would allow for either south- or west-facing solar panels. This standard is met.
B. Wind - The cooling effects of prevailing summer breezes and shading vegetation shall be accounted for in site design. The extent solar access to adjacent sites is not impaired vegetation shall be used to moderate prevailing winter wind on the site.
Response: Based on available weather data from the National Oceanic and Atmospheric Administration (NOAA), the prevailing wind patterns in southwest portion of metropolitan Portland during summer are from the northwest. In winter, they're predominantly from the south.

Passive cooling is possible from the placement of shade trees along the building's north elevation and within the planter strip along SW Tualatin- Sherwood Road. Internal to the site, building placement within the portion of the site proposed for warehousing and light industrial uses will allow prevailing summer breezes to evenly flow through the site. Trees placed along the perimeter of the site and within the parking area will provide ample shading at maturity. In the winter, trees planted along the south and west boundaries of the site and within the proposed parking areas will buffer winds from the south. The site has a considerable amount of protected wetlands on site that will further magnify the effects of shading on site. This standard is met.

### 16.156.030 - Variance to Permit Solar Access

Variances from zoning district standards relating to height, setback and yard requirements approved as per Chapter 16.84 may be granted by the Commission where necessary for the proper functioning of solar energy systems, or to otherwise preserve solar access on a site or to an adjacent site.
Response: The applicant is not seeking any variances to height, setbacks, or yards to accommodate solar energy. This standard does not apply.

## IV. CONCLUSION

Based on the information presented and discussed in this narrative and the attached supporting plans and documentation, this application meets applicable standards necessary for land use approval. The proposed development complies with all applicable standards of the Sherwood Zoning and Community Development Code. The applicant respectfully requests approval by the City.

As the applicant may or may not proceed with the final plat to subdivide the property, the applicant also requests that conditions of approval be specific to each land use approval so it is clear which conditions would not apply in the event that the property remains a single parcel.
Case No.
Fee
Receipt \#
Date
TYPE

## City of Sherwood <br> Application for Land Use Action

## Type of Land Use Action Requested: (check all that apply)

$\square$ Annexation $\qquad$
Planned Unit Development
Site Plan (square footage of building and parking area)
Variance (list standards to be varied in description)

> By submitting this form the Owner, or Owner's authorized agent/ representative, acknowledges and agrees that City of Sherwood employees, and appointed or elected City Officials, have authority to enter the project site at all reasonable times for the purpose of inspecting project site conditions and gathering information related specifically to the project site.

Note: See City of Sherwood current Fee Schedule, which includes the "Publication/Distribution of Notice" fee, at www.sherwoodoregon.gov. Click on Government/Finance/Fee Schedule.

## Owner/Applicant Information:

Applicant: Trammell Crow Company (Att: Kirk Olsen)
Applicant Address: 1300 SW 5th Ave, Suite 3050 Porlland, OR 97201
Phone: ${ }^{(5033)} 644.9400$
Owner: Willamete Water Supply System Commisision (At: Davididraska)
Email: Kolsen@tammellcrow.com
Owner Address: 1850 SW 1701m Ave. Beaverton, OR 97003
Phone: $(503)$ ) 941 -4661
Email: david.kaska@etwdo.org
Contact for Additional Information: Mackenzie (At: Brian Varicachione, bvaricchione@mcknza.com)

## Property Information:

Street Location: Southwest corner of Tualatin-Sherwood Road and 124 th Avenue.
Tax Lot and Map No: 2 S 1280001100
Existing Structures/Use: Vacant Lot
Existing Plan/Zone Designation: Employment Industrial (El)
Size of Property(ies) 46.5 acres

## Proposed Action:

Purpose and Description of Proposed Action:
The applicant proposes to construct five industrial buildings, approximately 535,000 square feet, for future warehousing and industrial uses. The property will be subdivided into five lots and five tracts and includes the construction of SW Cipole Place. The project requests a variance to 16.106 .040 .E. 1 to allow a cul-de-sac over 200 feet long.

Proposed Use: Warehousing and Light Manufacturing Uses
Proposed No. of Phases (one year each):

## Authorizing Signatures:

I am the owner/authorized agent of the owner empowered to submit this application and affirm that the information submitted with this application is correct to the best of my knowledge.

I further acknowledge that I have read the applicable standards for review of the land use action I am requesping and understand that I must demonstrate to the City review authorities compliance with these frand ards prior to approval of my request.

Applicant's Signature


Owner's Signature
Date

The following materials must be submitted with your application or it will not be accepted at the counter. Once taken at the counter, the City has up to 30 days to review the materials submitted to determine if we have everything we need to complete the review. Applicant can verify submittal includes specific materials necessary for the application per checklist.

回 3 Copies of Application Form* completely filled out and signed by the property owner (or person with authority to make decisions on the property.

Copy of Deed to verify ownership, easements, etc.
回 At least 3 folded sets of plans*
At least 3 copies of narrative addressing application criteria*
Fee (along with calculations utilized to determine fee if applicable)
$\square$ Neighborhood Meeting Verification including affidavit, sign-in sheet and meeting summary (required for Type III, IV and V projects)

* Note that the required numbers of copies identified on the checklist are required for completeness; however, upon initial submittal applicants are encouraged to submit only 3 copies for completeness review. Prior to completeness, the required number of copies identified on the checklist and one full electronic copy will be required to be submitted.

January 14, 2020
City of Sherwood
Attention: Joy Chang
22560 SW Pine Street
Sherwood, OR 97140

## Re: T-S Corporate Park

Property Owner Authorization for Land Use Applications

Dear Ms. Chang:
The Willamette Water Supply System Commission ("WWSS Commission") is the owner of the real property described below. Although the WWSS Commission will not be the Applicant, this letter provides written authorization from the property owner for Trammell Crow Company to apply for land use applications for the property, associated with the development of the T-S Corporate Park at the southwest corner of Tualatin-Sherwood Road and 124th Avenue.

## Project Details

Property Owner: Willamette Water Supply System Commission
Tax lot: 2S128D001100
Address: 12822 SW Tualatin-Sherwood Road

If you have any questions about this authorization, please contact me at (503) 941-4561 or feel free to reach out to the WWSS Commission's attorney, Tommy Brooks. Mr. Brooks can be reached at (503) 224-3092.

Sincerely,


David Kraska
General Manager
Willamette Water Supply System Commission

## T-S CORPORATE PARK

12822 SOUTHWEST TUALATIN-SHERWOOD ROAD, SHERWOOD, OR 97062
SITE PLAN REVIEW SET - JANUARY 17, 2020



SITLEE SHEET AND DRAWING INDEX

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PLANTING SCHEDULE

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TRAMMELL CROW COMPANY

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1300 SW 5 SH AVE.
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SUBDMSION AREAS
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##  <br>  <br> MACKENZIE <br>  <br> COMPANY <br> 1300 SW 5TH AVE., <br> STE 3050 PO OR 7201

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## T-S Corporate Park - Traffic Impact Analysis

Date: January 15, 2020
To: Bob Galati, PE, City of Sherwood
Jinde Zhu, PE, Washington County

From: Brian J. Dunn, PE, Kristine Connolly, PE \& Claire Dougherty

CC: $\quad$ Garth Appanaitis, PE - DKS Associates


Project: T-S Corporate Park - Sherwood, Oregon
Subject: Traffic Impact Analysis

This report presents the comprehensive traffic impact analysis (TIA) completed for the proposed T-S Corporate Park development, to be located the southwest quadrant of the SW Tualatin-Sherwood Road and SW $124^{\text {th }}$ Avenue intersection in Sherwood, Oregon. Based on the results of this TIA, the proposed T-S Corporate Park can be developed while maintaining acceptable levels of mobility and safety at the study intersections, assuming provision of the recommended mitigation measures. The primary findings and recommendations of this study are summarized below and in the following sections of this report.

## FINDINGS AND RECOMMENDATIONS

Based on the analysis herein, the following findings and recommendations are associated with the proposed development of the T-S Corporate Park project:

## Year 2019 Existing Conditions

- Crash History:
- The observed crash rates exceed the ODOT published $90^{\text {th }}$ percentile crash rate at three study intersections:
- SW Oregon Street/SW Tualatin-Sherwood Road
- SW $124^{\text {th }}$ Avenue/SW Tualatin-Sherwood Road
- SW 112 ${ }^{\text {th }}$ Avenue-SW Avery Street/SW Tualatin-Sherwood Road
- The ODOT published 2017 Washington County Safety Priority Index System (SPIS) List identifies the study intersection of SW $115^{\text {th }}$ Avenue/ W TualatinSherwood Road, with an SPIS score of 80.23 out of 100 .
- Five study intersections are identified on the Washington County maintained SPIS 2014-2016 list, with ranking and SPIS scores as follows:
- SW $124^{\text {th }}$ Avenue and SW Tualatin-Sherwood Road is ranked $20^{\text {th }}$ on the list, with an SPIS score of 78.3 out of 100; and
- SW 112 ${ }^{\text {th }}$ Avenue-SW Avery Street and SW Tualatin-Sherwood Road is ranked $22^{\text {nd }}$ on the list, with an SPIS score of 78.3 out of 100 ; and
- SW Cipole Road and SW Tualatin-Sherwood Road is ranked $29^{\text {th }}$ on the list, with an SPIS score of 75.7 out of 100; and
- SW Oregon Street and SW Tualatin-Sherwood Road is ranked $30^{\text {th }}$ on the list, with an SPIS score of 75.7 out of 100; and
- SW Langer Farms Parkway and SW Tualatin-Sherwood Road is ranked $146^{\text {th }}$ on the list, with an SPIS score of 42.0 out of 100.
- All study intersections currently operate at levels which meet the jurisdictional mobility standards.
- However, as observed in the field, and reported within the queuing analysis, vehicle queueing is prevalent east-west along the SW Tualatin-Sherwood Road corridor during both AM and PM peak hours, which is indicative of oversaturated conditions.


## Year 2021 Background Traffic Conditions

- This analysis assumed that the Blake Road connection from SW Oregon Street to SW $124^{\text {th }}$ Avenue is in place, with limited re-distributed trips from SW Tualatin-Sherwood Road.
- All study intersections are forecast to operate acceptably and meet jurisdictional mobility standards during the weekday AM and PM peak hours, except:
- The SW Oregon Street / SW Tualatin-Sherwood Road intersection is forecast to operate with a volume to capacity ratio greater than 1.0 during the PM peak hour.


## Proposed Development Plan

- The proposed development of up to 547,220 square-feet of industrial buildings is estimated to generate 1,844 net new weekday daily trips; including 219 net new trips (177 inbound, 42 outbound) during the weekday AM peak hour and 219 net new trips ( 46 inbound, 173 outbound) during the weekday PM peak hour.
- Site access is proposed via an extension of SW Cipole Road into the site, terminating as a local access cul-de-sac.


## Year 2021 Total Traffic Conditions

- This analysis assumed that the Blake Road connection from SW Oregon Street to SW $124^{\text {th }}$ Avenue is in place, with limited re-distributed trips from SW Tualatin-Sherwood Road.
- All study intersections are forecast to operate acceptably and meet the jurisdictional mobility standards during the weekday AM and PM peak hours, except:
- Similar to existing and background traffic conditions, the SW Oregon Street / SW Tualatin-Sherwood Road intersection is forecast to operate with a volume to capacity ratio greater than 1.0 during the PM peak hour.
- Under total traffic conditions only, the SW Oregon Street / SW Tonquin Road intersection is forecast to operate with a volume to capacity ratio greater than 1.0 during the PM peak hour.
- A SimTraffic queuing analysis showed that under year 2021 total traffic conditions, most $95^{\text {th }}$ percentile queues can generally be accommodated by the existing or assumed lane storage capacities. However, east-west queues on SW Tualatin-Sherwood Road may extend to adjacent intersections during peak hours.


## Year 2025 Background Traffic Conditions

- In addition to the Blake Road connection from SW Oregon Street to SW $124^{\text {th }}$ Avenue, the year 2025 background analysis also assumes Washington County's planned and funded widening of SW Tualatin-Sherwood Road to five lanes has been completed.
- All study intersections are forecast to operate acceptably and meet the jurisdictional mobility standards during the weekday AM and PM peak hours, except:
- The SW Oregon Street / SW Tonquin Road intersection is forecast to operate with a volume to capacity ratio greater than 1.0 during the PM peak hour.


## Year 2025 Total Traffic Conditions

- In addition to the Blake Road connection from SW Oregon Street to SW $124^{\text {th }}$ Avenue, the year 2025 total analysis also assumed that SW Tualatin-Sherwood Road has been widened to five lanes.
- All study intersections are forecast to operate acceptably and meet the jurisdictional mobility standards during the weekday AM and PM peak hours, except:
- The SW Oregon Street / SW Tonquin Road intersection is forecast to continue operating with a volume to capacity ratio greater than 1.0 during the PM peak hour. The proposed site traffic contributes $2.36 \%$ of the projected future total traffic through the intersection during the critical PM peak hour.
- A SimTraffic queuing analysis showed that under year 2025 total traffic conditions, most $95^{\text {th }}$ percentile queues can generally be accommodated by the existing or assumed lane storage capacities.


## Supplemental Access Analysis

- Per City of Sherwood request, a supplemental analysis was performed for a potential scenario in which SW Cipole Road would bisect the site and connect to the future Blake Road, rather than terminating as a cul-de-sac.
- A comparison of this scenario to the proposed site access led to the following findings which support limiting SW Cipole Road to a cul-de-sac ending, as proposed, rather than extending it through the site to Blake Road:
- Traffic Operations: Regardless of whether or not SW Cipole Road is extended through the site, the adjacent study intersections are all anticipated to meet the jurisdictional mobility standard. While the extension of SW Cipole Road results in slightly improved operations at the SW Cipole Road / SW Tualatin-Sherwood Road intersection, operations remain the same or slightly deteriorate at the SW $124^{\text {th }}$ Avenue / SW Tualatin-Sherwood Road, SW Cipole Road/Blake Road and SW $124^{\text {th }}$ Avenue / Blake Road intersections. Therefore, there appears to be no significant system-wide benefit to extending SW Cipole Road through the site to connect with the future Blake Road.
- Traffic Safety: A connection to Blake Road would add an access point to the roadway network, introducing conflict. Limiting SW Cipole Road to a cul-de-sac ending would result in fewer unprotected left-turn conflict points on the surrounding roadway network, especially those involving large trucks.


## Recommendations

Based on the analysis provided and documented herein, the proposed development can be constructed while meeting the traffic mobility and safety standards established for the surrounding transportation system, assuming Washington County completes the planned and funded widening of SW TualatinSherwood Road to five lanes by 2025 and the following site traffic impact mitigation measures are made:

- Provide a proportionate cost share allocation towards the future conversion of the SW Tonquin / SW Oregon Street intersection either to a roundabout or signalized intersection.
- Modify the existing traffic signal at the SW Cipole Road / SW Tualatin-Sherwood Road intersection to accommodate the addition of the proposed south leg.
- Provide a northbound left-turn lane with 150 feet of storage exiting the site.

The SW Oregon Street / SW Tualatin-Sherwood Road intersection is anticipated to exceed jurisdictional mobility standards by 2021, with or without the T-S Corporate Park development. However, when SW

Tualatin-Sherwood Road is widening to five lanes by year 2025, the SW Oregon Street / SW TualatinSherwood Road intersection will meet jurisdictional mobility standards. The planned widening will also aid in reducing existing crashes and queuing along SW Tualatin-Sherwood Road. Based on this finding, we are not recommending any mitigation associated with site development at this location.

Additionally, shrubbery and landscaping, as well as above ground utilities and signage should be appropriately located and maintained on-site and at the proposed site access to provide adequate intersection sight distance per City of Sherwood standards.

## INTRODUCTION

The Applicant, Trammell Crow Company, is proposing to develop up to 547,220 square-feet of industrial park on the subject property. The site is currently vacant and is bordered by the recent extension of SW $124^{\text {th }}$ Avenue to the east, SW Tualatin-Sherwood Road to the north, future industrial land uses to the west and a future east-west collector, Blake Road, to the south. The site was recently annexed into the City of Sherwood from unincorporated Washington County.

Figure 1 displays a site vicinity map and Figure 2 displays the proposed site plan. As shown in the site plan figure, SW Cipole Road will be extended into the site from SW Tualatin-Sherwood Road and terminate as a local access cul-de-sac. No site access driveways are planned on SW $124^{\text {th }}$ Avenue.

## Scope of Report

This study evaluates transportation conditions for the following scenarios:

- Year 2019 existing traffic conditions within the study area during the weekday AM and PM peak hours;
- Year 2021 background traffic conditions (without the proposed development) during the weekday AM and PM peak hours, assuming that the future Blake Road connection from SW Oregon Street to SW $124^{\text {th }}$ Avenue is in place;
- Year 2021 total traffic conditions (with full build-out of the proposed development) during the weekday AM and PM peak hours;
- Year 2025 background traffic conditions (without the proposed development) during the weekday AM and PM peak hours, assuming that the Blake Road connection from SW Oregon Street to SW $124^{\text {th }}$ Avenue is in place and that SW Tualatin-Sherwood Road has been widened to five lanes;
- Year 2025 total traffic conditions (with full build-out of the proposed development) during the weekday AM and PM peak hours;
- Supplemental analysis of total traffic conditions for a scenario in which SW Cipole Road bisects the site to connect to Blake Road, per City of Sherwood request.


KITTELSON
\& ASSOCIATES


The following study intersections were identified in a scoping memorandum submitted to the City of Sherwood and Washington County DLUT for review:

- SW Tualatin-Sherwood Road/SW Oregon Street;
- SW Tualatin-Sherwood Road/SW Wildrose Place;
- SW Tualatin-Sherwood Road/SW Cipole Road;
- SW Tualatin-Sherwood Road/SW 124 ${ }^{\text {th }}$ Avenue;
- SW Tualatin-Sherwood Road/SW 120 ${ }^{\text {th }}$ Avenue;
- SW Tualatin-Sherwood Road/SW 115 ${ }^{\text {th }}$ Avenue; and,
- SW Tualatin-Sherwood Road/SW $112^{\text {th }}$ Avenue-SW Avery Street.

After further scoping discussions with the City of Sherwood, the following study intersections were added for analysis:

- SW Tualatin-Sherwood Road/SW Langer Farms Parkway;
- SW Oregon Street/SW Tonquin Road;
- SW Oregon Street/SW Murdock Road;
- Blake Road / SW $124^{\text {th }}$ Avenue (future year only); and,
- Blake Road / SW Cipole Road (supplemental analysis of future year only).

Appendix " $A$ " contains the transportation scoping memorandum prepared for this analysis.

## EXISTING CONDITIONS

This section summarizes the existing characteristics of the transportation system and adjacent land uses in the vicinity of the proposed development, including an inventory of the existing multi-modal transportation facilities, an evaluation of existing intersection operations for motor vehicles at the study intersections, and a summary of recent crash history.

The site vicinity was visited and inventoried in February 2019. At that time, site conditions, adjacent land uses, existing traffic operations, and transportation facilities in the study area were collected. Figure 3 illustrates the existing lane configurations and traffic control devices at each of the study intersections. It should be emphasized that all observations and traffic counts were completed after the SW $124^{\text {th }}$ Avenue extension became operational.

## Site Conditions and Adjacent Land Uses

The proposed site was recently annexed and is now located in the City of Sherwood. The site is currently vacant and is specified as an Employment Industrial (EI) area on the City of Sherwood Zoning Map (Reference 1). The site is bordered by SW Tualatin-Sherwood Road to the north, industrial land uses to the west, SW $124^{\text {th }}$ Avenue to the east, and undeveloped land to the south.

SW LANGER FARMS PKWY/
SW TUALATIN-SHERWOOD RD
SW TUALATIN-SHERWOOD RD
(1)
 SW MURDOCK RD/
SW OREGON ST
(10)

SW OREGON ST SW TUALATIN-SHERWOOD RD


SW TONQUIN RD SW OREGON ST


SW WILDROSE PL/ SW TUALATIN-SHERWOOD RD (3)


SW CIPOLE RD/ BLAKE RD
(SUPPLEMENTAL ANALYSIS) (12)
 SW TUALATIN-SHERWOOD RD (4)


SW 124TH AVE/ BLAKE RD (FUTURE)
(11)
(11)

SW 124TH AVE/
SW TUALATIN-SHERWOOD RD
 SW 120TH AVE/
SW TUALATIN-SHERWOOD RD (6)


SW 115TH AVE/ SW TUALATIN-SHERWOOD RD (7)


SW 112 TH AVE-SW AVERY ST
SW TUALATIN-SHERWOOD RD



## Transportation Facilities

Table 1 summarizes the existing attributes of the key transportation facilities in the study area.
Table 1. Existing Transportation Facilities and Roadway Designations

| Roadway | Functional Classification | Number of Lanes | Posted Speed (mph) | Sidewalks? | Bicycle Lanes? | On-Street Parking? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| SW Tualatin-Sherwood Road | Arterial ${ }^{1}$ | 3 | 45 | Yes | Yes | No |
| SW Langer Farms Parkway | Collector ${ }^{1}$ | 3 | $25-30^{4}$ | Yes | No | No |
| SW Oregon Street | Arterial ${ }^{1}$ | 3 | 35 | Yes | Partial ${ }^{5}$ | No |
| SW Wildrose Place | Local ${ }^{1}$ | 2 | - | Yes | No | No |
| SW Cipole Road | Collector ${ }^{1}$ | 2 | 45 | Partial ${ }^{6}$ | No | No |
| SW 124 ${ }^{\text {th }}$ Avenue | Arterial ${ }^{1}$ | 2-5 | 45 | Partial ${ }^{7}$ | Partial ${ }^{8}$ | No |
| SW $120^{\text {th }}$ Avenue | Commercial/ Industrial Connector ${ }^{2}$ | 2 | - | Partial ${ }^{9}$ | No | No |
| SW 115 ${ }^{\text {th }}$ Avenue | Major Collector ${ }^{2}$ | 3 | - | Yes | Yes | No |
| SW 112 ${ }^{\text {th }}$ Avenue / SW Avery Street | Major Collector ${ }^{3}$ | 2-3 | 35 | Yes | Yes | No |
| SW Tonquin Road | Arterial ${ }^{1}$ | 2 | 45 | No | No | No |
| SW Murdock Road | Arterial ${ }^{1}$ |  | 35 | Partial ${ }^{10}$ | No | Partial ${ }^{11}$ |

${ }^{1}$ Per City of Sherwood Transportation System Plan (Reference 2);
${ }^{2}$ Per 2035 Washington County Transportation System Plan (Reference 3);
${ }^{3}$ Per City of Tualatin Transportation System Plan (Reference 4);
${ }^{4}$ Posted speed limit on SW Langer Farms Parkway is 30 mph north of SW Tualatin-Sherwood Road and 25 mph south of SW Tualatin-Sherwood Road;
${ }^{5}$ A bike lane exists on SW Oregon Street from SW Murdock Road to approximately 800 feet south of SW Tualatin-Sherwood Road;
${ }^{6}$ There is existing sidewalk on the east side of SW Cipole Road, and intermittent sidewalk on the west side;
${ }^{7}$ Sidewalk exists on both sides of SW $124{ }^{\text {th }}$ Avenue, north of SW Tualatin-Sherwood Road. No sidewalk is provided south of SW Tualatin-Sherwood Road;
${ }^{8}$ Striped bicycle lanes are provided along SW $1244^{\text {th }}$ Avenue, north of SW Tualatin-Sherwood Road. South of SW Tualatin-Sherwood Road, 7-foot wide paved shoulders are available to cyclists;
${ }^{9}$ Sidewalk only exists on the east side of SW $120^{\text {th }}$, south of SW Tualatin-Sherwood Road to the first driveway, approximately 275 feet total;
${ }^{10}$ Sidewalk exists only on the west side of SW Murdock Road;
${ }^{11}$ On-street parking is provided on the west side of SW Murdock Road.

## Non-Motorized Facilities

As shown in Table 1, SW Tualatin-Sherwood Road, SW Cipole Road, and SW $124^{\text {th }}$ Avenue, north of SW Tualatin-Sherwood Road, have sidewalks in the immediate site vicinity. Sidewalks are not provided on SW $124^{\text {th }}$ Avenue, south of SW Tualatin-Sherwood Road. Bicycle access within the study area is primarily provided with on-street bicycle lanes. SW Tualatin-Sherwood Road has buffered bicycle lanes. All signalized and roundabout study intersections have marked crosswalks.

## Transit Facilities

Local transit service is currently provided within the site vicinity by TriMet (Reference 5). TriMet Line 97 provides service between Sherwood and the Tualatin WES Station via SW Tualatin-Sherwood Road, Monday through Friday from 6:20 AM to 9:30 AM and 3:10 PM to 7:00 PM on 30-minute headways. Line 97 does not have scheduled service on Saturday or Sunday. Line 97 transit stops are located within 200 feet of the SW Tualatin-Sherwood Road / SW Cipole Road intersection, close to the study site.

TriMet Line 93 provides service between Sherwood and the Tigard Transit Center via SW Sherwood Boulevard, SW Langer Drive, SW Baler Way, and SW Tualatin-Sherwood Road (west of SW Baler Way) Monday through Sunday from 4:30 AM to 1:00 AM on approximately 45-minute headways. The closest Line 93 transit stop is located approximately 1.5 miles west of the study site. Trimet Line 94 follows a similar route, with additional weekday express service from Sherwood and Tigard to Portland City Center.

## Traffic Safety

The reported crash history at the existing study intersections was reviewed to identify potential safety issues. Oregon Department of Transportation (ODOT) provided crash records for the study intersections for the most recently available five-year period, from January 1, 2013 through December 31, 2017. Table 2 summarizes the reported crash data at the study intersections over the five-year period and shows the calculated crash rates per million entering vehicles for each study intersection. Note that the summarized ODOT intersection crash data may not encompass all intersection-related crashes occurring further from the intersection due to corridor congestion. Appendix " $B$ " contains the crash data obtained from ODOT.

Table 2: Intersection Crash History (January 1, 2013 - December 31, 2017)

| \# | Intersection | Collision Type |  |  |  | Severity |  |  | Total Crashes | Crash Rate (per MEV²) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Rear- <br> End | Turning | Angle | Other | PDO ${ }^{1}$ | Injury | Fatal |  |  |
| 1 | SW Langer Farms Parkway/ SW Tualatin-Sherwood Road | 13 | 9 | 1 | - | 11 | 12 | 0 | 23 | 0.52 |
| 2 | SW Oregon Street/ SW TualatinSherwood Road | 16 | 23 | 1 | 1 | 23 | 18 | 0 | 41 | 0.96 |
| 3 | SW Wildrose Place/ SW TualatinSherwood Road | 1 | 3 | - | 1 | 2 | 3 | 0 | 5 | 0.13 |
| 4 | SW Cipole Road/SW TualatinSherwood Road | 14 | 2 | - | 1 | 5 | 12 | 0 | 17 | 0.43 |
| 5 | SW $124^{\text {th }}$ Avenue/SW TualatinSherwood Road | 28 | 3 | - | 1 | 12 | 20 | 0 | 32 | $0.82{ }^{3}$ |
| 6 | SW 120 ${ }^{\text {th }}$ Avenue/SW TualatinSherwood Road | 2 | 1 | - | 1 | 1 | 3 | 0 | 4 | 0.12 |
| 7 | SW $115^{\text {th }}$ Avenue/SW TualatinSherwood Road | 7 | 4 | - | - | 1 | 10 | 0 | 11 | 0.30 |
| 8 | SW 112 ${ }^{\text {th }}$ Avenue-SW Avery Street/SW Tualatin-Sherwood Road | 23 | 9 | 1 | - | 16 | 17 | 0 | 33 | 0.93 |
| 9 | SW Oregon Street/ SW Tonquin Road | 1 | 3 | - | - | 3 | 1 | 0 | 4 | 0.18 |
| 10 | SW Oregon Street/ SW Murdock Road | 1 | - | - | - | 1 | 0 | 0 | 1 | 0.05 |

${ }^{1}$ PDO = Property Damage Only
${ }^{2}$ MEV = Million Entering Vehicles, calculated using 2019 PM peak hour volumes
${ }^{3}$ MEV calculation for SW $124^{\text {th }}$ Avenue / SW Tualatin-Sherwood road intersection does not include counted vehicles to/from the south leg, as that approach opened to traffic in late 2018, and is therefore not represented in crash data.

Table 3 provides a comparison between the calculated crash rates for each intersection and the published $90^{\text {th }}$ percentile crash rates from the Assessment of Statewide Intersection Safety Performance (Reference 6) per ODOT methodology as described in the Analysis Procedure Manual (Reference 7).

Table 3: Intersection Crash Rate Assessment

| \# | Intersection | Total Crashes | 90th Percentile Crash Rate | Observed Crash Rate at Intersection | Observed Crash Rate > 90th Percentile Crash Rate? |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | SW Langer Farms Parkway/SW TualatinSherwood Road | 23 | 0.86 | 0.52 | No |
| 2 | SW Oregon Street/SW Tualatin-Sherwood Road | 41 | 0.86 | 0.96 | Yes |
| 3 | SW Wildrose Place/SW Tualatin-Sherwood Road | 5 | 0.293 | 0.13 | No |
| 4 | SW Cipole Road/SW Tualatin-Sherwood Road | 17 | 0.509 | 0.43 | No |
| 5 | SW 124 ${ }^{\text {th }}$ Avenue/SW Tualatin-Sherwood Road ${ }^{1}$ | 32 | $0.509^{1}$ | 0.82 | Yes |
| 6 | SW 120 ${ }^{\text {th }}$ Avenue/SW Tualatin-Sherwood Road | 4 | 0.290 | 0.12 | No |
| 7 | SW 115 ${ }^{\text {th }}$ Avenue/SW Tualatin-Sherwood Road | 11 | 0.86 | 0.30 | No |
| 8 | SW 112 ${ }^{\text {th }}$ Avenue-SW Avery Street/SW TualatinSherwood Road | 33 | 0.86 | 0.93 | Yes |
| 9 | SW Oregon Street/ SW Tonquin Road | 4 | 0.293 | 0.18 | No |
| 10 | SW Oregon Street/ SW Murdock Road | 1 | $0.509^{2}$ | 0.05 | No |

${ }^{1}$ Compared to 3-leg signalized intersection rate.
${ }^{23}$-leg roundabout rates not published, therefore comparing to 3-leg signalized intersection rate.

As highlighted in Table 3, the observed crash rate exceeds the applicable $90^{\text {th }}$ percentile crash rate at the following study intersections:

- SW Oregon Street/SW Tualatin-Sherwood Road
- SW 124 ${ }^{\text {th }}$ Avenue/SW Tualatin-Sherwood Road
- SW 112 ${ }^{\text {th }}$ Avenue-SW Avery Street/SW Tualatin-Sherwood Road


## SW Oregon Street/SW Tualatin-Sherwood Road

The SW Oregon Street/SW Tualatin-Sherwood Road intersection currently operates permitted-only northbound and southbound left-turn movements. The eastbound/westbound left-turn movements are permitted-protected and incorporate Flashing Yellow Arrow (FYA) operations on the mainline street of SW Tualatin-Sherwood Road.

Of the 41 reported crashes at this intersection, a large component (16) were rear-end related. This type of crash pattern is typical for signalized intersections experiencing heavy traffic demand along arterial corridors, where the stop-and-go effect created by the signal cycles creates vehicle queues that result in rear-end crashes. The frequency of this crash pattern may reduce once SW Tualatin-Sherwood Road is widened from three to five lanes.

Turning type crashes were the most prevalent type of crash reported for this intersection (23), involving left-turns on the mainline and turn movements from the minor street approach. To help reduce this frequency of this crash patterns, it is recommended that Washington County review the signal timing plans and identify possible lengthening of the red clearance times between phases. Also, because SW Tualatin-Sherwood Road only has a single through lane in each direction, left-turn drivers may not be
finding acceptable gaps in oncoming traffic due to heavy demand during the peak travel periods. Once Tualatin-Sherwood Road is widened to five lanes, drivers may find more acceptable gaps to make these left turns without conflict.

## SW 124 ${ }^{\text {th }}$ Avenue/SW Tualatin-Sherwood Road

Though not reflected in the historic crash data, a fourth (northbound) approach was added to the SW $124^{\text {th }}$ Avenue/SW Tualatin-Sherwood Road intersection in late 2018. At that time, additional intersection modifications were made, including permitted-protected left-turn movements with FYA left-turn for all approaches. These improvements, while adding capacity to the overall intersection, may not affect the most prevalent crash pattern, where 28 of the 32 reported crashes were rear-end. However, the frequency of this type of crash may reduce once SW Tualatin Road is widened from three lanes to five lanes and vehicle queues created by the stop-and-go effect of the signal cycles is reduced.

## SW 112 ${ }^{\text {th }}$ Avenue- SW Avery Street/SW Tualatin-Sherwood Road

The SW 112 ${ }^{\text {th }}$ Avenue-SW Avery Street/SW Tualatin-Sherwood Road intersection currently operates protected left-turn phasing northbound and southbound, whereas the eastbound/westbound left-turn movements from Tualatin-Sherwood Road are operated under permitted-protected phasing and incorporate Flashing Yellow Arrow (FYA) operations.

Of the 33 reported crashes at this intersection, the largest component (23) were rear-end related. Like other signalized intersections in the Tualatin-Sherwood Road corridor, this type of crash pattern is common and due to heavy traffic demand along the arterial corridor, where the stop-and-go effect created by the signal cycles creates vehicle queues that result in rear-end crashes. The frequency of this crash pattern may reduce once SW Tualatin-Sherwood Road is widened from three to five lanes.

## ODOT and Washington County SPIS Review

ODOT and Washington County maintain Safety Priority Index System (SPIS) lists to identify existing hazardous intersections for potential safety improvements. The SPIS lists consider the crash data for the 3 prior years. The ODOT-published 2017 Washington County SPIS list (Reference 8) and the Washington County maintained 2014-2016 SPIS list (Reference 9) were reviewed to determine if any study intersections were identified as having an SPIS score in the top 10 percent and ranking amongst other projects. The SPIS score is calculated based on three factors:

- Frequency of crashes ( $25 \%$ of the SPIS score)
- Rate of crashes ( $25 \%$ of the SPIS score)
- Severity of crashes ( $50 \%$ of the SPIS score)


## ODOT Published 2017 Washington County SPIS List

The study intersection of SW $115^{\text {th }}$ Avenue and SW Tualatin-Sherwood Road is identified with an SPIS score of 80.23 out of 100 on the ODOT published Washington County SPIS list. No other study intersections were identified on the ODOT published SPIS list.

## Washington County SPIS List 2014-2016

Five study intersections are identified on the Washington County maintained SPIS 2014-2016 list, with ranking and SPIS scores as follows:

- SW $124^{\text {th }}$ Avenue and SW Tualatin-Sherwood Road is ranked $20^{\text {th }}$ on the list, with an SPIS score of 78.3 out of 100 ; and
- SW $112^{\text {th }}$ Avenue-SW Avery Street and SW Tualatin-Sherwood Road is ranked $22^{\text {nd }}$ on the list, with an SPIS score of 78.3 out of 100; and
- SW Cipole Road and SW Tualatin-Sherwood Road is ranked $29^{\text {th }}$ on the list, with an SPIS score of 75.7 out of 100 ; and
- SW Oregon Street and SW Tualatin-Sherwood Road is ranked $30^{\text {th }}$ on the list, with an SPIS score of 75.7 out of 100; and
- SW Langer Farms Parkway and SW Tualatin-Sherwood Road is ranked $146^{\text {th }}$ on the list, with an SPIS score of 42.0 out of 100 .

As stated previously, the three intersections identified with observed crash rates greater than the ODOT $90^{\text {th }}$ percentile crash rates and the six intersections identified on the ODOT or Washington County SPIS lists will be impacted by Washington County's planned widening of SW Tualatin-Sherwood Road from three lanes to five lanes, which will add capacity to the corridor and provide Washington County with an opportunity for incorporating design elements to improve safety.

## Traffic Operations Analysis Methodology

All level-of-service analyses described in this report were performed in accordance with the procedures stated in the 2000 Highway Capacity Manual (HCM) (Reference 10). The peak 15 -minute flow rates were used in the evaluation of all intersection level-of-service (LOS) and volume-to-capacity (V/C) ratios. For this reason, the analyses reflect conditions that are only likely to occur for the peak 15 minutes out of each average peak hour. Traffic conditions during non-peak weekday hours are expected to operate with lower levels of delay than those described in this report. The signalized and stop-controlled intersection operations analyses presented in this report were completed using Synchro 10 software. The roundabout intersection operations analyses were completed using SIDRA 7 software, based on the procedures stated in the Highway Capacity Manual, 6th Edition (HCM 6th Ed., Reference 11).

## Traffic Operating Standards

Per Section 8 of Sherwood's 2014 Transportation System Plan, "The City target for signalized, all way stop (AWSC), or roundabout intersections is level of service D or volume to capacity ratio equal to or less than 0.85 . The target for unsignalized two way stop control (TWSC) intersections is level of service E or a volume to capacity ratio equal to or less than 0.90 ."

For those streets owned by Washington County or city-owned streets that are labeled on the Arterial and Throughway Network Map of Metro's 2014 Regional Transportation Plan (Reference 12), a Regional 0.99 volume to capacity (V/C) operating standard applies. The Arterial and Throughway Network Map identifies SW Tualatin-Sherwood Road as a Major Arterial and SW Oregon Street as a Minor Arterial. As all existing study intersections are along SW Tualatin-Sherwood Road or SW Oregon Street, the $0.99 \mathrm{~V} / \mathrm{C}$ operating standard will be used. Additionally, as SW $124^{\text {th }}$ Avenue extension is also identified as a Minor Arterial on the Arterial and Throughway Network, the $0.99 \mathrm{~V} / \mathrm{C}$ standard will also be used for the assumed future TWSC intersection of Blake Road and SW $124^{\text {th }}$ Avenue.

## Existing Traffic Operations

Intersection turning-movement counts were conducted at the study intersections when local area schools were in session in February 2019, and after the new extension of SW $124^{\text {th }}$ Avenue was operational. All the weekday counts were conducted on a typical mid-week day during the morning (7:00 to 9:00 AM) and evening (4:00 to 6:00 PM) peak time periods. From the counts, the weekday AM peak hour was found to occur from 7:20 to 8:20 AM and the PM peak hour occurs from 4:45 to 5:45 PM. Appendix " $C$ " contains the February 2019 traffic count worksheets.

Table 4 and Figure 4 summarize the operational analysis for the study intersections under existing traffic conditions for the weekday AM and PM peak hours. As shown, all study intersections currently operate at levels that meet the jurisdictional mobility standards. However, as observed in the field, and reported within the queuing outputs in the Synchro worksheets, vehicle queueing is prevalent in the east-west directions along the SW Tualatin-Sherwood Road corridor during both AM and PM peak hours indicating oversaturated conditions.

Appendix "D" contains the year 2019 existing traffic level-of-service and queuing worksheets.

Table 4: Existing Conditions Operational Analysis Results

| \# | Intersection | LOS $^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Standard | Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 1 | SW Langer Farms Parkway/SW Tualatin-Sherwood Road | C (21.2) | C (26.1) | 0.72 | 0.82 | Regional | V/C of 0.99 | Yes |
| 2 | SW Oregon Street/SW TualatinSherwood Road | B (14.8) | C (28.2) | 0.77 | 0.96 | Regional | V/C of 0.99 | Yes |
| 3 | SW Wildrose Place/SW TualatinSherwood Road | D (25.5) | E (43.5) | 0.03 (SB) | 0.17 (SB) | Regional | V/C of 0.99 | Yes |
| 4 | SW Cipole Road/SW TualatinSherwood Road | A (7.3) | B (15.0) | 0.67 | 0.82 | Regional | V/C of 0.99 | Yes |
| 5 | SW $124^{\text {th }}$ Avenue/SW TualatinSherwood Road ${ }^{1}$ | D (35.9) | C (27.7) | 0.88 | 0.71 | Regional | V/C of 0.99 | Yes |
| 6 | SW $120^{\text {th }}$ Avenue/SW TualatinSherwood Road | D (26.7) | C (19.5) | 0.09 (NB) | 0.10 (NB) | Regional | V/C of 0.99 | Yes |
| 7 | SW 115 ${ }^{\text {th }}$ Avenue/SW TualatinSherwood Road | B (15.9) | B (15.4) | 0.71 | 0.62 | Regional | V/C of 0.99 | Yes |
| 8 | SW 112 ${ }^{\text {th }}$ Avenue-SW Avery Street/SW <br> Tualatin-Sherwood Road | C (24.6) | B (19.5) | 0.74 | 0.61 | Regional | V/C of 0.99 | Yes |
| 9 | SW Oregon Street/ SW Tonquin Road | B (14.2) | E (46.2) | 0.26 | 0.85 (NB) | Regional | V/C of 0.99 | Yes |
| 10 | SW Oregon Street/ SW Murdock Road | A (8.0) | A (8.7) | 0.53 | 0.62 | Regional | V/C of 0.99 | Yes |
| 11 | SW $124^{\text {th }}$ Avenue/Blake Road | N/A | N/A | N/A | N/A | Regional | V/C of 0.99 | N/A |

${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or critical movement delay (TWSC), HCM $6^{\text {th }}$ Ed. Level-of-Service and average delay per vehicle in seconds (roundabout);
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio. For TWSC intersections, the critical movement is shown in parenthesis;
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan (RTFP);
N/A = Not applicable. Intersection does not yet exist.


## TRAFFIC IMPACT ANALYSIS

The future conditions analysis identifies how the transportation facilities within the study area will operate in the proposed project completion year of 2021 and in year 2025, which is the anticipated completion year for the planned widening of SW Tualatin-Sherwood Road. The following elements were analyzed to account for the impacts of the proposed development:

- Year 2021 background traffic conditions (without the proposed development) during the weekday AM and PM peak hours, assuming that the future Blake Road connection from SW Oregon Street to SW $124^{\text {th }}$ Avenue is in place;
- Trips generated by the proposed development and assigned to the street network, with SW Cipole Road terminating as a local access cul-de-sac within the site.
- Year 2021 total traffic conditions (with full build-out of the proposed development) during the weekday AM and PM peak hours;
- Year 2025 background traffic conditions (without the proposed development) during the weekday AM and PM peak hours, assuming that the Blake Road connection from SW Oregon Street to SW $124^{\text {th }}$ Avenue is in place and that SW Tualatin-Sherwood Road has been widened to five lanes;
- Year 2025 total traffic conditions (with full build-out of the proposed development) during the weekday AM and PM peak hours;
- Supplemental analysis of total traffic conditions for a scenario in which SW Cipole Road bisects the site to connect to Blake Road, per City of Sherwood request.


## Year 2021 Background Traffic Conditions

The year 2021 background traffic conditions analysis identifies how the study area's transportation system will operate without the proposed development. This analysis includes trips from traffic attributed to general growth in the region (application of a 1.5 percent annual growth rate), but does not include traffic from the proposed development.

In-process trips from the following developments were also included in the background traffic volumes:

- Parkway Village South (SW Langer Farms Parkway)
- Spring Creek Industrial
- Four-S Corporate Warehouse
- IPT Tualatin
- Majestic SW $115^{\text {th }}$ Avenue Industrial Park
- Hedges C Building
- Tualatin Business Park

Additionally, it was assumed that Blake Road would be in place from SW Oregon Street to SW $124^{\text {th }}$ Avenue, with minor re-distribution of trips from the SW Oregon Street / SW Tualatin-Sherwood Road and SW 124 ${ }^{\text {th }}$ Avenue / SW Tualatin-Sherwood Road intersections.

The future year analyses assume the re-coordination of the traffic signals in the SW Tualatin-Sherwood Road corridor at the SW Cipole Road, SW $124^{\text {th }}$ Avenue, SW $115^{\text {th }}$ Avenue and SW $112^{\text {th }}$ Avenue $/$ SW Avery Street intersections. While existing signal timing parameters provided by Washington County show that during the AM peak hour, the SW Cipole Road and SW $124^{\text {th }}$ Avenue signals operate with a coordinated 120 second cycle length and the SW $115^{\text {th }}$ and SW $112^{\text {th }} /$ SW Avery Street signals operate with a coordinated 140 second cycle, the future years analysis assumed that all four signals would be coordinated with 150 second cycle length during the AM peak, accounting for the addition of the northbound approach at the SW $124^{\text {th }}$ Avenue intersection and regional growth. No cycle length changes were assumed in the future year PM peak hour analysis, as Washington County recently implemented changes at the SW Tualatin-Sherwood Road/SW Cipole Road and SW Tualatin-Sherwood Road/SW $124^{\text {th }}$ Avenue intersections, such that both intersections now operate as fully-actuated, uncoordinated signals, with AutoMax enabled during the PM peak hour. The coordination offset for the other coordinated signals was optimized to account for future traffic patterns.

Figure 5 and Table 5 summarize the operational analysis for the study intersections under the weekday AM and PM peak hour background 2021 traffic conditions. As indicated in Table 5, all study intersections are forecast to operate at levels which meet the jurisdictional mobility standards during both weekday AM and PM peak hours, except for the SW Oregon Street / SW Tualatin-Sherwood Road intersection. However, as noted later in this report, when SW Tualatin-Sherwood Road is widening to five lanes by year 2025, the SW Oregon Street / SW Tualatin-Sherwood Road intersection will meet jurisdictional operating standards.

Appendix "E" contains the year 2021 background traffic level-of-service worksheets, including Figures E1 detailing the in-process trips and E-2 showing the re-distributed Blake Road trips included in the background traffic volumes.


Table 5: Year 2021 Background Conditions Operational Analysis Results

| \# | Intersection | LOS ${ }^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operating <br> Standard | Standard <br> Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 1 | SW Langer Farms Parkway/SW Tualatin-Sherwood Road | A (24.1) | C (32.1) | 0.78 | 0.92 | Regional | V/C of 0.99 | Yes |
| 2 | SW Oregon Street/SW TualatinSherwood Road | B (16.4) | D (35.5) | 0.84 | 1.01 | Regional | V/C of 0.99 | No |
| 3 | SW Wildrose Place/SW TualatinSherwood Road | D (30.2) | F (76.7) | 0.04 (SB) | 0.28 (SB) | Regional | V/C of 0.99 | Yes |
| 4 | SW Cipole Road/SW TualatinSherwood Road | A (8.2) | B (19.4) | 0.71 | 0.89 | Regional | V/C of 0.99 | Yes |
| 5 | SW $124^{\text {th }}$ Avenue/SW TualatinSherwood Road | E (58.1) | C (34.6) | 0.98 | 0.79 | Regional | V/C of 0.99 | Yes |
| 6 | SW $120^{\text {th }}$ Avenue/SW TualatinSherwood Road | D (32.6) | C (22.8) | 0.11 (NB) | 0.13 (NB) | Regional | V/C of 0.99 | Yes |
| 7 | SW 115 ${ }^{\text {th }}$ Avenue/SW TualatinSherwood Road | B (18.2) | B (18.8) | 0.82 | 0.74 | Regional | V/C of 0.99 | Yes |
| 8 | SW 112 ${ }^{\text {th }}$ Avenue-SW Avery Street/SW Tualatin-Sherwood Road | C (31.9) | C (27.3) | 0.82 | 0.77 | Regional | V/C of 0.99 | Yes |
| 9 | SW Oregon Street/ SW Tonquin Road | C (15.2) | F (72.1) | 0.30 | 0.98 | Regional | V/C of 0.99 | Yes |
| 10 | SW Oregon Street/ SW Murdock Road | A (9.0) | A (9.8) | 0.60 | 0.67 | Regional | V/C of 0.99 | Yes |
| 11 | SW 124 ${ }^{\text {th }}$ Avenue / Blake Road | B (12.2) | B (11.2) | 0.05 (EB) | 0.02 (EB) | Regional | V/C of 0.99 | Yes |

${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or critical movement delay (TWSC), HCM 6 ${ }^{\text {th }}$ Ed. Level-of-Service and average delay per vehicle in seconds (roundabout);
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio. For TWSC intersections, the critical movement is shown in parenthesis;
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan (RTFP).

## Proposed Development Plan

The proposed development consists of up to 547,220 square-feet of industrial park. Site access is proposed via an extension of SW Cipole Road into the site, terminating as a local access cul-de-sac. Development is expected to be complete by year 2021.

## Trip Generation

A trip generation estimate for the proposed development was prepared based on the Institute of Transportation Engineers' (ITE) Trip Generation Manual, $10^{\text {th }}$ Edition (Reference 13). Table 6 displays the estimated trip generation for the proposed site, assuming the site is fully developed to a maximum of 547,200 square-feet of industrial park use.

Table 6. Estimated Site Trip Generation

| Land Use Category | ITE Code | Size (SF) | Total Daily Trips | Weekday AM Peak Hour |  |  | Weekday PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Total | In | Out | Total | In | Out |
| Industrial Park | 130 | 547,220 | 1,844 | 219 | 177 | 42 | 219 | 46 | 173 |

Per comments received from the City of Sherwood on the scoping memorandum, weekday peak hour driveway counts were conducted at a similar industrial park development nearby, to confirm that the ITE land use code for Industrial Park would not underestimate trips for the planned development. Counts were collected during peak periods for three consecutive weekdays and analysis showed a trip generation rate of approximately half that of ITE Industrial Park land use code. Therefore, for a conservative analysis, the ITE trip generation as presented in Table 6 was carried forward for the traffic analysis.

## Trip Distribution

Based on a review of general traffic patterns in the region, the proposed land use and external site access patterns, and prior history of our firm's involvement on other development projects in the City of Sherwood, the following site trip distribution was utilized:

- 35 percent to/from the west via SW Tualatin-Sherwood Road,
- 15 percent to/from the southwest via SW Oregon Street,
- 10 percent to/from the southeast via SW $124^{\text {th }}$ Avenue,
- 5 percent to/from the north via Cipole Road,
- 10 percent to/from the north via SW $124^{\text {th }}$ Avenue,
- 10 percent to/from the east via SW $112^{\text {th }}$ Avenue - SW Avery Street, and
- 15 percent to/from the east via SW Tualatin-Sherwood Road.

The trip distribution percentages and trip assignment patterns are shown in Figure 6.
Site truck traffic percentage and distribution was estimated by review of the nearby industrial development driveway counts heavy vehicle percentage and turning movement counts collected at the NE $115^{\text {th }}$ Avenue / SW Tualatin-Sherwood Road intersection. It was estimated that 13 percent of the proposed development traffic would be heavy vehicles during the AM peak hour and 8 percent would be heavy vehicles during the PM peak hour. The east/west directional distribution of heavy vehicles at the NE $115^{\text {th }}$ Avenue / SW Tualatin-Sherwood Road intersection was generally even, therefore the heavy percentages listed above were applied evenly to each movement to and from the study site.

## Year 2021 Total Traffic Conditions

The total traffic conditions analysis identifies how the study area's transportation system will operate with the proposed development trips added to the background traffic volumes. Similar to the background year 2021 analysis, this analysis assumed that Blake Road would be in place from SW Oregon Street to SW $124^{\text {th }}$ Avenue, with limited re-distribution of trips from the SW Oregon Street / SW Tualatin-Sherwood Road and SW $124^{\text {th }}$ Avenue/SW Tualatin-Sherwood Road intersections.


Addition of the site generated trips shown in Figure 6 to the background 2021 volumes in Figure 5 results in the operational characteristics presented in Table 7 and shown in Figure 7. Appendix "F" contains the year 2021 total traffic level-of-service worksheets.

Table 7: Year 2021 Total Traffic Conditions Operational Analysis Results

| \# | Intersection | LOS $^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operating <br> Standard | Standard Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 1 | SW Langer Farms Parkway/SW Tualatin-Sherwood Road | C (25.3) | C (34.0) | 0.81 | 0.94 | Regional | V/C of 0.99 | Yes |
| 2 | SW Oregon Street/SW TualatinSherwood Road | B (19.3) | D (41.8) | 0.86 | 1.09 | Regional | V/C of 0.99 | No |
| 3 | SW Wildrose Place/SW TualatinSherwood Road | E (35.9) | F (134.6) | 0.05 (SB) | 0.42 (SB) | Regional | V/C of 0.99 | Yes |
| 4 | SW Cipole Road/SW TualatinSherwood Road | B (14.7) | C (33.3) | 0.81 | 0.92 | Regional | V/C of 0.99 | Yes |
| 5 | SW $124^{\text {th }}$ Avenue/SW TualatinSherwood Road | E (57.5) | D (35.6) | 0.99 | 0.81 | Regional | V/C of 0.99 | Yes |
| 6 | SW 120 ${ }^{\text {th }}$ Avenue/SW TualatinSherwood Road | D (33.2) | C (23.6) | 0.11 (NB) | 0.13 (NB) | Regional | V/C of 0.99 | Yes |
| 7 | SW 115 ${ }^{\text {th }}$ Avenue/SW TualatinSherwood Road | C (21.2) | C (20.3) | 0.83 | 0.77 | Regional | V/C of 0.99 | Yes |
| 8 | SW 112 ${ }^{\text {th }}$ Avenue-SW Avery Street/SW Tualatin-Sherwood Road | D (36.2) | C (28.8) | 0.83 | 0.79 | Regional | V/C of 0.99 | Yes |
| 9 | SW Oregon Street/ SW Tonquin Road | C (15.5) | F (87.2) | 0.31 (NB) | 1.03 (NB) | Regional | V/C of 0.99 | No |
| 10 | SW Oregon Street/ SW Murdock Road | A (9.3) | B (10.2) | 0.62 | 0.69 | Regional | V/C of 0.99 | yes |
| 11 | Blake Road / SW 124 ${ }^{\text {th }}$ Avenue | B (12.4) | B (11.4) | 0.05 (EB) | 0.02 (EB) | Regional | V/C of 0.99 | Yes |

${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or critical movement delay (TWSC), HCM $6^{\text {th }}$ Ed. Level-of-Service and average delay per vehicle in seconds (roundabout);
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio. For TWSC intersections, the critical movement is shown in parenthesis;
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan (RTFP).

As indicated in Tables 5 and 7, the SW Oregon Street / SW Tualatin-Sherwood Road intersection v/c ratio is anticipated to exceed the jurisdictional operating standard during the PM peak hour, in year 2021 background conditions and with site development. However, as noted later in this report, when SW Tualatin-Sherwood Road is widened to five lanes by year 2025, the SW Oregon Street / SW TualatinSherwood Road intersection will meet jurisdictional operating standards.

Additionally, as highlighted in Table 7, the SW Oregon Street / SW Tonquin Road TWSC intersection v/c ratio is anticipated to exceed the jurisdictional operating standard during the PM peak hour with site development.


Year 2021 Total Traffic - Mitigation
The City of Sherwood Transportation System Plan and Five Year Capital Improvement Plan (CIP, Reference 14) identify the reconstruction of the SW Oregon Street / SW Tonquin Road intersection as a roundabout as a "short-term" improvement. Additionally, Washington County's Transportation Development Tax (TDT) Road Project List (Reference 15) identifies the reconstruction of the SW Oregon Street / SW Tonquin Road intersection as a roundabout in the 2014-2024 timeframe.

However, as the timeframe and funding of the project is unclear, mitigation of the SW Oregon Street / SW Tonquin Road intersection with either the installation of a traffic signal or roundabout was investigated. As summarized in Table 8, the SW Oregon Street / SW Tonquin Road intersection can meet the jurisdictional operating standards as a signalized or roundabout intersection. Appendix " $G$ " contains the year 2021 total traffic conditions mitigation service worksheets for the Oregon/Tonquin intersection.

Table 8: Year 2021 Total Traffic Conditions - Mitigation Operational Analysis Results

| \# | Intersection | LOS $^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operating <br> Standard | Standard Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 9 | SW Oregon Street/ SW Tonquin Road (signal) | A (7.9) | B (10.4) | 0.55 | 0.70 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 9 | SW Oregon Street/ SW Tonquin Road (roundabout) | A (2.7) | B (12.0) | 0.59 | 0.81 | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |

${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or HCM $6{ }^{\text {th }}$ Ed. Level-of-Service and average delay per vehicle in seconds (roundabout);
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio (signalized) or HCM 6 ${ }^{\text {th }}$ Ed. Volume-to-Capacity ratio (roundabout);
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan (RTFP).

## Year 2025 Background Traffic Conditions

The year 2025 background traffic conditions analysis identifies how the study area's transportation system will operate without the proposed development. Similar to the year 2021 background analysis, the year 2025 analysis includes trips from traffic attributed to general growth in the region (application of a 1.5 percent annual growth rate), trips from the in-process developments and some re-distribution of trips, assuming the connection of Blake Road from SW Oregon Street to SW $124^{\text {th }}$ Avenue.

Additionally, the 2025 background analysis accounts for the planned and funded widening of SW Tualatin-Sherwood Road to five lanes, as identified as Project \#318 in the Washington County Major Streets Transportation Improvement Program (MSTIP) 3e (Reference 16). Volumes on SW TualatinSherwood Road and SW $124^{\text {th }}$ Avenue were increased an additional 5 percent on top of regional growth, to account for increased future demand.

Assumed lane configurations are shown in Figure 8 and match the planned widening of SW TualatinSherwood Road, as determined from preliminary design layouts posted on the Washington County project website in August 2019 (Reference 17). Beyond the addition of eastbound and westbound through lanes on SW Tualatin-Sherwood Road, additional improvements anticipated in the year 2025 analyses included:

- An eastbound right-turn lane with 200 feet of storage capacity at the SW Oregon Street / SW Tualatin-Sherwood Road intersection;
- Dual left-turn lanes for the eastbound, westbound and northbound approaches to the SW $124^{\text {th }}$ Avenue / SW Tualatin-Sherwood Road intersection;
- With the lane re-configuration, it was assumed that these movements would become protected-only left turns.
- An eastbound right-turn lane with 130 feet of storage capacity at the SW $115^{\text {th }}$ Avenue / SW Tualatin-Sherwood Road intersection; and
- An eastbound right-turn lane with 300 feet of storage capacity at the SW $112^{\text {th }}$ Avenue - SW Avery St / SW Tualatin-Sherwood Road.

Table 9 and Figure 9 summarize the operational analysis for the study intersections under background 2025 traffic conditions during weekday AM and PM peak hours. As indicated in Table 9, all study intersections except for the SW Oregon Street / SW Tonquin Road intersection are forecast to operate at levels which meet the jurisdictional mobility standards during both weekday AM and PM peak hours. Appendix "H" contains the year 2025 background traffic level-of-service worksheets.

Table 9: Year 2025 Background Conditions Operational Analysis Results

| \# | Intersection | LOS $^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operating <br> Standard | Standard <br> Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 1 | SW Langer Farms Parkway/SW Tualatin-Sherwood Road | B (18.0) | C (24.9) | 0.64 | 0.80 | Regional | V/C of 0.99 | Yes |
| 2 | SW Oregon Street/SW TualatinSherwood Road | B (10.6) | B (16.4) | 0.70 | 0.86 | Regional | V/C of 0.99 | Yes |
| 3 | SW Wildrose Place/SW TualatinSherwood Road | C (16.1) | C (21.2) | 0.03 (SB) | 0.06 (SB) | Regional | V/C of 0.99 | Yes |
| 4 | SW Cipole Road/SW TualatinSherwood Road | A (5.6) | A (9.5) | 0.43 | 0.62 | Regional | V/C of 0.99 | Yes |
| 5 | SW $124^{\text {th }}$ Avenue/SW TualatinSherwood Road | C (32.0) | C (23.6) | 0.64 | 0.60 | Regional | V/C of 0.99 | Yes |
| 6 | SW $120^{\text {th }}$ Avenue/SW TualatinSherwood Road | F (59.5) | C (23.0) | 0.22 | 0.13 | Regional | V/C of 0.99 | Yes |
| 7 | SW 115 ${ }^{\text {th }}$ Avenue/SW TualatinSherwood Road | B (15.8) | B (14.1) | 0.53 | 0.48 | Regional | V/C of 0.99 | Yes |
| 8 | SW 112 ${ }^{\text {th }}$ Avenue-SW Avery Street/SW Tualatin-Sherwood Road | D (39.2) | B (19.6) | 0.62 | 0.53 | Regional | V/C of 0.99 | Yes |
| 9 | SW Oregon Street/ SW Tonquin Road | C (16.0) | F (107.5) | 0.33 (NB) | 1.09 (NB) | Regional | V/C of 0.99 | No |
| 10 | SW Oregon Street/ SW Murdock Road | A (9.9) | B (10.9) | 0.65 | 0.72 | Regional | V/C of 0.99 | Yes |
| 11 | Blake Road / SW 124 ${ }^{\text {th }}$ Avenue | B (12.7) | B (11.6) | 0.05 (EB) | 0.02 (EB) | Regional | V/C of 0.99 | Yes |

[^6]SW LANGER FARMS PKWY/
SW TUALATIN-SHERWOOD RD
SW TUALATIN-SHERWOOD RD


SW MURDOCK RD/
SW OREGON ST


## SW OREGON ST/

 SW TUALATIN-SHERWOOD RD SW TONQUIN RD/ SW OREGON ST
(9)


## SW WILDROSE PLI

 SW TUALATIN-SHERWOOD RD (3)

SW CIPOLE RD BLAKE RD
(SUPPLEMENTAL ANALYSIS)


SW CIPOLERD/ SW TUALATIN-SHERWOOD RD (4)

## SW 124TH AVE/

 SW TUALATIN-SHERWOOD RD (5)SW 120TH AVE/ SW TUALATIN-SHERWOOD RD SW TUALATIN-SHERWOOD RD
(6)


SW 115TH AVE/ SW TUALATIN-SHERWOOD RD (7)


SW 112TH AVE-SW AVERY ST/ SW TUALATIN-SHERWOOD RD (8)



## Year 2025 Total Traffic Conditions

The total traffic conditions analysis identifies how the study area's transportation system will operate with the proposed development trips added to the background traffic volumes. Similar to the background year 2025 analysis, this analysis assumed that Blake Road would be in place from SW Oregon Street to SW $124^{\text {th }}$ Avenue, and assumed the 5 -lane widening of SW Tualatin-Sherwood Road and associated intersection modifications.

Addition of the site trips shown in Figure 7 to the background 2025 volumes in Figure 9 results in the operational results presented in Table 10 and shown in Figure 10. Appendix "I" contains the year 2025 total traffic level-of-service worksheets.

Table 10: Year 2025 Total Traffic Conditions Operational Analysis Results

| \# | Intersection | LOS $^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdictio $\mathrm{n}^{3}$ | Operating <br> Standard | Standa rd Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 1 | SW Langer Farms Parkway/SW Tualatin-Sherwood Road | B (18.3) | C (25.7) | 0.65 | 0.84 | Regional | V/C of 0.99 | Yes |
| 2 | SW Oregon Street/SW TualatinSherwood Road | B (12.1) | B (17.6) | 0.75 | 0.88 | Regional | V/C of 0.99 | Yes |
| 3 | SW Wildrose Place/SW TualatinSherwood Road | C (18.8) | C (23.8) | $\begin{aligned} & 0.02 \\ & (S B) \end{aligned}$ | $\begin{aligned} & 0.06 \\ & (S B) \end{aligned}$ | Regional | V/C of 0.99 | Yes |
| 4 | SW Cipole Road/SW Tualatin-Sherwood Road | A (9.5) | B (14.3) | 0.50 | 0.62 | Regional | V/C of 0.99 | Yes |
| 5 | SW 124 ${ }^{\text {th }}$ Avenue/SW TualatinSherwood Road | C (29.8) | C (24.1) | 0.65 | 0.61 | Regional | V/C of 0.99 | Yes |
| 6 | SW $120^{\text {th }}$ Avenue/SW TualatinSherwood Road | F (61.1) | C (24.0) | $\begin{aligned} & 0.22 \\ & (N B) \end{aligned}$ | $\begin{aligned} & 0.13 \\ & \text { (NB) } \end{aligned}$ | Regional | V/C of 0.99 | Yes |
| 7 | SW $115^{\text {th }}$ Avenue/SW TualatinSherwood Road | B (15.8) | B (14.0) | 0.54 | 0.49 | Regional | V/C of 0.99 | Yes |
| 8 | SW 112 ${ }^{\text {th }}$ Avenue-SW Avery Street/SW Tualatin-Sherwood Road | D (42.4) | B (19.7) | 0.64 | 0.55 | Regional | V/C of 0.99 | Yes |
| 9 | SW Oregon Street/ SW Tonquin Road | C (16.4) | $\begin{gathered} \hline F \\ (129.1) \end{gathered}$ | $\begin{aligned} & \hline 0.34 \\ & \text { (NB) } \end{aligned}$ | $\begin{aligned} & 1.15 \\ & \text { (NB) } \end{aligned}$ | Regional | V/C of 0.99 | No |
| 10 | SW Oregon Street/ SW Murdock Road | B (10.3) | B (11.3) | 0.67 | 0.73 | Regional | V/C of 0.99 | yes |
| 11 | Blake Road / SW 124 ${ }^{\text {th }}$ Avenue | B (12.9) | B (11.7) | $\begin{aligned} & 0.05 \\ & \text { (EB) } \end{aligned}$ | $\begin{aligned} & 0.02 \\ & \text { (EB) } \end{aligned}$ | Regional | V/C of 0.99 | Yes |

${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or critical movement delay (TWSC), HCM $6^{\text {th }}$ Ed. Level-of-Service and average delay per vehicle in seconds (roundabout);
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio. For TWSC intersections, the critical movement is shown in parenthesis;
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan (RTFP).

As highlighted in Table 10, the SW Oregon Street / SW Tonquin Road TWSC intersection v/c ratio is anticipated to exceed the jurisdictional operating standard during the PM peak hour with site development.


Year 2025 Total Traffic - Mitigation
As shown in Tables 9 and 10, the SW Oregon Street / SW Tonquin Road TWSC intersection v/c ratio is anticipated to exceed the jurisdictional operating standard during the PM peak hour in year 2025 background conditions and with site development.

As previously discussed, the timeframe and funding for intersection improvements at the SW Oregon Street / SW Tonquin Road intersection are unclear, therefore mitigation with either the installation of a temporary traffic signal or permanent roundabout was investigated. As summarized in Table 11, the SW Oregon Street / SW Tonquin Road intersection can meet the jurisdictional operating standards as a signalized or roundabout intersection. Appendix " $J$ " contains the year 2025 total traffic mitigation worksheets for the Oregon/Tonquin intersection.

Table 11: Year 2025 Total Traffic Conditions - Mitigation Operational Analysis Results

|  | Intersection | LOS $^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operating <br> Standard | Standard Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 9 | SW Oregon Street/ SW Tonquin Road (signal) | A (8.2) | B (10.9) | 0.58 | 0.73 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 9 | SW Oregon Street/ SW Tonquin Road (roundabout) | A (2.9) | C (15.4) | 0.63 | 0.89 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |

${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or HCM $6{ }^{\text {th }}$ Ed. Level-of-Service and average delay per vehicle in seconds (roundabout);
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio (signalized) or HCM 6 ${ }^{\text {th }}$ Ed. Volume-to-Capacity ratio (roundabout);
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan (RTFP).

## Site Traffic Impact at SW Oregon Street/SW Tonquin Road Intersection

As the SW Oregon Street / SW Tonquin Road TWSC intersection v/c ratio is anticipated to exceed the jurisdictional operating standard during the PM peak hour in year 2021 with site development and by year 2025, with or without site development, this section summarizes the proposed development's relative impact and influence at the intersection, to inform mitigation proportionality discussions.

The percentage of site traffic impact was calculated to show how much of the projected future total traffic at the intersection is attributable to the proposed site development. Table 12 summarizes the estimated number of site trips added, as compared to the future volumes entering at the intersection, and provides an estimate of resulting percentage traffic impact.

Table 12: Estimated Percentage of Site Traffic Impact - SW Oregon Street / SW Tonquin Road Intersection

| \# | Intersection | Site Trips <br> Added to Intersection |  | Intersection Total Entering Trips ${ }^{1}$ |  | Percentage Site Traffic Impact |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM | AM | PM |
| 9 | SW Oregon Street/ SW Tonquin Road | 33 | 33 | 1187 | 1399 | 2.78\% | 2.36\% |

As shown in the table above, the estimated site traffic impact at the intersection ranges from 2.36\% during the PM peak hour to $2.78 \%$ during the AM peak hour.

## Vehicle Queuing Analysis

A $95^{\text {th }}$-percentile vehicle queuing analysis was completed under future build-out years 2021 and 2025. For the SimTraffic analysis, four 15-minute periods were recorded, with the second period representative of the peak 15 -minute period, with the report results averaging five runs. Appendix " $K$ " contains the updated year 2021 total traffic SimTraffic worksheets and Appendix " $L$ " contains the year 2025 total traffic SimTraffic worksheets.

## 2021 Traffic Conditions Vehicle Queuing

As shown in Table 13, under year 2021 total traffic conditions, most $95^{\text {th }}$ percentile queues can generally be accommodated by the existing or assumed lane storage capacities. Eastbound SW Tualatin-Sherwood Road through lane queues may extend to adjacent intersections during the AM peak hour and westbound through lane queues may extend to adjacent intersections during the PM peak hour. In the instances where demand in the striped turn bay storage is exceeded, as measured by the length of the white gore stripe, additional queue storage is available in the adjacent striped median or two-way left-turn lane (TWLTL) area, with the exception of:

- The eastbound right-turn lane at the SW Oregon Street / SW Tualatin-Sherwood Road intersection during the PM peak hour.
- The eastbound right-turn lane $95^{\text {th }}$ percentile queue is estimated at 175 feet during the PM peak, whereas the striped turn bay storage, as measured by the length of the white gore stripe, is 95 feet. Inclusive of the taper length, there is adequate storage to accommodate up to a 175 -foot-long queue before potentially impacting the adjacent bike lane or eastbound through lane.
- The southbound left-turn lane during the AM and PM peak hours and the westbound right-turn lane during the PM peak hour at the SW $124^{\text {th }}$ Avenue / SW Tualatin-Sherwood Road intersection.
- The southbound left-turn lane $95^{\text {th }}$ percentile queues are estimated at $275-325$ feet, whereas the striped turn bay storage, as measured by the length of the white gore stripe, is 240 feet. Inclusive of the taper length, there is adequate storage to accommodate a 300 -foot-long queue before a raised median limits additional storage. There is additional queue storage available in left-most southbound through lane, as only the right-most southbound through lane continues through the intersection.
- The westbound right-turn lane $95^{\text {th }}$ percentile queue is estimated at 425 feet, whereas the striped turn bay storage, as measured by the length of the white gore stripe, is 375 feet. Inclusive of the taper length, there is adequate storage to accommodate a 425 -footlong queue before potentially impacting the adjacent bike lane or westbound through lane.

Table 13: Year 2021 Total Traffic Conditions - 95 ${ }^{\text {th }}$ Percentile Queue Summary

| Intersection | Scenario |  | Eastbound |  |  | Westbound |  |  | Northbound |  |  | Southbound |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right |
| SW Oregon Street / SW TualatinSherwood Road |  | Storage (feet) | $250^{1}$ | 2000 | 95 | $350{ }^{1}$ | 1075 | - | - | $200^{1}$ | $200^{2}$ | 75 | - | - |
|  | Total Traffic Conditions | AM Queue | 50 | 525 | 150 | 250 | 525 | - | - | 225 | 400 | 25 | - | - |
|  |  | PM Queue | 50 | 550 | 175 | 425 | 650 | - | - | 225 | 150 | 50 | - | - |
| SW Cipole <br> Road / <br> SW Tualatin- <br> Sherwood <br> Road |  | Storage (feet) | $360{ }^{1}$ | 1100 | - | 250 | 790 | 125 | 200 | 200 | - | 300 | 725 | - |
|  | Total Traffic Conditions | AM Queue | 300 | 1025 | - | 150 | 225 | 50 | 75 | 75 | - | 175 | 75 | - |
|  |  | PM Queue | 75 | 425 | - | 125 | 825 | 50 | 150 | 125 | - | 125 | 150 |  |
| SW 124 ${ }^{\text {th }}$ <br> Avenue / SW TualatinSherwood Road |  | Storage (feet) | $360^{1}$ | 790 | 350 | 375 | 1180 | 375 | 460 | 1000 | - | $240^{3}$ | 730 | 250 |
|  | Total Traffic Conditions | AM Queue | 300 | 975 | 300 | 125 | 550 | 250 | 275 | 400 | - | 325 | 350 | 75 |
|  |  | PM <br> Queue | 125 | 725 | 350 | 300 | 1200 | 425 | 175 | 200 | - | 275 | 275 | 225 |
| SW 124 ${ }^{\text {th }}$ <br> Avenue / <br> Blake Road |  | Storage (feet) ${ }^{4}$ | 150 | 800 | - | 150 | - | - | 150 | 1000 | - | 150 | - | - |
|  | Total Traffic Conditions | AM Queue | 50 | 75 | - | 25 | - | - | 0 | 0 | - | 50 | - | - |
|  |  | PM Queue | 50 | 50 | - | 50 | - | - | 0 | 25 | - | 25 | - | - |

Notes:
$95^{\text {th }}$ percentile queue lengths are reported in feet and have been rounded up to the nearest car length, assuming one vehicle equals 25 feet;
Bold and highlighted cells indicate $95^{\text {th }}$ percentile queue lengths greater than the storage length;
${ }^{1}$ Storage measured as the length of white gore stripe for turn lane, additional queue storage available in striped median;
${ }^{2}$ Northbound right turn storage measured to first intersection to the south (SW Dahlke Lane), additional storage available to the south of the intersection;
${ }^{3}$ Storage measured as the length of white gore stripe for turn lane, additional queue storage available in left-most southbound through lane, as only the right southbound through lane continues through the intersection;
${ }^{4}$ Storage for future intersection eastbound left-turn lanes assumed to be 150 feet;

## 2025 Traffic Conditions Vehicle Queuing

As detailed in Table 14, under year 2025 total traffic conditions, including the planned widening of SW Tualatin-Sherwood Road, $95^{\text {th }}$ percentile queues can be accommodated by the planned lane configuration storage capacity, with the exception of:

- The southbound left-turn movement at the SW Tualatin-Sherwood Road/SW $124^{\text {th }}$ Avenue intersection during the AM peak hour.
- The southbound left-turn lane $95^{\text {th }}$ percentile queues are estimated at 300 feet, whereas the striped turn bay storage, as measured by the length of the white gore stripe, is 240 feet. Inclusive of the taper length, there is adequate storage to accommodate a 300 -foot-long queue before a raised median limits additional storage. Additional queue storage may be available depending upon ultimate Washington County SW Tualatin-Sherwood Road Widening project intersection lane modifications. No site-generated trips are added to this movement.

Table 14: Year 2025 Total Traffic Conditions - 95 ${ }^{\text {th }}$ Percentile Queue Summary

| Intersection | Scenario |  | Eastbound |  |  | Westbound |  |  | Northbound |  |  | Southbound |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right |
| SW Oregon <br> Street / <br> SW Tualatin- <br> Sherwood <br> Road |  | Storage (feet) | 2501 | 2000 | $200^{2}$ | $350{ }^{1}$ | 1075 | 1075 | - | $200^{1}$ | 2005 | 75 | - | - |
|  | Total Traffic Conditions | AM Queue | 25 | 175 | 75 | 125 | 225 | 250 | - | 150 | 200 | 50 | - | - |
|  |  | PM Queue | 25 | 200 | 125 | 275 | 300 | 300 | - | 150 | 125 | 50 | - | - |
| SW Cipole <br> Road / <br> SW Tualatin- <br> Sherwood <br> Road |  | Storage (feet) | $360{ }^{1}$ | 1100 | 1100 | 250 | 790 | 790 | 200 | 200 | - | 300 | 725 | - |
|  | Total Traffic Conditions | AM Queue | 75 | 150 | 200 | 125 | 150 | 175 | 75 | 50 | - | 150 | 75 | - |
|  |  | PM <br> Queue | 75 | 150 | 200 | 50 | 200 | 200 | 100 | 100 | - | 100 | 100 | - |
| SW $124^{\text {th }}$ <br> Avenue / <br> SW Tualatin- <br> Sherwood <br> Road |  | Storage (feet) ${ }^{3}$ | 250 | 790 | 375 | 375 | 1180 | 375 | 300 | 1000 | 1000 | $240^{4}$ | 730 | 730 |
|  | Total Traffic Conditions | AM Queue | 200 | 425 | 125 | 100 | 250 | 175 | 175 | 200 | 250 | 300 | 225 | 225 |
|  |  | PM Queue | 150 | 325 | 125 | 75 | 275 | 100 | 100 | 100 | 100 | 225 | 175 | 250 |
| SW $124^{\text {th }}$ <br> Avenue / Blake Road |  | Storage (feet) ${ }^{6}$ | 150 | 800 | - | 150 | - | - | 150 | 1000 | - | 150 | - | - |
|  | Total Traffic Conditions | AM Queue | 50 | 75 | - | 25 | - | - | 0 | 25 | - | 50 | - | - |
|  |  | PM Queue | 50 | 50 | - | 50 | - | - | 0 | 0 | - | 25 | - | - |

## Notes:

$95^{\text {th }}$ percentile queue lengths are reported in feet and have been rounded up to the nearest car length, assuming one vehicle equals 25 feet; Bold and highlighted cells indicate $95^{\text {th }}$ percentile queue lengths greater than the storage length;
${ }^{1}$ Storage measured as the length of white gore stripe for turn lane, additional queue storage available in striped median;
${ }^{2}$ Eastbound right-turn lane storage assumed to provide 200 feet of storage per intersection design as posted on Washington County SW TualatinSherwood Road Widening project website.
${ }^{3}$ Dual left-turn lanes for the eastbound, westbound and northbound approaches to the SW $124^{\text {th }}$ Avenue / SW Tualatin-Sherwood Road intersection and revisions to right-turn lane lengths assumed per intersection design as posted on Washington County SW Tualatin-Sherwood Road Widening project website.
${ }^{4}$ Storage measured as the length of existing white gore stripe for turn lane, additional queue storage may be available depending upon ultimate Washington County SW Tualatin-Sherwood Road Widening project intersection modifications.
${ }^{5}$ Storage capacity listed to first industrial driveway, additional storage available south of driveway.
${ }^{6}$ Storage for future intersection eastbound left-turn lanes assumed to be 150 feet.

## Sight Distance

Sight distance was not evaluated for the proposed site access, since it has not yet been completely designed or constructed. The following are recommended to ensure adequate safety and operation at the site internal intersections, roadways and site access intersections:

- All intersections should be designed to ensure adequate sight distance; and
- Shrubbery, weeds, and landscaping near intersections should be designed and maintained to provide adequate sight distance.


## Supplemental Access Analysis

The City of Sherwood requested that a supplemental analysis be performed for a potential scenario in which SW Cipole Road would bisect the site and connect to the future Blake Road, rather than terminating as a cul-de-sac. The same trip distribution was used for this scenario, though routing to and from the site varied. The trip assignment for this alternative access scenario is shown in Figure 11.

The assumed future TWSC intersection of SW Cipole Road and Blake Road for this supplemental analysis was compared to the City of Sherwood unsignalized TWSC intersection standards, under the assumption that properties west of SW $124^{\text {th }}$ Avenue are brought into the City limits of Sherwood as planned.

## 2021 Level-of-Service Analysis - Alternative Access Scenario

Addition of the site trips shown in Figure 11 to the background 2021 volumes in Figure 5 results in the operational characteristics presented in Table 15 and shown in Figure 12. Refer to Table 7 and Figure 7 for a comparison to the proposed site access plan. Appendix " $M$ " contains the year 2021 total traffic alternative access scenario level-of-service worksheets.

Table 15: Year 2021 Total Traffic Conditions - Alternative Access Scenario Operational Analysis Results

| \# | Intersection | LOS $^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operating <br> Standard | Standard Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 1 | SW Langer Farms Parkway/SW Tualatin-Sherwood Road | C (25.3) | C (34.0) | 0.81 | 0.94 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 2 | SW Oregon Street/SW TualatinSherwood Road | B (18.3) | D (40.0) | 0.85 | 1.08 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | No |
| 3 | SW Wildrose Place/SW TualatinSherwood Road | D (34.6) | F (110.8) | 0.05 (SB) | 0.37 (SB) | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 4 | SW Cipole Road/SW TualatinSherwood Road | B (13.2) | C (26.0) | 0.78 | 0.90 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 5 | SW $124^{\text {th }}$ Avenue/SW TualatinSherwood Road | E (58.3) | D (36.2) | 0.99 | 0.81 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 6 | SW $120^{\text {th }}$ Avenue/SW TualatinSherwood Road | D (33.2) | C (23.6) | 0.11 | 0.13 (NB) | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 7 | SW $115^{\text {th }}$ Avenue/SW TualatinSherwood Road | C (21.0) | C (20.3) | 0.83 | 0.77 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 8 | SW $112^{\text {th }}$ Avenue-SW Avery Street/SW Tualatin-Sherwood Road | D (36.3) | C (28.8) | 0.83 | 0.79 | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 9 | SW Oregon Street/ SW Tonquin Road | C (15.5) | F (87.2) | 0.31 (NB) | 1.03 (NB) | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | No |
| 10 | SW Oregon Street/ SW Murdock Road | A (9.3) | B (10.2) | 0.62 | 0.69 | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 11 | SW $124^{\text {th }}$ Avenue / Blake Road | B (12.7) | B (11.5) | 0.06 (EB) | 0.05 (EB) | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 12 | SW Cipole Road / Blake Road | A (9.1) | A (9.2) | 0.02 (SB) | 0.07 (SB) | City of Sherwood | $\begin{gathered} \hline \text { LOS "E" or } \\ \text { V/C of } \\ 0.90 \end{gathered}$ | Yes |

${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized, roundabout) or critical movement delay (TWSC);
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio. For TWSC intersections, the critical movement is shown in parenthesis;
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan (RTFP).
As indicated in Table 15, under year 2021 total traffic conditions, projected study intersection operations do not differ significantly from the performance expected with the proposed access.



## 2025 Level-of-Service Analysis - Alternative Access Scenario

Addition of the site trips shown in Figure 11 to the background 2025 volumes in Figure 9 results in the operational results presented in Table 16 and shown in Figure 13. Appendix " $N$ " contains the year 2025 total traffic alternative access scenario level-of-service worksheets.

Table 16: Year 2025 Total Traffic Conditions - Alternative Access Scenario Operational Analysis Results

| \# | Intersection | LOS $^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operating <br> Standard | Standard <br> Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 1 | SW Langer Farms Parkway/SW Tualatin-Sherwood Road | B (18.3) | C (25.7) | 0.65 | 0.84 | Regional | V/C of 0.99 | Yes |
| 2 | SW Oregon Street/SW TualatinSherwood Road | B (11.4) | B (17.0) | 0.73 | 0.88 | Regional | V/C of 0.99 | Yes |
| 3 | SW Wildrose Place/SW TualatinSherwood Road | C (18.6) | C (23.1) | 0.02 (SB) | 0.06 (SB) | Regional | V/C of 0.99 | Yes |
| 4 | SW Cipole Road/SW TualatinSherwood Road | A (8.6) | B (12.5) | 0.48 | 0.60 | Regional | V/C of 0.99 | Yes |
| 5 | SW $124^{\text {th }}$ Avenue/SW TualatinSherwood Road | C (29.6) | C (24.2) | 0.65 | 0.61 | Regional | V/C of 0.99 | Yes |
| 6 | SW $120^{\text {th }}$ Avenue/SW TualatinSherwood Road | F (61.1) | C (24.0) | 0.22 (NB) | 0.13 (NB) | Regional | V/C of 0.99 | Yes |
| 7 | SW 115 ${ }^{\text {th }}$ Avenue/SW TualatinSherwood Road | B (15.8) | B (14.0) | 0.54 | 0.49 | Regional | V/C of 0.99 | Yes |
| 8 | SW 112 ${ }^{\text {th }}$ Avenue-SW Avery Street/SW Tualatin-Sherwood Road | D (42.4) | B (19.7) | 0.64 | 0.55 | Regional | V/C of 0.99 | Yes |
| 9 | SW Oregon Street/ SW Tonquin Road | C (16.4) | F (129.1) | 0.34 (NB) | 1.15 (NB) | Regional | V/C of 0.99 | No |
| 10 | SW Oregon Street/ SW Murdock Road | B (10.3) | B (11.3) | 0.67 | 0.73 | Regional | V/C of 0.99 | Yes |
| 11 | Blake Road / SW 124 ${ }^{\text {th }}$ Avenue | B (13.2) | B (11.9) | 0.06 (EB) | 0.05 (EB) | Regional | V/C of 0.99 | Yes |
| 12 | SW Cipole Road / Blake Road | A (9.1) | A (9.2) | 0.02 (SB) | 0.07 (SB) | City of Sherwood | $\begin{aligned} & \text { LOS "E" or } \\ & \text { V/C of } 0.90 \end{aligned}$ | Yes |

${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or critical movement delay (TWSC), HCM $6^{\text {th }}$ Ed. Level-of-Service and average delay per vehicle in seconds (roundabout);
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio. For TWSC intersections, the critical movement is shown in parenthesis;
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan (RTFP).
As indicated in Table 16, under year 2025 total traffic conditions, projected study intersection operations do not differ significantly from the performance expected with the proposed access.


## 2021 Traffic Conditions Vehicle Queuing - Alternative Access Scenario

As shown in Table 17, under year 2021 total traffic conditions in the alternative access scenario, projected $95^{\text {th }}$ percentile queues do not differ significantly from the queues expected with the proposed access. Appendix " $K$ " also contains the SimTraffic worksheets for this alternative access scenario.

Table 17: Year 2021 Total Traffic Conditions - Alternative Access Scenario 95 ${ }^{\text {th }}$ Percentile Queue Summary

| Intersection | Scenario |  | Eastbound |  |  | Westbound |  |  | Northbound |  |  | Southbound |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right |
| SW Oregon <br> Street / <br> SW Tualatin- <br> Sherwood <br> Road |  | Storage (feet) | 2501 | 2000 | 95 | $350{ }^{1}$ | 1075 | - | - | $200^{1}$ | 200² | 75 | - | - |
|  | Total Traffic Conditions | AM Queue | 50 | 450 | 150 | 225 | 500 | - | - | 200 | 225 | 50 | - | - |
|  |  | PM Queue | 75 | 600 | 175 | 400 | 600 | - | - | 200 | 125 | 50 | - | - |
| SW Cipole <br> Road / <br> SW Tualatin- <br> Sherwood <br> Road |  | Storage (feet) | $360{ }^{1}$ | 1100 | - | 250 | 790 | 125 | 200 | 200 | - | 300 | 725 | - |
|  | Total Traffic Conditions | AM Queue | 300 | 925 | - | 150 | 225 | 75 | 75 | 75 | - | 325 | 425 | - |
|  |  | PM Queue | 125 | 400 | - | 125 | 725 | 75 | 100 | 100 | - | 125 | 150 | - |
| SW 124 ${ }^{\text {th }}$ <br> Avenue / <br> SW Tualatin- <br> Sherwood <br> Road |  | Storage (feet) | $360{ }^{1}$ | 790 | 350 | 375 | 1180 | 375 | 460 | 1000 | - | $240^{3}$ | 730 | 250 |
|  | Total Traffic Conditions | AM Queue | 350 | 1000 | 275 | 100 | 550 | 325 | 250 | 450 | - | 375 | 650 | 100 |
|  |  | PM <br> Queue | 175 | 600 | 250 | 250 | 900 | 325 | 150 | 200 | - | 300 | 400 | 225 |
| SW $124^{\text {th }}$ <br> Avenue / <br> Blake Road |  | Storage (feet) ${ }^{4}$ | 150 | 800 | - | 150 | - | - | 150 | 1000 | - | 150 | - | - |
|  | Total Traffic Conditions | AM Queue | 50 | 75 | - | 25 | - | - | 25 | 25 | - | 50 | - | - |
|  |  | PM Queue | 50 | 50 | - | 50 | - | - | 25 | 0 | - | 25 | - | - |
| SW Cipole <br> Road / Blake Road |  | Storage (feet) ${ }^{6}$ | 150 | - | - | - | - | - | - | - | - | $300^{6}$ | - |  |
|  | Total Traffic Conditions | AM Queue | 25 | - | - | - | - | - | - | - | - | 50 | - | - |
|  |  | PM Queue | 25 | - | - | - | - | - | - | - | - | 75 | - | - |

Notes:
$95^{\text {th }}$ percentile queue lengths are reported in feet and have been rounded up to the nearest car length, assuming one vehicle equals 25 feet;
Bold and highlighted cells indicate $95^{\text {th }}$ percentile queue lengths greater than the storage length;
${ }^{1}$ Storage measured as the length of white gore stripe for turn lane, additional queue storage available in striped median;
${ }^{2}$ Northbound right turn storage measured to first intersection to the south (SW Dahlke Lane), additional storage available to the south of the intersection;
${ }^{3}$ Storage measured as the length of white gore stripe for turn lane, additional queue storage available in left-most southbound through lane, as only the right southbound through lane continues through the intersection;
${ }^{4}$ Storage for future intersection eastbound left-turn lanes assumed to be 150 feet;

## 2025 Traffic Conditions Vehicle Queuing - Alternative Access Scenario

As detailed in Table 18, under year 2025 total traffic conditions in the alternative access scenario, projected $95^{\text {th }}$ percentile queues do not differ significantly from the queues expected with the proposed access. Appendix " $L$ " also contains the SimTraffic worksheets for this alternative access scenario.

Table 18: Year 2025 Total Traffic Conditions - Alternative Access Scenario 95 ${ }^{\text {th }}$ Percentile Queue Summary

| Intersection | Scenario |  | Eastbound |  |  | Westbound |  |  | Northbound |  |  | Southbound |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right |
| SW Oregon Street / SW TualatinSherwood Road |  | Storage (feet) | 2501 | 2000 | $200^{2}$ | $350{ }^{1}$ | 1075 | 1075 | - | $200^{1}$ | $200^{5}$ | 75 | - | - |
|  | Total Traffic Conditions | AM Queue | 25 | 150 | 75 | 125 | 225 | 250 | - | 150 | 175 | 25 | - | - |
|  |  | PM <br> Queue | 50 | 225 | 150 | 275 | 250 | 250 | - | 175 | 125 | 50 | - | - |
| SW Cipole <br> Road / <br> SW Tualatin- <br> Sherwood <br> Road |  | Storage (feet) | $360{ }^{1}$ | 1100 | 1100 | 250 | 790 | 790 | 200 | 200 | - | 300 | 725 | - |
|  | Total Traffic Conditions | AM Queue | 75 | 150 | 175 | 100 | 175 | 175 | 50 | 50 | - | 125 | 75 | - |
|  |  | PM <br> Queue | 75 | 150 | 175 | 50 | 200 | 225 | 100 | 75 | - | 100 | 100 | - |
| SW $124^{\text {th }}$ <br> Avenue / <br> SW Tualatin- <br> Sherwood <br> Road |  | Storage (feet) ${ }^{3}$ | 250 | 790 | 375 | 375 | 1180 | 375 | 300 | 1000 | 1000 | $240^{4}$ | 730 | 730 |
|  | Total Traffic Conditions | AM Queue | 175 | 350 | 75 | 100 | 275 | 175 | 150 | 225 | 250 | 350 | 375 | 300 |
|  |  | PM <br> Queue | 150 | 325 | 100 | 75 | 275 | 100 | 100 | 100 | 125 | 225 | 200 | 275 |
| SW $124^{\text {th }}$ <br> Avenue / Blake Road |  | Storage (feet) ${ }^{6}$ | 150 | 800 | - | 150 | - | - | 150 | 1000 | - | 150 | - | - |
|  | Total Traffic Conditions | AM Queue | 50 | 75 | - | 25 | - | - | 25 | 25 | - | 50 | - | - |
|  |  | PM <br> Queue | 50 | 50 | - | 50 | - | - | 25 | 25 | - | 25 | - | - |
| SW Cipole Road / Blake Road |  | Storage (feet) | $150^{8}$ | - | - | - | - | - | - | - | - | $300^{9}$ | - | - |
|  | Total Traffic Conditions | AM Queue | 25 | - | - | - | - | - | - | - | - | 50 | - | - |
|  |  | PM <br> Queue | 25 | - | - | - | - | - | - | - | - | 75 | - | - |

Notes:
$95^{\text {th }}$ percentile queue lengths are reported in feet and have been rounded up to the nearest car length, assuming one vehicle equals 25 feet;
Bold and highlighted cells indicate $95^{\text {th }}$ percentile queue lengths greater than the storage length;
${ }^{1}$ Storage measured as the length of white gore stripe for turn lane, additional queue storage available in striped median;
${ }^{2}$ Eastbound right-turn lane storage assumed to provide 200 feet of storage per intersection design as posted on Washington County SW Tualatin-
Sherwood Road Widening project website.
${ }^{3}$ Dual left-turn lanes for the eastbound, westbound and northbound approaches to the SW $124^{\text {th }}$ Avenue / SW Tualatin-Sherwood Road intersection and revisions to right-turn lane lengths assumed per intersection design as posted on Washington County SW Tualatin-Sherwood Road Widening project website.
${ }^{4}$ Storage measured as the length of existing white gore stripe for turn lane, additional queue storage may be available depending upon ultimate Washington County SW Tualatin-Sherwood Road Widening project intersection modifications.
${ }^{5}$ Storage capacity listed to first industrial driveway, additional storage available south of driveway.
${ }^{6}$ Storage for future intersection eastbound left-turn lanes assumed to be 150 feet.

Whether or not SW Cipole Road is extended through the site, the adjacent study intersections are all anticipated to meet the regional mobility standard of $\mathrm{v} / \mathrm{c}$ of 0.99 or less. Nevertheless, while the extension of SW Cipole Road results in slightly improved operations at the SW Cipole Road / SW TualatinSherwood Road intersection, operations remain the same or slightly deteriorate at the SW $124^{\text {th }}$ Avenue / SW Tualatin-Sherwood Road, SW Cipole Road/Blake Road and SW 124 th Avenue / Blake Road intersections. Therefore, there appears to be no significant system-wide benefit to extending SW Cipole Road through the site to connect with the future Blake Road.

In addition to the operational impacts of the SW Cipole Road extension, the impacts on traffic safety should also be considered. A connection to Blake Road would add an access point to the roadway
network, introducing conflict. Were the connection to be made, vehicles (including large trucks) associated with the T-S Corporate Park would enter or leave the site by making unprotected left turns across a collector street (Blake Road) and arterial roadway (124 ${ }^{\text {th }}$ Avenue), whereas, without the connection to Blake Road, left-turning vehicles would have the added protection of traffic signal phasing at both the SW Cipole Road / SW Tualatin-Sherwood Road and SW $124^{\text {th }}$ Avenue/SW Tualatin-Sherwood Road intersections. In our opinion, limiting Cipole Road to a cul-de-sac ending would result in fewer unprotected left-turn conflict points on the surrounding roadway network, especially those involving large trucks.

## Recommendations

Based on the analysis provided and documented herein, the proposed development can be constructed while meeting the traffic mobility and safety standards established for the surrounding transportation system, assuming Washington County completes the planned and funded widening of SW TualatinSherwood Road to five lanes by 2025 and the following site traffic impact mitigation measures are made:

- Provide a proportionate cost share allocation towards the future conversion of the SW Tonquin / SW Oregon Street intersection either to a roundabout or signalized intersection.
- Modify the existing traffic signal at the SW Cipole Road / SW Tualatin-Sherwood Road intersection to accommodate the addition of the proposed south leg.
- Provide a northbound left-turn lane with 150 feet of storage exiting the site.

The SW Oregon Street / SW Tualatin-Sherwood Road intersection is anticipated to exceed jurisdictional mobility standards by 2021, with or without the T-S Corporate Park development. However, when SW Tualatin-Sherwood Road is widening to five lanes by year 2025, the SW Oregon Street / SW TualatinSherwood Road intersection will meet jurisdictional mobility standards. The planned widening will also aid in reducing existing crashes and queuing along SW Tualatin-Sherwood Road. Based on this finding, we are not recommending any mitigation associated with site development at this location.

Additionally, shrubbery and landscaping, as well as above ground utilities and signage should be appropriately located and maintained on-site and at the proposed site access to provide adequate intersection sight distance per City of Sherwood standards.

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## APPENDICES

A. Scoping Memorandum
B. Crash Data
C. Traffic Counts
D. Year 2019 Existing Conditions Worksheets
E. Year 2021 Background Conditions Worksheets
F. Year 2021 Total Traffic Conditions Worksheets
G. Year 2021 Total Traffic Conditions - Mitigation Worksheets
H. Year 2025 Background Conditions Worksheets
I. Year 2025 Total Traffic Conditions Worksheets
J. Year 2025 Total Traffic Conditions - Mitigation Worksheets
K. Year 2021 SimTraffic Queuing Worksheets
L. Year 2025 SimTraffic Queuing Worksheets
M. Year 2021 Total Traffic Conditions - Alternative Access Scenario
N. Year 2025 Total Traffic Conditions - Alternative Access Scenario

## Appendix A Scoping Memorandum

| From： | Garth Appanaitis＜gaa＠dksassociates．com＞ |
| :--- | :--- |
| Sent： | Friday，February 01，2019 4：44 PM |
| To： | Kristine Connolly |
| Cc： | Joy Chang；Bob Galati；Brian Dunn；Clarissa Dougherty |
| Subject： | Re：FW：Sherwood Industrial Park Traffic Study Scope |

Hi Kristine，

Here are some initial comments on the scoping memo．It would be good to have a call next week to run through these since some require additional discussion．Let me know your general availability．

1．Page 1 －TIA Scope－In addition to the other items noted in the Development Code，be sure to include a review of site circulation and an assessment of safe ped crossings for adjacent roads．These items are often overlooked and I wanted to flag them now．
2．Page 5 －Trip Generation－Additional description of the site uses and potential tenant spectrum should be provided to verify that the ITE trip gen category is appropriate．
3．Page 5 －Trip Distribution－No major issues with the initial assumptions，but it would be good to understand how these values may change with the collection of traffic counts．
4．Page 8 －Study intersections－This will be dependent on trip generation and trip distribution（see related comments），but will likely need to add a few intersections，including TS／LFP，Oregon／Tonquin，and Oregon／Murdock
5．Page 8－Queuing－SimTraffic or similar stochastic analysis tools should be used rather than the base queuing estimation within Synchro 10．This may have been intended，but was not clear from the narrative．
6．Page 8 －In process developments－To be provided．
7．Page 9 －Future roadway network．We＇ll need to discuss and clarify the assumed future roadway network．It appears that the Blake Road extension is being proposed to be assumed for the traffic analysis，yet it is an unfunded improvement．
8．Page 9 －Traffic counts－There will likely be some initial normalization and balancing of traffic flows as drivers adjust to the new 124th extension．How will this be addressed in the traffic analysis if the counts are collected soon after the road is opened？

Thanks，
Garth
Garth Appanaitis，PE｜Project Manager｜Portland Planning Group Manager
Phone：（503．243．3500）｜Cell：（971．570．4709）｜gaa＠dksassociates．com


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DKS Associates is an employee－owned company．

[^7]From: Kristine Connolly [kconnolly@kittelson.com](mailto:kconnolly@kittelson.com)
Sent: Tuesday, December 18, 2018 2:39 PM
To: Bob Galati [GalatiB@SherwoodOregon.gov](mailto:GalatiB@SherwoodOregon.gov); Jinde Zhu <Jinde Zhu@co.washington.or.us>
Cc: Brian Dunn [bdunn@kittelson.com](mailto:bdunn@kittelson.com); Olsen, Kirk @ Portland [KOIsen@trammellcrow.com](mailto:KOIsen@trammellcrow.com); Clarissa Dougherty [cdougherty@kittelson.com](mailto:cdougherty@kittelson.com)
Subject: Sherwood Industrial Park Traffic Study Scope

Bob/Jinde,

Attached is our proposed scope of work for the Sherwood Industrial Park project on the southwest corner of TualatinSherwood Road and $124^{\text {th }}$. Please review and let us know if you have any comments or questions on the proposed scope.

Please also advise regarding in-process trips.

Thank you,

Kristine Connolly, PE
Senior Engineer

Kittelson \& Associates, Inc.
Transportation Engineering / Planning
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Portland OR 97204
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Streetwise Twitter Facebook

## SCOPING MEMORANDUM

| Date: | December 18, 2018 | Project \#: 23278 |
| :--- | :--- | :--- |
| To: | Bob Galati, City of Sherwood <br>  <br> Jinde Zhu, PE, Washington County |  |
| From: | Brian Dunn, PE, Kristine Connolly, PE, \& Claire Dougherty |  |
| Project: | Sherwood Industrial Park |  |
| Subject: | Traffic Impact Study Scoping Memorandum |  |
|  |  |  |

This memorandum represents a scoping needs assessment for preparing the Traffic Impact Study (TIS) associated with the proposed Sherwood Industrial Park development located on the southwest corner of the SW Tualatin Sherwood Road/SW $124^{\text {th }}$ Avenue intersection in Washington County, OR (soon to be Sherwood). The assumptions for scoping the TIS are based on discussions between the City of Sherwood and the Applicant, our review of the conceptual site plans, and our working knowledge of the transportation policies of the City of Sherwood.

## TRAFFIC IMPACT ANALYSIS

Kittelson and Associates, Inc. (KAI) will prepare a TIS per the requirements enumerated in Sherwood's Development Code Section 16.106.080, Washington County's Resolution \& Order 86-95, and scoping direction received from the City and County staff. Key assumptions are outlined in the remainder of this document.

## Proposed Development

The Applicant, Trammell Crow Company, is in the process of preparing an application to develop 547,220 square feet of industrial buildings on the subject property. The site is currently vacant and is bordered by the SW $124^{\text {th }}$ Avenue future extension to the east, and shopping centers to the north, industrial land uses to the west and a future east-west collector, Blake Road, to the south.

Figure 1 displays a site vicinity map and Figures 2 and 3 display two proposed site plan alternatives. The site plan as shown in Figure 2 details a possible extension of Cipole Road into the site terminating as a private cul-de-sac within the subject site, whereas Figure 3 shows Cipole Road bisecting the site as a public street, extending to intersect with the future Blake Road. As shown in both site plans, no site access driveways are planned on NE $124^{\text {th }}$ Avenue.




## Trip Generation

A preliminary trip generation estimate for the proposed development was prepared based on the Institute of Transportation Engineers' (ITE) Trip Generation Manual, $10^{\text {th }}$ Edition. Table 1 displays the preliminary trip generation for the proposed site.

Table 1. Preliminary Trip Generation Estimate

| Land Use Category | ITE Code | Size (SF) | Total Daily Trips | Weekday AM Peak Hour |  |  | Weekday PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Total | In | Out | Total | In | Out |
| Industrial Park | 130 | 547,200 | 1844 | 219 | 177 | 42 | 219 | 46 | 173 |

## Trip Distribution

Based on a review of general traffic patterns in the region, the proposed land use and external site access patterns, and prior history of our firm's involvement on other development projects in the City of Sherwood, the following site trip distributions are proposed for each site plan scenario:

## Cipole Road Cul-de-sac Site Plan

- 15 percent to/from the west via SW Tualatin-Sherwood Road,
- 10 percent to/from the southwest via SW Oregon Street,
- 5 percent to/from the north via Cipole Road,
- 15 percent to/from the north via SW $124^{\text {th }}$ Avenue,
- 15 percent to/from the south via the SW $124^{\text {th }}$ Avenue extension,
- 10 percent to/from the east via SW $112^{\text {th }}$ Avenue - SW Avery Street,
- 30 percent to/from the east via SW Tualatin-Sherwood Road.


## Cipole Road Connection to Blake Road Site Plan

- 10 percent to/from the west via SW Tualatin-Sherwood Road,
- 5 percent to/from the southwest via SW Oregon Street,
- 10 percent to/from the southwest via the future Blake Road,
- 20 percent to/from the southeast via the future Blake Road and SW $124^{\text {th }}$ Avenue extension,
- 5 percent to/from the north via Cipole Road,
- 15 percent to/from the north via SW $124^{\text {th }}$ Avenue,
- 10 percent to/from the east via SW $112^{\text {th }}$ Avenue - SW Avery Street,
- 25 percent to/from the east via SW Tualatin-Sherwood Road.

The preliminary trip distribution patterns for each site plan concept are displayed in Figures 4 and 5 for informational purposes. The estimated patterns shown in these figures represent our best guess and are subject to change pending collection of new traffic counts and technical analysis needed to prepare the TIS.



## Study Area and Intersections

Based on the estimated trip generation and assignment patterns, the following intersections and accesses are proposed for analysis:

1. SW Tualatin-Sherwood Road/SW Oregon Street
2. SW Tualatin-Sherwood Road/SW Wildrose Place
3. SW Tualatin-Sherwood Road/SW Cipole Road - Proposed Site Access
4. SW Tualatin-Sherwood Road/SW $124^{\text {th }}$ Avenue
5. SW Tualatin-Sherwood Road/ SW $120^{\text {th }}$ Avenue
6. SW Tualatin-Sherwood Road/SW 115 ${ }^{\text {th }}$ Avenue
7. SW Tualatin-Sherwood Road/SW 112 ${ }^{\text {th }}$ Avenue - SW Avery Street

## Time Periods for Analysis

Existing and estimated build-out year 2021 and future year 2023 operating conditions and $95^{\text {th }}$-percentile queuing conditions at the identified study intersections will be analyzed using Synchro Version 10 software only. The weekday AM (7:00 AM to 9:00 AM) and weekday PM (4:00 PM to 6:00 PM) peak hours will be assessed.

## Operating Standards

Per Section 8 of The City of Sherwood Transportation System Plan (adopted 2014), "The City target for signalized, all way stop (AWSC), or roundabout intersections is level of service D or volume to capacity ratio equal to or less than 0.85 . The target for unsignalized two way stop control (TWSC) intersections is level of service E or a volume to capacity ration equal to or less than 0.90 ."

As SW Tualatin-Sherwood Road is designated on the Arterial and Throughway Network, and all proposed study intersections are along SW Tualatin-Sherwood Road, the 0.99 volume to capacity operating standard will be used. This is consistent with the Washington County operating targets.

## In-Process Developments and Planned Transportation Improvements

We anticipate a 1.5 percent annual growth rate can be applied to existing traffic to generate future background traffic volumes on the surrounding street network before any trips associated with approved in-process developments are added to the background traffic volumes. This growth rate is consistent with other previous traffic impact studies in the area.

In-process developments that KAI is aware of include the IPT Tualatin development north of the SW Tualatin-Sherwood/ $124^{\text {th }}$ Avenue Intersection and Majestic Properties development at the south end of SW $115^{\text {th }}$ Avenue. We request that the City of Sherwood provide the trip estimates and assignments for any additional developments in the site vicinity to be included as in-process.

The City of Sherwood Transportation System Plan (TSP) and Capital Improvement Plan (CIP) identify the following projects in the study area vicinity:

- Extension of SW $124^{\text {th }}$ Avenue, from SW Tualatin Sherwood Road to SW Grahams Ferry Road, which is currently under construction and expected to open to traffic by January 1, 2019;
- Tonquin Employment Area East/West Collector (Blake Road) -A future collector street connecting SW Oregon Street to the SW $124^{\text {th }}$ Avenue Extension. The project is listed as unfunded through FY23 in the CIP and as a low priority.
- Widening of SW Tualatin-Sherwood Road - Design is underway to widen the existing three lane arterial road to five lanes, with bicycle facilities. The estimated completion date is end of 2023.

For the TIS build-out year 2021 analysis, only the SW $124^{\text {th }}$ Avenue Extension and future Blake Road will be considered in the roadway network.

For the TIS future year 2023 analysis, the SW $124^{\text {th }}$ Avenue Extension, future Blake Road and widening of SW Tualatin-Sherwood Road will be considered in the roadway network.

No other funded transportation improvements have been identified or are anticipated in the study within the development timeline of this project.

## Crash Analysis

The most recent five years of reported crash data at the study intersections will be requested from ODOT and reviewed in detail. The ODOT Statewide Priority Index System (SPIS) will also be reviewed to identify any sites where safety issues may encourage further investigation.

## Signal Timing

We have downloaded the latest signal timing and phasing information for the five signalized intersections from the Washington County GIS Traffic Plans portal. We request that Washington County provide the signal phasing and timing plan that will be implemented at with the SW Tualatin-Sherwood Road/SW $124^{\text {th }}$ Avenue intersection upon opening of the $124^{\text {th }}$ Avenue extension, if available.

## Analysis Scenarios

The following analysis scenarios will be included in the TIS analysis:

## Existing Conditions - Year 2019

Traffic counts will be collected in mid-January 2019, once the $124^{\text {th }}$ Ave Extension is open and schools are back in session.

## Background Conditions - Year 2021

In this analysis, it will be assumed that the Blake Road connection from SW Oregon Street to SW $124^{\text {th }}$ Avenue is in place, with limited re-distributed trips from SW $124^{\text {th }}$ Avenue.

## Total Traffic Conditions - Year 2021 Cipole Cul-de-sac

In this analysis, it will be assumed that Blake Road connection to SW $124^{\text {th }}$ Avenue is in place, but that SW Cipole Road terminates as a cul-de-sac within the project site.

## Total Traffic Conditions - Year 2021 Cipole Road Extension to Blake Road

In this analysis, it will be assumed that the Blake Road connection to SW $124^{\text {th }}$ Avenue is in place, and that SW Cipole Road bisects the project site, connecting to Blake Road as a TWSC intersection.

## Background Conditions - Year 2023

In this analysis, it will be assumed that the Blake Road connection from SW Oregon Street to SW $124^{\text {th }}$ Avenue is in place, with limited re-distributed trips from SW $124^{\text {th }}$ Avenue. It will also be assumed that SW Tualatin-Sherwood Road has been widened to 5 lanes.

## Total Traffic Conditions - Year 2023 Cipole Cul-de-sac

In this analysis, it will be assumed that Blake Road connection to SW $124^{\text {th }}$ Avenue is in place and SW Tualatin-Sherwood Road has been widened, but that SW Cipole Road terminates as a cul-desac within the project site.

## Total Traffic Conditions - Year 2023 Cipole Road Extension to Blake Road

In this analysis, it will be assumed that the Blake Road connection to SW $124^{\text {th }}$ Avenue is in place, SW Tualatin-Sherwood Road has been widened, and that SW Cipole Road bisects the project site, connecting to Blake Road as a TWSC intersection.

## Next Steps

We trust this memorandum provides adequate documentation of the proposed land use action, estimated site trip generation and distribution patterns, and specific study intersections and analysis periods to address in the TIS. We formally request that the City of Sherwood and Washington County provide written confirmation and/or questions regarding the proposed methodology and project TIS assumptions as soon as possible so that we may proceed with our analysis. If you have any questions, please give us a call at (503) 228-5230.

## Appendix B Crash Data

Intersectional Crashes at SWTualatin-Sherwood Rd \& SW Oregon St

$$
\text { January 1, } 2013 \text { through December 31, } 2017
$$

| COLLISION TYPE | FATAL CRASHES | NON- <br> FATAL CRASHES | PROPERTY DAMAGE ONLY | TOTAL CRASHES | PEOPLE <br> KILLED | PEOPLE INJURED | TRUCKS | $\begin{aligned} & \text { DRY } \\ & \text { SURF } \end{aligned}$ | WET <br> SURF | DAY | DARK | INTERSECTION | INTERSECTION RELATED | $\begin{aligned} & \text { OFF- } \\ & \text { ROAD } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| YEAR: 2017 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 3 | 2 | 5 | 0 | 4 | 0 | 2 | 3 | 5 | 0 | 5 | 0 | 0 |
| TURNING MOVEMENTS | 0 | 3 | 6 | 9 | 0 | 7 | 4 | 8 | 1 | 6 | 3 | 9 | 0 | 0 |
| 2017 TOTAL | 0 | 6 | 8 | 14 | 0 | 11 | 4 | 10 | 4 | 11 | 3 | 14 | 0 | 0 |
| YEAR: 2016 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| HEAD-ON | 0 | 1 | 0 | 1 | 0 | 2 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 0 |
| REAR-END | 0 | 2 | 1 | 3 | 0 | 2 | 0 | 2 | 1 | 3 | 0 | 3 | 0 | 0 |
| TURNING MOVEMENTS | 0 | 3 | 3 | 6 | 0 | 4 | 2 | 3 | 3 | 5 | 1 | 6 | 0 | 0 |
| 2016 TOTAL | 0 | 6 | 4 | 10 | 0 | 8 | 2 | 6 | 4 | 8 | 2 | 10 | 0 | 0 |
| YEAR: 2015 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 1 | 2 | 3 | 0 | 1 | 0 | 3 | 0 | 3 | 0 | 3 | 0 | 0 |
| TURNING MOVEMENTS | 0 | 2 | 0 | 2 | 0 | 5 | 0 | 2 | 0 | 1 | 1 | 2 | 0 | 0 |
| 2015 TOTAL | 0 | 3 | 2 | 5 | 0 | 6 | 0 | 5 | 0 | 4 | 1 | 5 | 0 | 0 |
| YEAR: 2014 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 0 | 2 | 2 | 0 | 0 | 0 | 2 | 0 | 2 | 0 | 2 | 0 | 0 |
| TURNING MOVEMENTS | 0 | 1 | 0 | 1 | 0 | 4 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 0 |
| 2014 TOTAL | 0 | 1 | 2 | 3 | 0 | 4 | 0 | 3 | 0 | 2 | 1 | 3 | 0 | 0 |
| YEAR: 2013 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ANGLE | 0 | 1 | 0 | 1 | 0 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| REAR-END | 0 | 1 | 2 | 3 | 0 | 2 | 0 | 2 | 1 | 3 | 0 | 3 | 0 | 0 |
| TURNING MOVEMENTS | 0 | 0 | 5 | 5 | 0 | 0 | 1 | 5 | 0 | 4 | 1 | 5 | 0 | 0 |
| 2013 TOTAL | 0 | 2 | 7 | 9 | 0 | 4 | 1 | 8 | 1 | 8 | 1 | 9 | 0 | 0 |
| FINAL TOTAL | 0 | 18 | 23 | 41 | 0 | 33 | 7 | 32 | 9 | 33 | 8 | 41 | 0 | 0 |

Disclaimers: Effective 2016, collection of "Property Damage Only" (PDO) crash data elements was reduced for vehicles and participants. Age, Gender, License, Error and other elements are no longer available for PDO crash reporting. Please keep this in mind when comparing 2016 PDO crash data to prior years.

A higher number of crashes may be reported as of 2011 compared to prior years. This does not necessarily reflect an increase in annual crashes. The higher numbers may result from a change to an internal departmental process that allows the Crash Analysis and Reporting Unit to add previously unavailable, non-fatal crash reports to the annual data file. Please be aware of this change when comparing pre-2011 crash statistics. For all disclaimers,
see https://www.oregon.gov/ODOT/Data/documents/Crash_Data_Disclaimers.pdf.

Intersectional Crashes at SW Oregon St \& SW Murdock Rd
January 1, 2013 through December 31, 2017

| COLLISION TYPE | FATAL CRASHES | NON- <br> FATAL <br> CRASHES | PROPERTY DAMAGE ONLY | TOTAL CRASHES | PEOPLE <br> KILLED | PEOPLE <br> INJURED | TRUCKS | $\begin{gathered} \text { DRY } \\ \text { SURF } \end{gathered}$ | $\begin{aligned} & \text { WET } \\ & \text { SURF } \end{aligned}$ | DAY | DARK | INTERSECTION | INTERSECTION RELATED | $\begin{aligned} & \text { OFF- } \\ & \text { ROAD } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| YEAR: 2014 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| 2014 TOTAL | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| FINAL TOTAL | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |

Disclaimers: Effective 2016, collection of "Property Damage Only" (PDO) crash data elements was reduced for vehicles and participants. Age, Gender, License, Error and other elements are no longer available for PDO crash reporting. Please keep this in mind when comparing 2016 PDO crash data to prior years.

A higher number of crashes may be reported as of 2011 compared to prior years. This does not necessarily reflect an increase in annual crashes. The higher numbers may result from a change to an internal departmental process that allows the Crash Analysis and Reporting Unit to add previously unavailable, non-fatal crash reports to the annual data file. Please be aware of this change when comparing pre-2011 crash statistics. For all disclaimers
see https://www.oregon.gov/ODOT/Data/documents/Crash_Data_Disclaimers.pdf.

Intersectional Crashes at SW Oregon St \& SW Tonquin Rd
January 1, 2013 through December 31, 2017

| COLLISION TYPE | FATAL CRASHES | NON- <br> FATAL CRASHES | PROPERTY DAMAGE ONLY | TOTAL CRASHES | PEOPLE <br> KILLED | PEOPLE INJURED | TRUCKS | DRY SURF | WET <br> SURF | DAY | DARK | INTERSECTION | INTERSECTION RELATED | $\begin{aligned} & \text { OFF- } \\ & \text { ROAD } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| YEAR: 2017 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| TURNING MOVEMENTS | 0 | 1 | 1 | 2 | 0 | 2 | 0 | 2 | 0 | 2 | 0 | 2 | 0 | 0 |
| 2017 TOTAL | 0 | 1 | 1 | 2 | 0 | 2 | 0 | 2 | 0 | 2 | 0 | 2 | 0 | 0 |
| YEAR: 2015 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| 2015 TOTAL | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| YEAR: 2013 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| TURNING MOVEMENTS | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| 2013 TOTAL | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| FINAL TOTAL | 0 | 1 | 3 | 4 | 0 | 2 | 0 | 4 | 0 | 4 | 0 | 4 | 0 | 0 |

Disclaimers: Effective 2016, collection of "Property Damage Only" (PDO) crash data elements was reduced for vehicles and participants. Age, Gender, License, Error and other elements are no longer available for PDO crash reporting. Please keep this in mind when comparing 2016 PDO crash data to prior years.

A higher number of crashes may be reported as of 2011 compared to prior years. This does not necessarily reflect an increase in annual crashes. The higher numbers may result from a change to an internal departmental process that allows the Crash Analysis and Reporting Unit to add previously unavailable, non-fatal crash reports to the annual data file. Please be aware of this change when comparing pre-2011 crash statistics. For all disclaimers
see https://www.oregon.gov/ODOT/Data/documents/Crash Data Disclaimers.pdf.

Intersectional Crashes at SW Tualatin-Sherwood Rd \& SW 112th Ave / SW Avery St January 1, 2013 through December 31, 2017

| COLLISION TYPE | FATAL CRASHES | NON- <br> FATAL CRASHES | PROPERTY DAMAGE ONLY | TOTAL CRASHES | PEOPLE <br> KILLED | PEOPLE <br> INJURED | TRUCKS | $\begin{aligned} & \text { DRY } \\ & \text { SURF } \end{aligned}$ | WET <br> SURF | DAY | DARK | INTERSECTION | INTERSECTION RELATED | $\begin{aligned} & \text { OFF- } \\ & \text { ROAD } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| YEAR: 2017 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 2 | 1 | 3 | 0 | 3 | 0 | 2 | 1 | 2 | 1 | 3 | 0 | 0 |
| TURNING MOVEMENTS | 0 | 1 | 5 | 6 | 0 | 1 | 0 | 2 | 4 | 5 | 1 | 6 | 0 | 0 |
| 2017 TOTAL | 0 | 3 | 6 | 9 | 0 | 4 | 0 | 4 | 5 | 7 | 2 | 9 | 0 | 0 |
| YEAR: 2016 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 2 | 3 | 5 | 0 | 2 | 0 | 3 | 2 | 4 | 1 | 5 | 0 | 0 |
| TURNING MOVEMENTS | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 0 |
| 2016 TOTAL | 0 | 2 | 4 | 6 | 0 | 2 | 0 | 3 | 3 | 4 | 2 | 6 | 0 | 0 |
| YEAR: 2015 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 1 | 1 | 2 | 0 | 2 | 0 | 1 | 0 | 2 | 0 | 2 | 0 | 0 |
| 2015 TOTAL | 0 | 1 | 1 | 2 | 0 | 2 | 0 | 1 | 0 | 2 | 0 | 2 | 0 | 0 |
| YEAR: 2014 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ANGLE | 0 | 1 | 0 | 1 | 0 | 5 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 0 |
| REAR-END | 0 | 9 | 3 | 12 | 0 | 20 | 0 | 6 | 5 | 11 | 1 | 12 | 0 | 0 |
| 2014 TOTAL | 0 | 10 | 3 | 13 | 0 | 25 | 0 | 7 | 5 | 11 | 2 | 13 | 0 | 0 |
| YEAR: 2013 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 1 | 0 | 1 | 0 | 3 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| TURNING MOVEMENTS | 0 | 0 | 2 | 2 | 0 | 0 | 0 | 2 | 0 | 2 | 0 | 2 | 0 | 0 |
| 2013 TOTAL | 0 | 1 | 2 | 3 | 0 | 3 | 0 | 3 | 0 | 3 | 0 | 3 | 0 | 0 |
| FINAL TOTAL | 0 | 17 | 16 | 33 | 0 | 36 | 0 | 18 | 13 | 27 | 6 | 33 | 0 | 0 |

Disclaimers: Effective 2016, collection of "Property Damage Only" (PDO) crash data elements was reduced for vehicles and participants. Age, Gender,
License, Error and other elements are no longer available for PDO crash reporting. Please keep this in mind when comparing 2016 PDO crash data to prior years.

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Intersectional Crashes at SW Tualatin-Sherwood Rd \& SW 115th Ave

$$
\text { January 1, } 2013 \text { through December 31, } 2017
$$

| COLLISION TYPE | FATAL CRASHES | NON- <br> FATAL <br> CRASHES | PROPERTY DAMAGE ONLY | TOTAL CRASHES | PEOPLE <br> KILLED | PEOPLE <br> INJURED | TRUCKS | $\begin{aligned} & \text { DRY } \\ & \text { SURF } \end{aligned}$ | WET <br> SURF | DAY | DARK | INTERSECTION | INTERSECTION RELATED | $\begin{aligned} & \text { OFF- } \\ & \text { ROAD } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| YEAR: 2017 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 2 | 0 | 2 | 0 | 3 | 0 | 1 | 1 | 2 | 0 | 2 | 0 | 0 |
| 2017 TOTAL | 0 | 2 | 0 | 2 | 0 | 3 | 0 | 1 | 1 | 2 | 0 | 2 | 0 | 0 |
| YEAR: 2016 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 0 |
| TURNING MOVEMENTS | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| 2016 TOTAL | 0 | 1 | 1 | 2 | 0 | 1 | 0 | 2 | 0 | 1 | 1 | 2 | 0 | 0 |
| YEAR: 2015 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 2 | 0 | 2 | 0 | 3 | 1 | 2 | 0 | 2 | 0 | 2 | 0 | 0 |
| 2015 TOTAL | 0 | 2 | 0 | 2 | 0 | 3 | 1 | 2 | 0 | 2 | 0 | 2 | 0 | 0 |
| YEAR: 2014 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| TURNING MOVEMENTS | 0 | 2 | 0 | 2 | 0 | 3 | 0 | 2 | 0 | 1 | 1 | 2 | 0 | 0 |
| 2014 TOTAL | 0 | 2 | 0 | 2 | 0 | 3 | 0 | 2 | 0 | 1 | 1 | 2 | 0 | 0 |
| YEAR: 2013 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 2 | 0 | 2 | 0 | 3 | 0 | 2 | 0 | 2 | 0 | 2 | 0 | 0 |
| TURNING MOVEMENTS | 0 | 1 | 0 | 1 | 0 | 2 | 0 | 0 | 1 | 1 | 0 | 1 | 0 | 0 |
| 2013 TOTAL | 0 | 3 | 0 | 3 | 0 | 5 | 0 | 2 | 1 | 3 | 0 | 3 | 0 | 0 |
| FINAL TOTAL | 0 | 10 | 1 | 11 | 0 | 15 | 1 | 9 | 2 | 9 | 2 | 11 | 0 | 0 |

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see https://www.oregon.gov/ODOT/Data/documents/Crash_Data_Disclaimers.pdf.

Intersectional Crashes at SW Tualatin-Sherwood Rd \& SW 124th Ave
January 1, 2013 through December 31, 2017

| COLLISION TYPE | FATAL CRASHES | NONFATAL CRASHES | PROPERTY DAMAGE ONLY | TOTAL CRASHES | PEOPLE <br> KILLED | PEOPLE INJURED | TRUCKS | $\begin{gathered} \text { DRY } \\ \text { SURF } \end{gathered}$ | $\begin{aligned} & \text { WET } \\ & \text { SURF } \end{aligned}$ | DAY | DARK | INTERSECTION | INTERSECTION RELATED | $\begin{aligned} & \text { OFF- } \\ & \text { ROAD } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| YEAR: 2017 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 6 | 2 | 8 | 0 | 9 | 1 | 6 | 2 | 5 | 3 | 8 | 0 | 0 |
| TURNING MOVEMENTS | 0 | 2 | 0 | 2 | 0 | 2 | 0 | 1 | 1 | 2 | 0 | 2 | 0 | 0 |
| 2017 TOTAL | 0 | 8 | 2 | 10 | 0 | 11 | 1 | 7 | 3 | 7 | 3 | 10 | 0 | 0 |
| YEAR: 2016 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| FIXED / OTHER OBJECT | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 |
| REAR-END | 0 | 2 | 4 | 6 | 0 | 2 | 1 | 6 | 0 | 6 | 0 | 6 | 0 | 0 |
| TURNING MOVEMENTS | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| 2016 TOTAL | 0 | 3 | 5 | 8 | 0 | 3 | 1 | 8 | 0 | 8 | 0 | 8 | 0 | 1 |
| YEAR: 2015 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 2 | 2 | 4 | 0 | 3 | 0 | 4 | 0 | 4 | 0 | 4 | 0 | 0 |
| 2015 TOTAL | 0 | 2 | 2 | 4 | 0 | 3 | 0 | 4 | 0 | 4 | 0 | 4 | 0 | 0 |
| YEAR: 2014 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 6 | 3 | 9 | 0 | 13 | 0 | 7 | 2 | 8 | 1 | 9 | 0 | 0 |
| 2014 TOTAL | 0 | 6 | 3 | 9 | 0 | 13 | 0 | 7 | 2 | 8 | 1 | 9 | 0 | 0 |
| YEAR: 2013 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 1 | 0 | 1 | 0 | 2 | 0 | 0 | 1 | 1 | 0 | 1 | 0 | 0 |
| 2013 TOTAL | 0 | 1 | 0 | 1 | 0 | 2 | 0 | 0 | 1 | 1 | 0 | 1 | 0 | 0 |
| FINAL TOTAL | 0 | 20 | 12 | 32 | 0 | 32 | 2 | 26 | 6 | 28 | 4 | 32 | 0 | 1 |

Disclaimers: Effective 2016, collection of "Property Damage Only" (PDO) crash data elements was reduced for vehicles and participants. Age, Gender, License, Error and other elements are no longer available for PDO crash reporting. Please keep this in mind when comparing 2016 PDO crash data to prior years.

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see https://www.oregon.gov/ODOT/Data/documents/Crash_Data_Disclaimers.pdf.

Intersectional Crashes at SW Tualatin-Sherwood Rd \& SW 120th Ave
January 1, 2013 through December 31, 2017

| COLLISION TYPE | FATAL CRASHES | NON- <br> FATAL <br> CRASHES | PROPERTY DAMAGE ONLY | TOTAL CRASHES | PEOPLE <br> KILLED | PEOPLE INJURED | TRUCKS | $\begin{aligned} & \text { DRY } \\ & \text { SURF } \end{aligned}$ | WET SURF | DAY | DARK | INTERSECTION | INTERSECTION RELATED | $\begin{aligned} & \text { OFF- } \\ & \text { ROAD } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| YEAR: 2017 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| BACKING | 0 | 1 | 0 | 1 | 0 | 3 | 1 | 0 | 1 | 1 | 0 | 1 | 0 | 0 |
| REAR-END | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| TURNING MOVEMENTS | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| 2017 TOTAL | 0 | 3 | 0 | 3 | 0 | 5 | 1 | 2 | 1 | 3 | 0 | 3 | 0 | 0 |
| YEAR: 2014 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 1 | 0 | 0 |
| 2014 TOTAL | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 1 | 0 | 0 |
| FINAL TOTAL | 0 | 3 | 1 | 4 | 0 | 5 | 1 | 2 | 2 | 4 | 0 | 4 | 0 | 0 |

Disclaimers: Effective 2016, collection of "Property Damage Only" (PDO) crash data elements was reduced for vehicles and participants. Age, Gender, License, Error and other elements are no longer available for PDO crash reporting. Please keep this in mind when comparing 2016 PDO crash data to prior years.

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see https://www.oregon.gov/ODOT/Data/documents/Crash_Data_Disclaimers.pdf.

Intersectional Crashes at SW Tualatin-Sherwood Rd \& SW Cipole Rd
January 1, 2013 through December 31, 2017

| COLLISION TYPE | FATAL CRASHES | NONFATAL CRASHES | PROPERTY DAMAGE ONLY | TOTAL CRASHES | PEOPLE <br> KILLED | PEOPLE <br> INJURED | TRUCKS | $\begin{gathered} \text { DRY } \\ \text { SURF } \end{gathered}$ | $\begin{aligned} & \text { WET } \\ & \text { SURF } \end{aligned}$ | DAY | DARK | INTERSECTION | INTERSECTION RELATED | $\begin{aligned} & \text { OFF- } \\ & \text { ROAD } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| YEAR: 2016 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 1 | 1 | 2 | 0 | 1 | 0 | 2 | 0 | 2 | 0 | 2 | 0 | 0 |
| TURNING MOVEMENTS | 0 | 1 | 0 | 1 | 0 | 1 | 1 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| 2016 TOTAL | 0 | 2 | 1 | 3 | 0 | 2 | 1 | 3 | 0 | 3 | 0 | 3 | 0 | 0 |
| YEAR: 2015 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| BACKING | 0 | 0 | 1 | 1 | 0 | 0 | 1 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| REAR-END | 0 | 1 | 3 | 4 | 0 | 1 | 0 | 3 | 1 | 3 | 1 | 4 | 0 | 0 |
| 2015 TOTAL | 0 | 1 | 4 | 5 | 0 | 1 | 1 | 4 | 1 | 4 | 1 | 5 | 0 | 0 |
| YEAR: 2014 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 4 | 0 | 4 | 0 | 8 | 0 | 2 | 2 | 3 | 1 | 4 | 0 | 0 |
| 2014 TOTAL | 0 | 4 | 0 | 4 | 0 | 8 | 0 | 2 | 2 | 3 | 1 | 4 | 0 | 0 |
| YEAR: 2013 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 4 | 0 | 4 | 0 | 5 | 0 | 3 | 1 | 3 | 1 | 4 | 0 | 0 |
| TURNING MOVEMENTS | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 0 |
| 2013 TOTAL | 0 | 5 | 0 | 5 | 0 | 6 | 0 | 4 | 1 | 3 | 2 | 5 | 0 | 0 |
| FINAL TOTAL | 0 | 12 | 5 | 17 | 0 | 17 | 2 | 13 | 4 | 13 | 4 | 17 | 0 | 0 |

Disclaimers: Effective 2016, collection of "Property Damage Only" (PDO) crash data elements was reduced for vehicles and participants. Age, Gender, License, Error and other elements are no longer available for PDO crash reporting. Please keep this in mind when comparing 2016 PDO crash data to prior years.

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Intersectional Crashes at SW Tualatin-Sherwood Rd \& SW Langer Farms Pkwy January 1, 2013 through December 31, 2017

| COLLISION TYPE | FATAL CRASHES | NON- <br> FATAL CRASHES | PROPERTY DAMAGE ONLY | TOTAL CRASHES | PEOPLE <br> KILLED | PEOPLE INJURED | TRUCKS | DRY SURF | WET <br> SURF | DAY | DARK | INTERSECTION | INTERSECTION RELATED | $\begin{aligned} & \text { OFF- } \\ & \text { ROAD } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| YEAR: 2017 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 2 | 1 | 3 | 0 | 4 | 1 | 2 | 1 | 3 | 0 | 3 | 0 | 0 |
| TURNING MOVEMENTS | 0 | 2 | 1 | 3 | 0 | 2 | 0 | 3 | 0 | 2 | 1 | 3 | 0 | 0 |
| 2017 TOTAL | 0 | 4 | 2 | 6 | 0 | 6 | 1 | 5 | 1 | 5 | 1 | 6 | 0 | 0 |
| YEAR: 2016 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ANGLE | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| REAR-END | 0 | 4 | 0 | 4 | 0 | 4 | 0 | 2 | 2 | 3 | 1 | 4 | 0 | 0 |
| TURNING MOVEMENTS | 0 | 1 | 3 | 4 | 0 | 1 | 0 | 1 | 3 | 3 | 1 | 4 | 0 | 0 |
| 2016 TOTAL | 0 | 6 | 3 | 9 | 0 | 6 | 0 | 4 | 5 | 7 | 2 | 9 | 0 | 0 |
| YEAR: 2015 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 0 | 3 | 3 | 0 | 0 | 0 | 2 | 1 | 2 | 1 | 3 | 0 | 0 |
| 2015 TOTAL | 0 | 0 | 3 | 3 | 0 | 0 | 0 | 2 | 1 | 2 | 1 | 3 | 0 | 0 |
| YEAR: 2014 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 0 | 2 | 2 | 0 | 0 | 0 | 1 | 1 | 2 | 0 | 2 | 0 | 0 |
| TURNING MOVEMENTS | 0 | 1 | 0 | 1 | 0 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| 2014 TOTAL | 0 | 1 | 2 | 3 | 0 | 2 | 0 | 2 | 1 | 3 | 0 | 3 | 0 | 0 |
| YEAR: 2013 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 |
| TURNING MOVEMENTS | 0 | 1 | 0 | 1 | 0 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| 2013 TOTAL | 0 | 1 | 1 | 2 | 0 | 2 | 0 | 2 | 0 | 2 | 0 | 2 | 0 | 1 |
| FINAL TOTAL | 0 | 12 | 11 | 23 | 0 | 16 | 1 | 15 | 8 | 19 | 4 | 23 | 0 | 1 |

Disclaimers: Effective 2016, collection of "Property Damage Only" (PDO) crash data elements was reduced for vehicles and participants. Age, Gender, License, Error and other elements are no longer available for PDO crash reporting. Please keep this in mind when comparing 2016 PDO crash data to prior years

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Intersectional Crashes at SW Tualatin-Sherwood Rd \& SW Wildrose PI January 1, 2013 through December 31, 2017

| COLLISION TYPE | FATAL CRASHES | NON- <br> FATAL CRASHES | PROPERTY <br> DAMAGE ONLY | TOTAL CRASHES | PEOPLE <br> KILLED | PEOPLE <br> INJURED | TRUCKS | $\begin{gathered} \text { DRY } \\ \text { SURF } \end{gathered}$ | WET <br> SURF | DAY | DARK | INTERSECTION | INTERSECTION RELATED | $\begin{aligned} & \text { OFF- } \\ & \text { ROAD } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| YEAR: 2017 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| FIXED / OTHER OBJECT | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 1 |
| TURNING MOVEMENTS | 0 | 1 | 0 | 1 | 0 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| 2017 TOTAL | 0 | 1 | 1 | 2 | 0 | 2 | 0 | 1 | 0 | 2 | 0 | 2 | 0 | 1 |
| YEAR: 2014 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| 2014 TOTAL | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| YEAR: 2013 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| TURNING MOVEMENTS | 0 | 1 | 1 | 2 | 0 | 1 | 0 | 0 | 2 | 0 | 2 | 2 | 0 | 0 |
| 2013 TOTAL | 0 | 1 | 1 | 2 | 0 | 1 | 0 | 0 | 2 | 0 | 2 | 2 | 0 | 0 |
| FINAL TOTAL | 0 | 3 | 2 | 5 | 0 | 4 | 0 | 2 | 2 | 3 | 2 | 5 | 0 | 1 |

Disclaimers: Effective 2016, collection of "Property Damage Only" (PDO) crash data elements was reduced for vehicles and participants. Age, Gender, License, Error and other elements are no longer available for PDO crash reporting. Please keep this in mind when comparing 2016 PDO crash data to prior years.

A higher number of crashes may be reported as of 2011 compared to prior years. This does not necessarily reflect an increase in annual crashes. The higher numbers may result from a change to an internal departmental process that allows the Crash Analysis and Reporting Unit to add previously unavailable, non-fatal crash reports to the annual data file. Please be aware of this change when comparing pre-2011 crash statistics. For all disclaimers,
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## Appendix C Traffic Counts



Peak-Hour: 4:45 PM -- 5:45 PM
Peak 15-Min: 5:00 PM -- 5:15 PM


| $\begin{aligned} & \text { 5-Min Count } \\ & \text { Period } \\ & \text { Beginning At } \end{aligned}$ | $\begin{gathered} \text { Oregon St } \\ \text { (Northbound) } \\ \hline \end{gathered}$ |  |  |  | $\begin{gathered} \text { Oregon St } \\ \text { (Southbound) } \\ \hline \end{gathered}$ |  |  |  | Tualatin-Sherwood Rd (Eastbound) |  |  |  | Tualatin-Sherwood Rd (Westbound) |  |  |  | Total | Hourly Totals |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U |  |  |
| 4:00 PM | 5 | 0 | 11 | 0 | 0 | 0 | 1 | 0 | 0 | 62 | 11 | 0 | 25 | 70 | 0 | 0 | 185 |  |
| 4:05 PM | 12 | 0 | 15 | 0 | 0 | 1 | 0 | 0 | 0 | 58 | 11 | 0 | 20 | 55 | 0 | 0 | 172 |  |
| 4:10 PM | 12 | 0 | 22 | 0 | 3 | 1 | 0 | 0 | 0 | 49 | 8 | 0 | 29 | 65 | 0 | 0 | 189 |  |
| 4:15 PM | 6 | 0 | 7 | 0 | 2 | 0 | 0 | 0 | 1 | 64 | 7 | 0 | 24 | 63 | 0 | 0 | 174 |  |
| 4:20 PM | 9 | 0 | 14 | 0 | 1 | 0 | 0 | 0 | 0 | 42 | 13 | 0 | 29 | 68 | 0 | 0 | 176 |  |
| 4:25 PM | 6 | 1 | 9 | 0 | 0 | 1 | 2 | 0 | 0 | 43 | 11 | 0 | 26 | 62 | 2 | 0 | 163 |  |
| 4:30 PM | 6 | 0 | 7 | 0 | 1 | 0 | 0 | 0 | 0 | 57 | 9 | 0 | 33 | 78 | 0 | 0 | 191 |  |
| 4:35 PM | 11 | 0 | 12 | 0 | 0 | 0 | 0 | 0 | 0 | 62 | 13 | 0 | 22 | 55 | 0 | 0 | 175 |  |
| 4:40 PM | 6 | 1 | 13 | 0 | 1 | 0 | 1 | 0 | 1 | 46 | 9 | 0 | 36 | 77 | 0 | 0 | 191 |  |
| 4:45 PM | 12 | 0 | 20 | 0 | 1 | 0 | 0 | 0 | 0 | 46 | 11 | 0 | 25 | 64 | 1 | 0 | 180 |  |
| 4:50 PM | 13 | 0 | 8 | 0 | 1 | 0 | 0 | 0 | 0 | 54 | 12 | 0 | 31 | 70 | 0 | 0 | 189 |  |
| 4:55 PM | 13 | 0 | 14 | 0 | 1 | 1 | 0 | 0 | 0 | 58 | 7 | 0 | 29 | 61 | 0 | 0 | 184 | 2169 |
| 5:00 PM | 5 | 0 | 12 | 0 | 4 | 2 | 0 | 0 | 0 | 64 | 12 | 0 | 28 | 67 | 0 | 0 | 194 | 2178 |
| 5:05 PM | 10 | 0 | 23 | 0 | 0 | 1 | 1 | 0 | 0 | 74 | 17 | 0 | 27 | 62 | 2 | 0 | 217 | 2223 |
| 5:10 PM | 10 | 0 | 22 | 0 | 3 | 4 | 2 | 0 | 1 | 68 | 9 | 0 | 28 | 74 | 1 | 0 | 222 | 2256 |
| 5:15 PM | 10 | 0 | 19 | 0 | 0 | 0 | 1 | 0 | 1 | 58 | 7 | 0 | 32 | 59 | 0 | 0 | 187 | 2269 |
| 5:20 PM | 8 | 0 | 11 | 0 | 0 | 0 | 1 | 0 | 0 | 52 | 9 | 0 | 37 | 79 | 1 | 0 | 198 | 2291 |
| 5:25 PM | 9 | 0 | 8 | 0 | 0 | 0 | 0 | 0 | 1 | 50 | 9 | 0 | 31 | 76 | 0 | 0 | 184 | 2312 |
| 5:30 PM | 10 | 1 | 15 | 0 | 1 | 2 | 1 | 0 | 1 | 50 | 12 | 0 | 35 | 66 | 3 | 0 | 197 | 2318 |
| 5:35 PM | 16 | 0 | 11 | 0 | 0 | 0 | 1 | 0 | 1 | 54 | 7 | 0 | 34 | 69 | 0 | 0 | 193 | 2336 |
| 5:40 PM | 6 | 0 | 12 | 0 | 0 | 0 | 1 | 0 | 2 | 57 | 3 | 0 | 40 | 82 | 0 | 0 | 203 | 2348 |
| 5:45 PM | 5 | 0 | 13 | 0 | 0 | 0 | 0 | 0 | 0 | 46 | 6 | 0 | 32 | 66 | 1 | 0 | 169 | 2337 |
| 5:50 PM | 11 | 0 | 13 | 0 | 1 | 0 | 0 | 0 | 0 | 45 | 4 | 0 | 27 | 64 | 1 | 0 | 166 | 2314 |
| 5:55 PM | 7 | 0 | 14 | 0 | 1 | 0 | 0 | 0 | 1 | 52 | 6 | 0 | 17 | 74 | 1 | 0 | 173 | 2303 |
| Peak 15-Min Flowrates | Northbound |  |  |  | Southbound |  |  |  | Eastbound |  |  |  | Westbound |  |  |  | Total |  |
|  | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U |  |  |  |
| All Vehicles | 100 | 0 | 228 | 0 | 28 | 28 | 12 | 0 | 4 | 824 | 152 | 0 | 332 | 812 | 12 | 0 |  | 532 |
| Heavy Trucks | 4 | 0 | 8 |  | 0 | 0 | 0 |  | 0 | 40 | 20 |  |  | 8 | 0 |  |  | 84 |
| Pedestrians |  | 0 |  |  |  | 0 |  |  |  | 0 |  |  |  | 0 |  |  |  | 0 |
| Bicycles Railroad Stopped Buses | 0 | 0 | 0 |  | 0 | 0 | 0 |  | 0 | 1 | 0 |  | 0 | 0 | 0 |  |  | 1 |

Comments:




Peak-Hour: 4:45 PM -- 5:45 PM
Peak 15-Min: 5:05 PM -- 5:20 PM

Quality Counts
DATA THAT DRIVES COMMUNITIES


| $\begin{aligned} & \text { 5-Min Count } \\ & \text { Period } \\ & \text { Beginning At } \end{aligned}$ | Cipole Rd (Northbound) |  |  |  | Cipole Rd(Southbound) |  |  |  | Tualatin-Sherwood Rd (Eastbound) |  |  |  | Tualatin-Sherwood Rd (Westbound) |  |  |  | Total | Hourly Totals |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U |  |  |
| 4:00 PM | 0 | 0 | 0 | 0 | 22 | 0 | 14 | 0 | 4 | 66 | 0 | 0 | 0 | 75 | 3 | 0 | 184 |  |
| 4:05 PM | 0 | 0 | 0 | 0 | 18 | 0 | 12 | 0 | 5 | 62 | 0 | 0 | 0 | 73 | 2 | 0 | 172 |  |
| 4:10 PM | 0 | 0 | 0 | 0 | 10 | 0 | 16 | 0 | 2 | 78 | 0 | 0 | 0 | 71 | 7 | 0 | 184 |  |
| 4:15 PM | 0 | 0 | 0 | 0 | 11 | 0 | 14 | 0 | 6 | 72 | 0 | 0 | 0 | 77 | 2 | 0 | 182 |  |
| 4:20 PM | 0 | 0 | 0 | 0 | 9 | 0 | 6 | 0 | 3 | 50 | 0 | 0 | 0 | 81 | 4 | 0 | 153 |  |
| 4:25 PM | 0 | 0 | 0 | 0 | 5 | 0 | 8 | 0 | 3 | 68 | 0 | 0 | 0 | 92 | 2 | 0 | 178 |  |
| 4:30 PM | 0 | 0 | 0 | 0 | 6 | 0 | 12 | 0 | 1 | 62 | 0 | 0 | 0 | 90 | 1 | 0 | 172 |  |
| 4:35 PM | 0 | 0 | 0 | 0 | 3 | 0 | 8 | 0 | 2 | 67 | 0 | 0 | 0 | 86 | 3 | 0 | 169 |  |
| 4:40 PM | 0 | 0 | 0 | 0 | 7 | 0 | 12 | 0 | 4 | 58 | 0 | 0 | 0 | 87 | 4 | 0 | 172 |  |
| 4:45 PM | 0 | 0 | 0 | 0 | 10 | 0 | 9 | 0 | 3 | 64 | 0 | 0 | 0 | 85 | 1 | 0 | 172 |  |
| 4:50 PM | 0 | 0 | 0 | 0 | 5 | 0 | 7 | 0 | 1 | 70 | 0 | 0 | 0 | 85 | 4 | 0 | 172 |  |
| 4:55 PM | 0 | 0 | 0 | 0 | 6 | 0 | 11 | 0 | 5 | 71 | 0 | 0 | 0 | 89 | 1 | 0 | 183 | 2093 |
| 5:00 PM | 0 | 0 | 0 | 0 | 8 | 0 | 12 | 0 | 2 | 65 | 0 | 0 | 0 | 77 | 0 | 0 | 164 | 2073 |
| 5:05 PM | 0 | 0 | 0 | 0 | 9 | 0 | 15 | 0 | 8 | 81 | 0 | 0 | 0 | 82 | 1 | 0 | 196 | 2097 |
| 5:10 PM | 0 | 0 | 0 | 0 | 3 | 0 | 11 | 0 | 7 | 92 | 0 | 0 | 0 | 86 | 2 | 0 | 201 | 2114 |
| 5:15 PM | 0 | 0 | 0 | 0 | 7 | 0 | 11 | 0 | 4 | 86 | 0 | 0 | 0 | 87 | 0 | 0 | 195 | 2127 |
| 5:20 PM | 0 | 0 | 0 | 0 | 2 | 0 | 12 | 0 | 3 | 63 | 0 | 0 | 0 | 94 | 1 | 0 | 175 | 2149 |
| 5:25 PM | 0 | 0 | 0 | 0 | 3 | 0 | 8 | 0 | 1 | 69 | 0 | 0 | 0 | 95 | 1 | 0 | 177 | 2148 |
| 5:30 PM | 0 | 0 | 0 | 0 | 3 | 0 | 11 | 0 | 1 | 53 | 0 | 0 | 0 | 102 | 0 | 0 | 170 | 2146 |
| 5:35 PM | 0 | 0 | 0 | 0 | 4 | 0 | 8 | 0 | 1 | 78 | 0 | 0 | 0 | 100 | 1 | 0 | 192 | 2169 |
| 5:40 PM | 0 | 0 | 0 | 0 | 3 | 0 | 9 | 0 | 1 | 64 | 0 | 0 | 0 | 97 | 2 | 0 | 176 | 2173 |
| 5:45 PM | 0 | 0 | 0 | 0 | 1 | 0 | 8 | 0 | 0 | 63 | 0 | 0 | 0 | 90 | 3 | 0 | 165 | 2166 |
| 5:50 PM | 0 | 0 | 0 | 0 | 5 | 0 | 7 | 0 | 3 | 58 | 0 | 0 | 0 | 89 | 1 | 0 | 163 | 2157 |
| 5:55 PM | 0 | 0 | 0 | 0 | 1 | 0 | 4 | 0 | 1 | 67 | 0 | 0 | 0 | 91 | 0 | 0 | 164 | 2138 |
| Peak 15-Min Flowrates | Northbound |  |  |  | Southbound |  |  |  | Eastbound |  |  |  | Westbound |  |  |  | Total |  |
|  | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U |  |  |  |
| All Vehicles | 0 | 0 | 0 | 0 | 76 | 0 | 148 | 0 | 76 | 1036 | 0 | 0 | 0 | 1020 | 12 | 0 |  | 368 |
| Heavy Trucks | 0 | 0 | 0 |  | 8 | 0 | 0 |  | 4 | 52 | 0 |  | 0 | 24 | 8 |  |  | 96 |
| Pedestrians |  | 8 |  |  |  | 0 |  |  |  | 0 |  |  |  | 0 |  |  |  | 8 |
| Bicycles Railroad Stopped Buses | 0 | 0 | 0 |  | 0 | 0 | 0 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |  |  | 2 |

Comments:



Peak-Hour: 7:20 AM -- 8:20 AM
Peak 15-Min: 7:40 AM -- 7:55 AM

Quality Counts
DATA THAT DRIVES COMMUNITIES


| $\begin{aligned} & \text { 5-Min Count } \\ & \text { Period } \\ & \text { Beginning At } \end{aligned}$ | 120th Ave(Northbound) |  |  |  | $\begin{gathered} \text { 120th Ave } \\ \text { (Southbound) } \\ \hline \end{gathered}$ |  |  |  | Tualatin-Sherwood Rd (Eastbound) |  |  |  | Tualatin-Sherwood Rd (Westbound) |  |  |  | Total | Hourly Totals |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U |  |  |
| 7:00 AM | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 86 | 6 | 0 | 2 | 42 | 0 | 0 | 137 |  |
| 7:05 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 79 | 3 | 0 | 1 | 54 | 0 | 0 | 137 |  |
| 7:10 AM | 2 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 74 | 6 | 0 | 0 | 49 | 0 | 0 | 132 |  |
| 7:15 AM | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 97 | 1 | 0 | 0 | 52 | 0 | 0 | 152 |  |
| 7:20 AM | 3 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 74 | 4 | 0 | 1 | 47 | 0 | 0 | 131 |  |
| 7:25 AM | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 93 | 3 | 0 | 0 | 61 | 0 | 0 | 160 |  |
| 7:30 AM | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 87 | 1 | 0 | 1 | 38 | 0 | 0 | 129 |  |
| 7:35 AM | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 87 | 1 | 0 | 0 | 42 | 0 | 0 | 132 |  |
| 7:40 AM | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 85 | 2 | 0 | 1 | 63 | 0 | 0 | 154 |  |
| 7:45 AM | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 84 | 4 | 0 | 2 | 60 | 0 | 0 | 151 |  |
| 7:50 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 80 | 6 | 0 | 4 | 61 | 0 | 0 | 151 |  |
| 7:55 AM | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 75 | 8 | 0 | 0 | 56 | 0 | 0 | 141 | 1707 |
| 8:00 AM | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 91 | 2 | 0 | 0 | 46 | 0 | 0 | 141 | 1711 |
| 8:05 AM | 1 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 76 | 1 | 0 | 0 | 48 | 0 | 0 | 130 | 1704 |
| 8:10 AM | 2 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 76 | 4 | 0 | 2 | 55 | 0 | 0 | 141 | 1713 |
| 8:15 AM | 2 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 97 | 3 | 0 | 0 | 55 | 0 | 0 | 158 | 1719 |
| 8:20 AM | 1 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 86 | 2 | 0 | 1 | 39 | 0 | 0 | 131 | 1719 |
| 8:25 AM | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 76 | 0 | 0 | 1 | 55 | 0 | 0 | 133 | 1692 |
| 8:30 AM | 1 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 71 | 4 | 0 | 1 | 51 | 0 | 0 | 132 | 1695 |
| 8:35 AM | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 72 | 3 | 0 | 1 | 60 | 0 | 0 | 137 | 1700 |
| 8:40 AM | 2 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 61 | 1 | 0 | 3 | 52 | 0 | 0 | 122 | 1668 |
| 8:45 AM | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 65 | 3 | 0 | 1 | 46 | 0 | 0 | 117 | 1634 |
| 8:50 AM | 2 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 76 | 0 | 0 | 2 | 46 | 0 | 0 | 127 | 1610 |
| 8:55 AM | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 74 | 4 | 0 | 1 | 52 | 0 | 0 | 133 | 1602 |
| Peak 15-Min Flowrates | Northbound |  |  |  | Southbound |  |  |  | Eastbound |  |  |  | Westbound |  |  |  | Total |  |
|  | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U |  |  |  |
| All Vehicles | 4 | 0 | 12 | 0 | 0 | 0 | 0 | 0 | 0 | 996 | 48 | 0 | 28 | 736 | 0 | 0 |  | 824 |
| Heavy Trucks | 4 | 0 | 12 |  | 0 | 0 | 0 |  | 0 | 68 | 12 |  | 24 | 64 | 0 |  |  | 84 |
| Pedestrians |  | 0 |  |  |  | 0 |  |  |  | 0 |  |  |  | 0 |  |  |  | 0 |
| Bicycles Railroad Stopped Buses | 0 | 0 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |  |  | 0 |

Comments:






Peak-Hour: 7:20 AM -- 8:20 AM
Peak 15-Min: 7:55 AM -- 8:10 AM


| $\begin{aligned} & \text { 5-Min Count } \\ & \text { Period } \\ & \text { Beginning At } \end{aligned}$ | Langer Farms Pkwy (Northbound) |  |  |  | Langer Farms Pkwy (Southbound) |  |  |  | Tualatin-Sherwood Rd (Eastbound) |  |  |  | Tualatin-Sherwood Rd (Westbound) |  |  |  | Total | Hourly Totals |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U |  |  |
| 7:00 AM | 9 | 2 | 7 | 0 | 3 | 1 | 0 | 0 | 1 | 95 | 12 | 0 | 4 | 27 | 6 | 0 | 167 |  |
| 7:05 AM | 8 | 9 | 5 | 0 | 3 | 2 | 0 | 0 | 0 | 82 | 12 | 0 | 1 | 32 | 4 | 0 | 158 |  |
| 7:10 AM | 3 | 7 | 9 | 0 | 1 | 0 | 0 | 0 | 1 | 62 | 15 | 0 | 1 | 40 | 3 | 0 | 142 |  |
| 7:15 AM | 7 | 7 | 11 | 0 | 4 | 3 | 0 | 0 | 0 | 49 | 10 | 0 | 4 | 31 | 2 | 0 | 128 |  |
| 7:20 AM | 5 | 8 | 14 | 0 | 0 | 2 | 1 | 0 | 0 | 73 | 11 | 0 | 5 | 30 | 4 | 0 | 153 |  |
| 7:25 AM | 8 | 11 | 8 | 0 | 0 | 1 | 2 | 0 | 0 | 61 | 9 | 0 | 5 | 40 | 2 | 0 | 147 |  |
| 7:30 AM | 7 | 8 | 7 | 0 | 1 | 6 | 0 | 0 | 0 | 55 | 20 | 0 | 6 | 41 | 3 | 0 | 154 |  |
| 7:35 AM | 6 | 7 | 11 | 0 | 0 | 4 | 0 | 0 | 0 | 60 | 10 | 0 | 9 | 34 | 3 | 0 | 144 |  |
| 7:40 AM | 3 | 7 | 9 | 0 | 1 | 4 | 2 | 0 | 2 | 61 | 16 | 0 | 0 | 24 | 5 | 0 | 134 |  |
| 7:45 AM | 11 | 11 | 15 | 0 | 3 | 5 | 0 | 0 | 1 | 63 | 11 | 0 | 6 | 42 | 2 | 0 | 170 |  |
| 7:50 AM | 10 | 9 | 9 | 0 | 0 | 5 | 0 | 0 | 1 | 56 | 21 | 0 | 3 | 42 | 2 | 0 | 158 |  |
| 7:55 AM | 9 | 8 | 9 | 0 | 0 | 4 | 2 | 0 | 1 | 67 | 14 | 0 | 6 | 42 | 3 | 0 | 165 | 1820 |
| 8:00 AM | 10 | 12 | 10 | 0 | 3 | 3 | 0 | 0 | 0 | 77 | 13 | 0 | 8 | 42 | 3 | 0 | 181 | 1834 |
| 8:05 AM | 9 | 11 | 10 | 0 | 2 | 0 | 0 | 0 | 2 | 76 | 12 | 0 | 9 | 42 | 3 | 0 | 176 | 1852 |
| 8:10 AM | 10 | 8 | 3 | 0 | 3 | 4 | 0 | 0 | 1 | 50 | 14 | 0 | 3 | 39 | 3 | 0 | 138 | 1848 |
| 8:15 AM | 5 | 4 | 6 | 0 | 2 | 1 | 0 | 0 | 3 | 44 | 10 | 0 | 8 | 42 | 2 | 0 | 127 | 1847 |
| 8:20 AM | 5 | 8 | 7 | 0 | 3 | 2 | 0 | 0 | 0 | 71 | 15 | 0 | 7 | 35 | 2 | 0 | 155 | 1849 |
| 8:25 AM | 3 | 7 | 9 | 0 | 4 | 4 | 0 | 0 | 2 | 56 | 10 | 0 | 5 | 45 | 4 | 0 | 149 | 1851 |
| 8:30 AM | 11 | 5 | 8 | 0 | 6 | 2 | 0 | 0 | 0 | 55 | 12 | 0 | 3 | 24 | 0 | 0 | 126 | 1823 |
| 8:35 AM | 8 | 5 | 6 | 0 | 3 | 4 | 0 | 0 | 0 | 62 | 10 | 0 | 11 | 44 | 2 | 0 | 155 | 1834 |
| 8:40 AM | 10 | 8 | 9 | 0 | 2 | 4 | 0 | 0 | 2 | 52 | 6 | 0 | 9 | 34 | 4 | 0 | 140 | 1840 |
| 8:45 AM | 3 | 4 | 5 | 0 | 2 | 2 | 0 | 0 | 0 | 52 | 6 | 0 | 8 | 40 | 2 | 0 | 124 | 1794 |
| 8:50 AM | 5 | 5 | 7 | 0 | 2 | 7 | 0 | 0 | 0 | 61 | 11 | 0 | 5 | 31 | 1 | 0 | 135 | 1771 |
| 8:55 AM | 4 | 6 | 7 | 0 | 2 | 2 | 0 | 0 | 0 | 50 | 7 | 0 | 9 | 43 | 1 | 0 | 131 | 1737 |
| Peak 15-Min Flowrates | Northbound |  |  |  | Southbound |  |  |  | Eastbound |  |  |  | Westbound |  |  |  | Total |  |
|  | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U |  |  |  |
| All Vehicles | 112 | 124 | 116 | 0 | 20 | 28 | 8 | 0 | 12 | 880 | 156 | 0 | 92 | 504 | 36 | 0 |  | 888 |
| Heavy Trucks | 8 | 8 | 0 |  | 0 | 4 | 0 |  | 0 | 60 | 4 |  | 4 | 108 | 16 |  |  | 12 |
| Pedestrians |  | 0 |  |  |  | 8 |  |  |  | 0 |  |  |  | 4 |  |  |  | 12 |
| Bicycles Railroad Stopped Buses | 0 | 0 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |  |  | 0 |

Comments:




Peak-Hour: 4:45 PM -- 5:45 PM
Peak 15-Min: 5:05 PM -- 5:20 PM


| 5-Min Count Period Beginning At | Tonquin Rd (Northbound) |  |  |  | Tonquin Rd (Southbound) |  |  |  | Oregon St (Eastbound) |  |  |  | Oregon St(Westbound) |  |  |  | Total | Hourly Totals |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U |  |  |
| 4:00 PM | 21 | 0 | 9 | 0 | 0 | 0 | 0 | 0 | 0 | 10 | 13 | 0 | 13 | 24 | 0 | 0 | 90 |  |
| 4:05 PM | 20 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 13 | 10 | 0 | 10 | 28 | 0 | 0 | 88 |  |
| 4:10 PM | 25 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 19 | 10 | 0 | 10 | 28 | 0 | 0 | 99 |  |
| 4:15 PM | 21 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 12 | 11 | 0 | 11 | 23 | 0 | 0 | 85 |  |
| 4:20 PM | 31 | 0 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 8 | 0 | 10 | 34 | 0 | 0 | 97 |  |
| 4:25 PM | 31 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 12 | 16 | 0 | 9 | 20 | 0 | 0 | 92 |  |
| 4:30 PM | 25 | 0 | 10 | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 14 | 0 | 12 | 30 | 0 | 0 | 98 |  |
| 4:35 PM | 23 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 16 | 18 | 0 | 6 | 26 | 0 | 0 | 94 |  |
| 4:40 PM | 16 | 0 | 8 | 0 | 0 | 0 | 0 | 0 | 0 | 14 | 12 | 0 | 7 | 44 | 0 | 0 | 101 |  |
| 4:45 PM | 26 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 10 | 8 | 0 | 11 | 31 | 0 | 0 | 90 |  |
| 4:50 PM | 42 | 0 | 9 | 0 | 0 | 0 | 0 | 0 | 0 | 13 | 10 | 0 | 10 | 23 | 0 | 0 | 107 |  |
| 4:55 PM | 23 | 0 | 13 | 0 | 0 | 0 | 0 | 0 | 0 | 10 | 9 | 0 | 10 | 34 | 0 | 0 | 99 | 1140 |
| 5:00 PM | 27 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 17 | 5 | 0 | 13 | 29 | 0 | 0 | 93 | 1143 |
| 5:05 PM | 19 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 23 | 16 | 0 | 17 | 28 | 0 | 0 | 110 | 1165 |
| 5:10 PM | 25 | 0 | 8 | 0 | 0 | 0 | 0 | 0 | 0 | 24 | 8 | 0 | 15 | 44 | 0 | 0 | 124 | 1190 |
| 5:15 PM | 35 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 12 | 12 | 0 | 8 | 31 | 0 | 0 | 105 | 1210 |
| 5:20 PM | 27 | 0 | 9 | 0 | 0 | 0 | 0 | 0 | 0 | 14 | 15 | 0 | 7 | 32 | 0 | 0 | 104 | 1217 |
| 5:25 PM | 26 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 10 | 8 | 0 | 10 | 37 | 0 | 0 | 95 | 1220 |
| 5:30 PM | 24 | 0 | 8 | 0 | 0 | 0 | 0 | 0 | 0 | 17 | 18 | 0 | 10 | 34 | 0 | 0 | 111 | 1233 |
| 5:35 PM | 33 | 0 | 9 | 0 | 0 | 0 | 0 | 0 | 0 | 14 | 12 | 0 | 6 | 38 | 0 | 0 | 112 | 1251 |
| 5:40 PM | 26 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 15 | 11 | 0 | 2 | 38 | 0 | 0 | 95 | 1245 |
| 5:45 PM | 14 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 13 | 7 | 0 | 6 | 38 | 0 | 0 | 83 | 1238 |
| 5:50 PM | 24 | 0 | 9 | 0 | 0 | 0 | 0 | 0 | 0 | 16 | 7 | 0 | 2 | 27 | 0 | 0 | 85 | 1216 |
| 5:55 PM | 25 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 15 | 11 | 0 | 9 | 22 | 0 | 0 | 87 | 1204 |
| Peak 15-Min Flowrates | Northbound |  |  |  | Southbound |  |  |  | Eastbound |  |  |  | Westbound |  |  |  | Total |  |
|  | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U |  |  |  |
| All Vehicles | 316 | 0 | 88 | 0 | 0 | 0 | 0 | 0 | 0 | 236 | 144 | 0 | 160 | 412 | 0 | 0 |  | 56 |
| Heavy Trucks | 0 | 0 | 4 |  | 0 | 0 | 0 |  | 0 | 0 | 4 |  | 12 | 4 | 0 |  |  | 4 |
| Pedestrians |  | 0 |  |  |  | 0 |  |  |  | 0 |  |  |  | 0 |  |  |  | 0 |
| Bicycles Railroad Stopped Buses | 0 | 0 | 0 |  | 0 | 0 | 0 |  | 0 |  | 0 |  | 0 | 0 | 0 |  |  | 0 |

Comments:



## Appendix D Year 2019 Existing Conditions Worksheets



C Critical Lane Group



c Critical Lane Group

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


|  | $\rightarrow$ | 7 | $\checkmark$ |  | 4 | $p$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |  |
| Lane Configurations | 4 | 「 | ${ }^{4}$ | $\uparrow$ | \% | 「 |  |
| Traffic Volume (veh/h) | 1005 | 39 | 11 | 632 | 17 | 15 |  |
| Future Volume (Veh/h) | 1005 | 39 | 11 | 632 | 17 | 15 |  |
| Sign Control | Free |  |  | Free | Stop |  |  |
| Grade | 0\% |  |  | 0\% | 0\% |  |  |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |  |
| Hourly flow rate (vph) | 1069 | 41 | 12 | 672 | 18 | 16 |  |
| Pedestrians |  |  |  |  |  |  |  |
| Lane Width (ft) |  |  |  |  |  |  |  |
| Walking Speed (ft/s) |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |
| Median type | TWLTL |  |  | TWLTL |  |  |  |
| Median storage veh) | 2 |  |  | 2 |  |  |  |
| Upstream signal (ft) |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |
| vC , conflicting volume |  |  | 1110 |  | 1765 | 1069 |  |
| vC1, stage 1 conf vol |  |  |  |  | 1069 |  |  |
| $\mathrm{vC2}$, stage 2 conf vol |  |  |  |  | 696 |  |  |
| vCu, unblocked vol |  |  | 1110 |  | 1765 | 1069 |  |
| tC, single (s) |  |  | 4.9 |  | 7.4 | 7.1 |  |
| $\mathrm{tC}, 2$ stage (s) |  |  |  |  | 6.4 |  |  |
| tF (s) |  |  | 2.9 |  | 4.4 | 4.1 |  |
| p0 queue free \% |  |  | 97 |  | 90 | 91 |  |
| cM capacity (veh/h) |  |  | 409 |  | 185 | 182 |  |
| Direction, Lane \# | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 |  |
| Volume Total | 1069 | 41 | 12 | 672 | 18 | 16 |  |
| Volume Left | 0 | 0 | 12 | 0 | 18 | 0 |  |
| Volume Right | 0 | 41 | 0 | 0 | 0 | 16 |  |
| cSH | 1700 | 1700 | 409 | 1700 | 185 | 182 |  |
| Volume to Capacity | 0.63 | 0.02 | 0.03 | 0.40 | 0.10 | 0.09 |  |
| Queue Length 95th (ft) | 0 | 0 | 2 | 0 | 8 | 7 |  |
| Control Delay (s) | 0.0 | 0.0 | 14.1 | 0.0 | 26.5 | 26.7 |  |
| Lane LOS |  |  | B |  | D | D |  |
| Approach Delay (s) | 0.0 |  | 0.2 |  | 26.6 |  |  |
| Approach LOS |  |  |  |  | D |  |  |
| Intersection Summary |  |  |  |  |  |  |  |
| Average Delay |  |  | 0.6 |  |  |  |  |
| Intersection Capacity Utilization |  |  | 62.9\% | ICU Level of Service |  |  | B |
| Analysis Period (min) |  |  | 15 |  |  |  |  |


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ | F | ${ }^{7}$ | $\uparrow$ | 「 | ${ }^{7}$ | $\hat{6}$ |  | ${ }^{7}$ | 1 |  |
| Traffic Volume (vph) | 20 | 717 | 263 | 18 | 513 | 48 | 216 | 30 | 16 | 15 | 6 | 5 |
| Future Volume (vph) | 20 | 717 | 263 | 18 | 513 | 48 | 216 | 30 | 16 | 15 | 6 | 5 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 | 5.5 | 4.0 | 5.0 |  | 4.0 | 5.0 |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Frpb, ped/bikes | 1.00 | 1.00 | 0.98 | 1.00 | 1.00 | 0.98 | 1.00 | 1.00 |  | 1.00 | 0.99 |  |
| Flpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Frt | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 0.95 |  | 1.00 | 0.93 |  |
| Flt Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.95 | 1.00 |  |
| Satd. Flow (prot) | 1719 | 1667 | 1500 | 1543 | 1624 | 1491 | 1656 | 1656 |  | 1043 | 1278 |  |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.95 | 1.00 |  |
| Satd. Flow (perm) | 1719 | 1667 | 1500 | 1543 | 1624 | 1491 | 1656 | 1656 |  | 1043 | 1278 |  |
| Peak-hour factor, PHF | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Adj. Flow (vph) | 22 | 797 | 292 | 20 | 570 | 53 | 240 | 33 | 18 | 17 | 7 | 6 |
| RTOR Reduction (vph) | 0 | 0 | 38 | 0 | 0 | 18 | 0 | 15 | 0 | 0 | 6 | 0 |
| Lane Group Flow (vph) | 22 | 797 | 254 | 20 | 570 | 35 | 240 | 36 | 0 | 17 | 7 | 0 |
| Confl. Peds. (\#/hr) |  |  | 1 | 1 |  |  | 1 |  |  |  |  | 1 |
| Confl. Bikes (\#/hr) |  |  | 4 |  |  | 2 |  |  |  |  |  |  |
| Heavy Vehicles (\%) | 5\% | 14\% | 5\% | 17\% | 17\% | 6\% | 9\% | 3\% | 19\% | 73\% | 0\% | 80\% |
| Turn Type | Prot | NA | Perm | Prot | NA | Perm | Prot | NA |  | Prot | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |


| Permitted Phases | 2 |  |  |  |  |  |  |  | 6 |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Actuated Green, G (s) | 4.1 | 92.0 | 92.0 | 4.2 | 92.1 | 92.1 | 22.7 | 22.2 | 3.1 | 2.6 |
| Effective Green, g (s) | 4.1 | 92.0 | 92.0 | 4.2 | 92.1 | 92.1 | 22.7 | 22.2 | 3.1 | 2.6 |
| Actuated g/C Ratio | 0.03 | 0.66 | 0.66 | 0.03 | 0.66 | 0.66 | 0.16 | 0.16 | 0.02 | 0.02 |
| Clearance Time (s) | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 | 5.5 | 4.0 | 5.0 | 4.0 | 5.0 |
| Vehicle Extension (s) | 1.5 | 4.0 | 4.0 | 1.5 | 4.0 | 4.0 | 1.5 | 1.5 | 1.5 | 1.5 |
| Lane Grp Cap (vph) | 50 | 1095 | 985 | 46 | 1068 | 980 | 268 | 262 | 23 | 23 |
| v/s Ratio Prot | 0.01 | $\mathrm{co.48}$ |  | $\mathrm{co.01}$ | 0.35 |  | $\mathrm{co.14}$ | 0.02 | 0.02 | c 0.01 |


|  |  |  | 0.17 |  | 0.02 |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| v/s Ratio Perm | 0.44 | 0.73 | 0.26 | 0.43 | 0.53 | 0.04 | 0.90 | 0.14 | 0.74 | 0.31 |
| v/c Ratio | 66.8 | 15.8 | 9.9 | 66.7 | 12.6 | 8.4 | 57.5 | 50.7 | 68.0 | 67.8 |
| Uniform Delay, d1 | 1.19 | 0.66 | 0.58 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Progression Factor | 1.6 | 3.1 | 0.5 | 2.4 | 1.9 | 0.1 | 28.7 | 0.1 | 69.6 | 2.8 |
| Incremental Delay, d2 | 81.4 | 13.5 | 6.2 | 69.1 | 14.5 | 8.5 | 86.2 | 50.7 | 137.7 | 70.6 |
| Delay (s) | F | B | A | E | B | A | F | D | F | E |
| Level of Service |  | 12.9 |  |  | 15.7 |  |  | 80.0 | 108.6 |  |
| Approach Delay (s) |  | B |  |  | B |  | E |  | F |  |


| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 2000 Control Delay | 24.6 | HCM 2000 Level of Service | C |
| HCM 2000 Volume to Capacity ratio | 0.74 |  | 18.5 |
| Actuated Cycle Length (s) | 140.0 | Sum of lost time (s) | C |
| Intersection Capacity Utilization | $65.1 \%$ | ICU Level of Service |  |
| Analysis Period (min) | 15 |  |  |
| C Critical Lane Group |  |  |  |



## MOVEMENT SUMMARY

Site: 10 [SW Oregon St \& Murdock Rd]
Year 2019 - Existing AM Peak Hour Conditions
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | $\begin{aligned} & \text { OD } \\ & \text { Mov } \end{aligned}$ | Dema <br> Total veh/h | $\begin{array}{r} \text { =lows } \\ \text { HV } \\ \% \\ \hline \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | f Queue Distance ft | Prop. Queued | Effective Stop Rate per veh | Average Speed mph |
| South: Murdock Road |  |  |  |  |  |  |  |  |  |  |  |
| 3 | L2 | 80 | 1.0 | 0.531 | 10.9 | LOS B | 3.6 | 91.7 | 0.69 | 0.67 | 30.4 |
| 18 | R2 | 409 | 1.0 | 0.531 | 10.9 | LOS B | 3.6 | 91.7 | 0.69 | 0.67 | 28.9 |
| Appro |  | 489 | 1.0 | 0.531 | 10.9 | LOS B | 3.6 | 91.7 | 0.69 | 0.67 | 29.1 |
| East: Oregon St |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 93 | 14.0 | 0.227 | 5.2 | LOS A | 1.1 | 28.6 | 0.24 | 0.11 | 32.6 |
| 6 | T1 | 169 | 8.0 | 0.227 | 5.2 | LOS A | 1.1 | 28.6 | 0.24 | 0.11 | 32.1 |
| Approach |  | 262 | 10.1 | 0.227 | 5.2 | LOS A | 1.1 | 28.6 | 0.24 | 0.11 | 32.3 |
| West: Oregon St. |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T1 | 378 | 2.0 | 0.360 | 6.4 | LOS A | 2.1 | 54.3 | 0.33 | 0.18 | 32.6 |
| 12 | R2 | 60 | 2.0 | 0.360 | 6.4 | LOS A | 2.1 | 54.3 | 0.33 | 0.18 | 31.4 |
| Approach |  | 438 | 2.0 | 0.360 | 6.4 | LOS A | 2.1 | 54.3 | 0.33 | 0.18 | 32.4 |
| All Vehicles |  | 1189 | 3.4 | 0.531 | 8.0 | LOS A | 3.6 | 91.7 | 0.46 | 0.37 | 31.0 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: Same as Sign Control.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: US HCM 6.
HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies. Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: H:|23123278 - Orr Property Corporate ParklsynchrolDec 2019 TIA analysislSidralExisting 2019l23278_Existing AM.sip7

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ | F | \% | $\hat{\beta}$ |  | \% | $\uparrow$ | 「 | \% | $\uparrow$ |  |
| Traffic Volume (vph) | 13 | 611 | 223 | 185 | 786 | 13 | 176 | 92 | 102 | 29 | 169 | 13 |
| Future Volume (vph) | 13 | 611 | 223 | 185 | 786 | 13 | 176 | 92 | 102 | 29 | 169 | 13 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 5.5 | 4.0 | 4.0 | 5.5 |  | 4.0 | 4.5 | 4.5 | 4.0 | 4.5 |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Frpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Flpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Frt | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 |  | 1.00 | 1.00 | 0.85 | 1.00 | 0.99 |  |
| Flt Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  |
| Satd. Flow (prot) | 1805 | 1778 | 1586 | 1770 | 1826 |  | 1786 | 1900 | 1568 | 1752 | 1848 |  |
| Flt Permitted | 0.14 | 1.00 | 1.00 | 0.18 | 1.00 |  | 0.29 | 1.00 | 1.00 | 0.69 | 1.00 |  |
| Satd. Flow (perm) | 259 | 1778 | 1586 | 343 | 1826 |  | 554 | 1900 | 1568 | 1279 | 1848 |  |
| Peak-hour factor, PHF | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Adj. Flow (vph) | 14 | 650 | 237 | 197 | 836 | 14 | 187 | 98 | 109 | 31 | 180 | 14 |
| RTOR Reduction (vph) | 0 | 0 | 82 | 0 | 0 | 0 | 0 | 0 | 86 | 0 | 3 | 0 |
| Lane Group Flow (vph) | 14 | 650 | 155 | 197 | 850 | 0 | 187 | 98 | 23 | 31 | 191 | 0 |
| Confl. Peds. (\#/hr) | . |  |  |  |  | 1 | 2 |  |  |  |  | 2 |
| Confl. Bikes (\#/hr) |  |  |  |  |  |  |  |  |  |  |  |  |
| Heavy Vehicles (\%) | 0\% | 6\% | 1\% | 2\% | 3\% | 0\% | 1\% | 0\% | 3\% | 3\% | 1\% | 8\% |
| Bus Blockages (\#/hr) | 0 | 2 | 2 | 0 | 2 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| Turn Type | pm+pt | NA | pm+ov | pm+pt | NA |  | pm+pt | NA | Perm | pm+pt | NA |  |
| Protected Phases | 5 | 2 | 3 | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  | 2 | 6 |  |  | 8 |  | 8 | 4 |  |  |
| Actuated Green, G (s) | 44.8 | 43.2 | 53.4 | 55.4 | 49.8 |  | 27.2 | 19.9 | 19.9 | 16.3 | 13.0 |  |
| Effective Green, g (s) | 44.8 | 43.2 | 53.4 | 55.4 | 49.8 |  | 27.2 | 19.9 | 19.9 | 16.3 | 13.0 |  |
| Actuated g/C Ratio | 0.48 | 0.47 | 0.58 | 0.60 | 0.54 |  | 0.29 | 0.21 | 0.21 | 0.18 | 0.14 |  |
| Clearance Time (s) | 4.0 | 5.5 | 4.0 | 4.0 | 5.5 |  | 4.0 | 4.5 | 4.5 | 4.0 | 4.5 |  |
| Vehicle Extension (s) | 1.5 | 3.5 | 1.5 | 1.5 | 3.5 |  | 1.5 | 8.0 | 8.0 | 1.5 | 2.0 |  |
| Lane Grp Cap (vph) | 152 | 829 | 914 | 331 | 982 |  | 298 | 408 | 336 | 241 | 259 |  |
| v/s Ratio Prot | 0.00 | 0.37 | 0.02 | c0.05 | c0.47 |  | c0.07 | 0.05 |  | 0.00 | 0.10 |  |
| v/s Ratio Perm | 0.04 |  | 0.08 | 0.30 |  |  | c0.11 |  | 0.01 | 0.02 |  |  |
| $\mathrm{v} / \mathrm{c}$ Ratio | 0.09 | 0.78 | 0.17 | 0.60 | 0.87 |  | 0.63 | 0.24 | 0.07 | 0.13 | 0.74 |  |
| Uniform Delay, d1 | 16.2 | 20.8 | 9.2 | 13.5 | 18.5 |  | 26.3 | 30.1 | 29.0 | 32.0 | 38.2 |  |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Incremental Delay, d2 | 0.1 | 5.0 | 0.0 | 1.9 | 8.2 |  | 3.0 | 1.3 | 0.4 | 0.1 | 9.1 |  |
| Delay (s) | 16.3 | 25.8 | 9.2 | 15.4 | 26.7 |  | 29.3 | 31.4 | 29.3 | 32.1 | 47.3 |  |
| Level of Service | B | C | A | B | C |  | C | C | C | C | D |  |
| Approach Delay (s) |  | 21.3 |  |  | 24.6 |  |  | 29.8 |  |  | 45.2 |  |
| Approach LOS |  | C |  |  | C |  |  | C |  |  | D |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 2000 Control Delay |  |  | 26.1 |  | HCM 2000 | Level of S | Service |  | C |  |  |  |
| HCM 2000 Volume to Capacity ratio |  |  | 0.82 |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length (s) |  |  | 92.6 |  | Sum of los | time (s) |  |  | 18.0 |  |  |  |
|  |  |  | 81.5\% | ICU Level of Service |  |  |  |  | D |  |  |  |
| Intersection Capacity UtilizationAnalysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |

c Critical Lane Group

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |



c Critical Lane Group

c Critical Lane Group


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | * | $\uparrow$ | 「 | \% | $\uparrow$ | 「 | ${ }^{4}$ | $\uparrow$ |  | ${ }^{4}$ | $\uparrow$ |  |
| Traffic Volume (vph) | 15 | 646 | 311 | , | 648 | 21 | 180 | 16 | 14 | 44 | 25 | 15 |
| Future Volume (vph) | 15 | 646 | 311 | 5 | 648 | 21 | 180 | 16 | 14 | 44 | 25 | 15 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 | 5.5 | 4.0 | 5.0 |  | 4.0 | 5.0 |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Frpb, ped/bikes | 1.00 | 1.00 | 0.98 | 1.00 | 1.00 | 0.98 | 1.00 | 0.99 |  | 1.00 | 1.00 |  |
| Flpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Frt | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 0.93 |  | 1.00 | 0.94 |  |
| Flt Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.95 | 1.00 |  |
| Satd. Flow (prot) | 1805 | 1810 | 1550 | 1805 | 1810 | 1436 | 1787 | 1643 |  | 1719 | 1794 |  |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.95 | 1.00 |  |
| Satd. Flow (perm) | 1805 | 1810 | 1550 | 1805 | 1810 | 1436 | 1787 | 1643 |  | 1719 | 1794 |  |
| Peak-hour factor, PHF | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Adj. Flow (vph) | 16 | 695 | 334 | 5 | 697 | 23 | 194 | 17 | 15 | 47 | 27 | 16 |
| RTOR Reduction (vph) | 0 | 0 | 56 | 0 | 0 | 8 | 0 | 14 | 0 | 0 | 15 | 0 |
| Lane Group Flow (vph) | 16 | 695 | 278 | 5 | 697 | 15 | 194 | 19 | 0 | 47 | 28 | 0 |
| Confl. Peds. (\#/hr) |  |  |  |  |  |  |  |  | 1 | 1 |  |  |
| Confl. Bikes (\#/hr) |  |  | 2 |  |  | 3 |  |  |  |  |  |  |
| Heavy Vehicles (\%) | 0\% | 5\% | 2\% | 0\% | 5\% | 10\% | 1\% | 12\% | 0\% | 5\% | 0\% | 0\% |
| Turn Type | Prot | NA | Perm | Prot | NA | Perm | Prot | NA |  | Prot | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |


| Permitted Phases | 2 |  |  |  |  | 6 |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Actuated Green, G (s) | 2.5 | 78.1 | 78.1 | 1.1 | 76.7 | 76.7 | 16.2 | 12.0 | 10.3 | 6.1 |
| Effective Green, g (s) | 2.5 | 78.1 | 78.1 | 1.1 | 76.7 | 76.7 | 16.2 | 12.0 | 10.3 | 6.1 |
| Actuated g/C Ratio | 0.02 | 0.65 | 0.65 | 0.01 | 0.64 | 0.64 | 0.13 | 0.10 | 0.09 | 0.05 |
| Clearance Time (s) | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 | 5.5 | 4.0 | 5.0 | 4.0 | 5.0 |
| Vehicle Extension (s) | 1.5 | 4.0 | 4.0 | 1.5 | 4.0 | 4.0 | 1.5 | 1.5 | 1.5 | 1.5 |
| Lane Grp Cap (vph) | 37 | 1178 | 1008 | 16 | 1156 | 917 | 241 | 164 | 147 | 91 |
| v/s Ratio Prot | c0.01 | 0.38 |  | 0.00 | c0.39 |  | c0.11 | 0.01 | 0.03 | c0.02 |
| v/s Ratio Perm |  |  | 0.18 |  |  | 0.01 |  |  |  |  |
| v/c Ratio | 0.43 | 0.59 | 0.28 | 0.31 | 0.60 | 0.02 | 0.80 | 0.11 | 0.32 | 0.31 |
| Uniform Delay, d1 | 58.0 | 11.9 | 8.9 | 59.1 | 12.7 | 7.9 | 50.4 | 49.2 | 51.6 | 54.9 |
| Progression Factor | 1.09 | 0.67 | 0.75 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | 2.4 | 1.8 | 0.6 | 4.0 | 2.3 | 0.0 | 16.6 | 0.1 | 0.5 | 0.7 |
| Delay (s) | 65.8 | 9.8 | 7.3 | 63.1 | 15.0 | 7.9 | 67.0 | 49.3 | 52.0 | 55.6 |
| Level of Service | E | A | A | E | B | A | E | D | D | E |
| Approach Delay (s) |  | 9.8 |  |  | 15.1 |  |  | 64.5 |  | 53.7 |
| Approach LOS |  | A |  |  | B |  |  | E |  | D |


| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 2000 Control Delay | 19.5 | HCM 2000 Level of Service | B |
| HCM 2000 Volume to Capacity ratio | 0.61 |  | 18.5 |
| Actuated Cycle Length (s) | 120.0 | Sum of lost time (s) | B |
| Intersection Capacity Utilization | $59.6 \%$ | ICU Level of Service |  |
| Analysis Period (min) | 15 |  |  |
| C Critical Lane Group |  |  |  |



## MOVEMENT SUMMARY

Site: 10 [SW Oregon St \& Murdock Rd]
Year 2019 - Existing PM Peak Hour Conditions
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | $\begin{aligned} & \text { OD } \\ & \text { Mov } \end{aligned}$ | Dema Total veh/h | $\begin{gathered} \text { lows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | f Queue <br> Distance | Prop. Queued | Effective Stop Rate per veh | Average Speed mph |
| South: Murdock Road mid |  |  |  |  |  |  |  |  |  |  |  |
| 3 | L2 | 85 | 0.0 | 0.214 | 5.1 | LOS A | 1.0 | 26.4 | 0.36 | 0.23 | 32.4 |
| 18 | R2 | 156 | 4.0 | 0.214 | 5.1 | LOS A | 1.0 | 26.4 | 0.36 | 0.23 | 30.6 |
| Appr |  | 241 | 2.6 | 0.214 | 5.1 | LOS A | 1.0 | 26.4 | 0.36 | 0.23 | 31.2 |
| East: Oregon St |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 389 | 1.0 | 0.617 | 10.4 | LOS B | 5.8 | 146.2 | 0.46 | 0.24 | 30.1 |
| 6 | T1 | 387 | 0.0 | 0.617 | 10.4 | LOS B | 5.8 | 146.2 | 0.46 | 0.24 | 29.6 |
| Approach |  | 777 | 0.5 | 0.617 | 10.4 | LOS B | 5.8 | 146.2 | 0.46 | 0.24 | 29.8 |
| West: Oregon St. |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T1 | 172 | 2.0 | 0.276 | 6.8 | LOS A | 1.3 | 33.3 | 0.55 | 0.48 | 32.3 |
|  | R2 | 80 | 0.0 | 0.276 | 6.8 | LOS A | 1.3 | 33.3 | 0.55 | 0.48 | 31.2 |
| Approach |  | 252 | 1.4 | 0.276 | 6.8 | LOS A | 1.3 | 33.3 | 0.55 | 0.48 | 31.9 |
| All Vehicles |  | 1269 | 1.1 | 0.617 | 8.7 | LOS A | 5.8 | 146.2 | 0.46 | 0.29 | 30.5 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: Same as Sign Control.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: US HCM 6.
HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies. Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## Appendix E Year 2021 Background Conditions Worksheets




|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

c Critical Lane Group




Analysis Period (min) 15
C Critical Lane Group

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |



|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | $\uparrow$ | F | \% | 4 | F | 7 | $\hat{F}$ |  | 7 | 1 |  |
| Traffic Volume (vph) | 72 | 770 | 274 | 19 | 679 | 101 | 253 | 49 | 16 | 25 | 9 | 15 |
| Future Volume (vph) | 72 | 770 | 274 | 19 | 679 | 101 | 253 | 49 | 16 | 25 | 9 | 15 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 | 5.5 | 4.0 | 5.0 |  | 4.0 | 5.0 |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Frpb, ped/bikes | 1.00 | 1.00 | 0.97 | 1.00 | 1.00 | 0.98 | 1.00 | 1.00 |  | 1.00 | 0.99 |  |
| Flpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Frt | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 0.96 |  | 1.00 | 0.91 |  |
| Flt Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.95 | 1.00 |  |
| Satd. Flow (prot) | 1719 | 1667 | 1499 | 1543 | 1624 | 1491 | 1656 | 1709 |  | 1043 | 1128 |  |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.95 | 1.00 |  |
| Satd. Flow (perm) | 1719 | 1667 | 1499 | 1543 | 1624 | 1491 | 1656 | 1709 |  | 1043 | 1128 |  |
| Peak-hour factor, PHF | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Adj. Flow (vph) | 80 | 856 | 304 | 21 | 754 | 112 | 281 | 54 | 18 | 28 | 10 | 17 |
| RTOR Reduction (vph) | 0 | 0 | 40 | 0 | 0 | 43 | 0 | 9 | 0 | 0 | 17 | 0 |
| Lane Group Flow (vph) | 80 | 856 | 264 | 21 | 754 | 69 | 281 | 63 | 0 | 28 | 10 | 0 |
| Confl. Peds. (\#/hr) |  |  | 1 | 1 |  |  | 1 |  |  |  |  | 1 |
| Confl. Bikes (\#/hr) |  |  | 4 |  |  | 2 |  |  |  |  |  |  |
| Heavy Vehicles (\%) | 5\% | 14\% | 5\% | 17\% | 17\% | 6\% | 9\% | 3\% | 19\% | 73\% | 0\% | 80\% |
| Turn Type | Prot | NA | Perm | Prot | NA | Perm | Prot | NA |  | Prot | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |


| Permitted Phases | 2 |  |  |  |  | 6 |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Actuated Green, G (s) | 12.2 | 94.7 | 94.7 | 4.4 | 86.9 | 86.9 | 28.0 | 26.7 | 5.7 | 4.4 |
| Effective Green, g (s) | 12.2 | 94.7 | 94.7 | 4.4 | 86.9 | 86.9 | 28.0 | 26.7 | 5.7 | 4.4 |
| Actuated g/C Ratio | 0.08 | 0.63 | 0.63 | 0.03 | 0.58 | 0.58 | 0.19 | 0.18 | 0.04 | 0.03 |
| Clearance Time (s) | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 | 5.5 | 4.0 | 5.0 | 4.0 | 5.0 |
| Vehicle Extension (s) | 1.5 | 4.0 | 4.0 | 1.5 | 4.0 | 4.0 | 1.5 | 1.5 | 1.5 | 1.5 |
| Lane Grp Cap (vph) | 139 | 1052 | 946 | 45 | 940 | 863 | 309 | 304 | 39 | 33 |
| v/s Ratio Prot | c0.05 | c0.51 |  | 0.01 | 0.46 |  | c0.17 | c0.04 | 0.03 | 0.01 |
| $\mathrm{v} / \mathrm{s}$ Ratio Perm |  |  | 0.18 |  |  | 0.05 |  |  |  |  |
| v/c Ratio | 0.58 | 0.81 | 0.28 | 0.47 | 0.80 | 0.08 | 0.91 | 0.21 | 0.72 | 0.32 |
| Uniform Delay, d1 | 66.4 | 21.0 | 12.4 | 71.6 | 24.8 | 13.9 | 59.8 | 52.6 | 71.4 | 71.3 |
| Progression Factor | 1.13 | 0.49 | 0.28 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | 2.1 | 4.2 | 0.4 | 2.8 | 7.2 | 0.2 | 28.3 | 0.1 | 40.9 | 2.0 |
| Delay (s) | 76.9 | 14.6 | 3.9 | 74.4 | 32.0 | 14.1 | 88.1 | 52.7 | 112.2 | 73.4 |


| Level of Service | E | B | A | E | C | B | F | D | F | E |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Approach Delay (s) |  | 16.0 |  |  | 30.7 |  |  | 80.9 | C | 93.1 |
| Approach LOS |  | B |  |  | C |  |  | F |  | F |


| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 2000 Control Delay | 31.9 | HCM 2000 Level of Service | C |
| HCM 2000 Volume to Capacity ratio | 0.82 |  | 18.5 |
| Actuated Cycle Length (s) | 150.0 | Sum of lost time (s) | D |
| Intersection Capacity Utilization | $77.5 \%$ | ICU Level of Service |  |
| Analysis Period (min) | 15 |  |  |
| C Critical Lane Group |  |  |  |



## MOVEMENT SUMMARY

Site: 10 [SW Oregon St \& Murdock Rd]
Year 2021 - Background AM Peak Hour Conditions
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | $\begin{aligned} & \text { OD } \\ & \text { Mov } \end{aligned}$ | Dema Total veh/h | $\begin{gathered} \text { Fows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | f Queue Distance ft | Prop. Queued | Effective Stop Rate per veh | Average Speed mph |
| South: Murdock Road |  |  |  |  |  |  |  |  |  |  |  |
| 3 | L2 | 91 | 1.0 | 0.596 | 12.9 | LOS B | 4.6 | 116.0 | 0.75 | 0.80 | 29.6 |
| 18 | R2 | 438 | 1.0 | 0.596 | 12.9 | LOS B | 4.6 | 116.0 | 0.75 | 0.80 | 28.1 |
| Appr |  | 528 | 1.0 | 0.596 | 12.9 | LOS B | 4.6 | 116.0 | 0.75 | 0.80 | 28.4 |
| East: Oregon St |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 95 | 14.0 | 0.251 | 5.5 | LOS A | 1.2 | 32.2 | 0.26 | 0.13 | 32.5 |
| 6 | T1 | 192 | 8.0 | 0.251 | 5.5 | LOS A | 1.2 | 32.2 | 0.26 | 0.13 | 32.1 |
| Approach |  | 287 | 10.0 | 0.251 | 5.5 | LOS A | 1.2 | 32.2 | 0.26 | 0.13 | 32.2 |
| West: Oregon St. |  |  |  |  |  |  |  |  |  |  |  |
| 212 | T1 | $\begin{array}{r} 416 \\ 65 \end{array}$ | 2.0 | 0.397 | 6.9 | LOS A | 2.5 | 62.7 | 0.35 | 0.20 | 32.4 |
|  | R2 |  | 2.0 | 0.397 | 6.9 | LOS A | 2.5 | 62.7 | 0.35 | 0.20 | 31.2 |
| Approach |  | 481 | 2.0 | 0.397 | 6.9 | LOS A | 2.5 | 62.7 | 0.35 | 0.20 | 32.2 |
| All Vehicles |  | 1296 | 3.4 | 0.596 | 9.0 | LOS A | 4.6 | 116.0 | 0.50 | 0.43 | 30.5 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: Same as Sign Control.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: US HCM 6.
HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies. Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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|  | $\downarrow$ |  |  | 7 |  |  |  | $\dagger$ | $p$ |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\hat{F}$ |  | \% | $\uparrow$ |  | \% | ¢ |  | ${ }^{7}$ | $\uparrow$ |  |
| Traffic Volume (veh/h) | 23 | 10 | 20 | 4 | 4 | 2 | 0 | 373 | 24 | 16 | 193 | 12 |
| Future Volume (Veh/h) | 23 | 10 | 20 | 4 | 4 | 2 | 0 | 373 | 24 | 16 | 193 | 12 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 25 | 11 | 22 | 4 | 4 | 2 | 0 | 401 | 26 | 17 | 208 | 13 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (ft) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (fts) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | TWLTL |  |  | TWLTL |  |
| Median storage veh) |  |  |  |  |  |  |  | 2 |  |  | 2 |  |
| Upstream signal (ft) |  |  |  |  |  |  |  |  |  |  | 1007 |  |
| pX, platoon unblocked | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |  | 0.94 |  |  |  |  |  |
| vC , conflicting volume | 654 | 676 | 214 | 684 | 669 | 414 | 221 |  |  | 427 |  |  |
| VC1, stage 1 conf vol | 248 | 248 |  | 414 | 414 |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol | 405 | 427 |  | 270 | 255 |  |  |  |  |  |  |  |
| vCu , unblocked vol | 595 | 619 | 126 | 627 | 612 | 414 | 133 |  |  | 427 |  |  |
| tC, single (s) | 7.2 | 6.7 | 6.4 | 7.2 | 6.7 | 6.4 | 4.2 |  |  | 4.2 |  |  |
| $\mathrm{tC}, 2$ stage (s) | 6.2 | 5.7 |  | 6.2 | 5.7 |  |  |  |  |  |  |  |
| tF (s) | 3.6 | 4.1 | 3.4 | 3.6 | 4.1 | 3.4 | 2.3 |  |  | 2.3 |  |  |
| p0 queue free \% | 95 | 98 | 97 | 99 | 99 | 100 | 100 |  |  | 98 |  |  |
| cM capacity (veh/h) | 527 | 501 | 833 | 519 | 514 | 611 | 1287 |  |  | 1066 |  |  |
| Direction, Lane \# | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 | SB 1 | SB 2 |  |  |  |  |
| Volume Total | 25 | 33 | 4 | 6 | 0 | 427 | 17 | 221 |  |  |  |  |
| Volume Left | 25 | 0 | 4 | 0 | 0 | 0 | 17 | 0 |  |  |  |  |
| Volume Right | 0 | 22 | 0 | 2 | 0 | 26 | 0 | 13 |  |  |  |  |
| CSH | 527 | 682 | 519 | 543 | 1700 | 1700 | 1066 | 1700 |  |  |  |  |
| Volume to Capacity | 0.05 | 0.05 | 0.01 | 0.01 | 0.00 | 0.25 | 0.02 | 0.13 |  |  |  |  |
| Queue Length 95th (ft) | 4 | 4 | 1 | 1 | 0 | 0 | 1 | 0 |  |  |  |  |
| Control Delay (s) | 12.2 | 10.5 | 12.0 | 11.7 | 0.0 | 0.0 | 8.4 | 0.0 |  |  |  |  |
| Lane LOS | B | B | B | B |  |  | A |  |  |  |  |  |
| Approach Delay (s) | 11.2 |  | 11.8 |  | 0.0 |  | 0.6 |  |  |  |  |  |
| Approach LOS | B |  | B |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 1.2 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 35.7\% |  | CU Level | f Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


c Critical Lane Group



c Critical Lane Group

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |

C Critical Lane Group

|  | $\rightarrow$ | 7 | $\checkmark$ |  | 4 | $p$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |  |
| Lane Configurations | 4 | 「 | \% | $\uparrow$ | \% | 「 |  |
| Traffic Volume (veh/h) | 1010 | 22 | 3 | 1014 | 27 | 17 |  |
| Future Volume (Veh/h) | 1010 | 22 | 3 | 1014 | 27 | 17 |  |
| Sign Control | Free |  |  | Free | Stop |  |  |
| Grade | 0\% |  |  | 0\% | 0\% |  |  |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |  |
| Hourly flow rate (vph) | 1074 | 23 | 3 | 1079 | 29 | 18 |  |
| Pedestrians |  |  |  |  |  |  |  |
| Lane Width (ft) |  |  |  |  |  |  |  |
| Walking Speed (ft/s) |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |
| Median type | TWLTL |  |  | TWLTL |  |  |  |
| Median storage veh) | 2 |  |  | 2 |  |  |  |
| Upstream signal (ft) |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |
| vC , conflicting volume |  |  | 1097 |  | 2159 | 1074 |  |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  | 1074 |  |  |
| $\mathrm{vC2}$, stage 2 conf vol |  |  |  |  | 1085 |  |  |
| vCu, unblocked vol |  |  | 1097 |  | 2159 | 1074 |  |
| tC, single (s) |  |  | 4.1 |  | 6.4 | 6.2 |  |
| $\mathrm{tC}, 2$ stage (s) |  |  |  |  | 5.4 |  |  |
| tF (s) |  |  | 2.2 |  | 3.5 | 3.3 |  |
| p0 queue free \% |  |  | 100 |  | 87 | 93 |  |
| cM capacity (veh/h) |  |  | 644 |  | 231 | 270 |  |
| Direction, Lane \# | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 |  |
| Volume Total | 1074 | 23 | 3 | 1079 | 29 | 18 |  |
| Volume Left | 0 | 0 | 3 | 0 | 29 | 0 |  |
| Volume Right | 0 | 23 | 0 | 0 | 0 | 18 |  |
| cSH | 1700 | 1700 | 644 | 1700 | 231 | 270 |  |
| Volume to Capacity | 0.63 | 0.01 | 0.00 | 0.63 | 0.13 | 0.07 |  |
| Queue Length 95th (ft) | 0 | 0 | 0 | 0 | 11 | 5 |  |
| Control Delay (s) | 0.0 | 0.0 | 10.6 | 0.0 | 22.8 | 19.3 |  |
| Lane LOS |  |  | B |  | C | C |  |
| Approach Delay (s) | 0.0 |  | 0.0 |  | 21.5 |  |  |
| Approach LOS |  |  |  |  | C |  |  |
| Intersection Summary |  |  |  |  |  |  |  |
| Average Delay |  |  | 0.5 |  |  |  |  |
| Intersection Capacity Utilization |  |  | 63.4\% | ICU Level of Service |  |  | B |
| Analysis Period (min) |  |  | 15 |  |  |  |  |



|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |



## MOVEMENT SUMMARY

Site: 10 [SW Oregon St \& Murdock Rd]
Year 2021 - Background PM Peak Hour Conditions
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | $\begin{aligned} & \text { OD } \\ & \text { Mov } \end{aligned}$ | Dema Total veh/h | $\begin{array}{r} \text { lows } \\ \text { HV } \\ \% \\ \hline \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | f Queue Distance ft | Prop. Queued | Effective Stop Rate per veh | Average Speed mph |
| South: Murdock Road |  |  |  |  |  |  |  |  |  |  |  |
| 3 | L2 | 91 | 1.0 | 0.229 | 5.4 | LOS A | 1.1 | 28.3 | 0.39 | 0.26 | 32.2 |
| 18 | R2 | 160 | 4.0 | 0.229 | 5.4 | LOS A | 1.1 | 28.3 | 0.39 | 0.26 | 30.5 |
| Appr |  | 251 | 2.9 | 0.229 | 5.4 | LOS A | 1.1 | 28.3 | 0.39 | 0.26 | 31.1 |
| East: Oregon St |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 412 | 1.0 | 0.671 | 11.9 | LOS B | 7.0 | 175.9 | 0.53 | 0.29 | 29.5 |
| 6 | T1 | 424 | 1.0 | 0.671 | 11.9 | LOS B | 7.0 | 175.9 | 0.53 | 0.29 | 29.0 |
| Approach |  | 836 | 1.0 | 0.671 | 11.9 | LOS B | 7.0 | 175.9 | 0.53 | 0.29 | 29.3 |
| West: Oregon St. |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T1 | 194 | 2.0 | 0.317 | 7.5 | LOS A | 1.5 | 39.1 | 0.58 | 0.52 | 32.0 |
| 12 | R2 | 87 | 2.0 | 0.317 | 7.5 | LOS A | 1.5 | 39.1 | 0.58 | 0.52 | 30.9 |
| Approach |  | 281 | 2.0 | 0.317 | 7.5 | LOS A | 1.5 | 39.1 | 0.58 | 0.52 | 31.6 |
| All Vehicles |  | 1367 | 1.6 | 0.671 | 9.8 | LOS A | 7.0 | 175.9 | 0.51 | 0.33 | 30.0 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: Same as Sign Control.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: US HCM 6.
HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies. Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## Appendix F Year 2021 Total Traffic Conditions Worksheets


c Critical Lane Group

|  | 4 |  |  | 7 |  |  | 4 | 4 | \% |  | $\dagger$ | / |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{1}$ | 4 | 「' | ${ }^{1}$ | $\uparrow$ |  |  | 4 | 「' | ${ }^{1}$ | $\uparrow$ |  |
| Traffic Volume (vph) | 8 | 836 | 121 | 105 | 598 | 6 | 87 | 3 | 354 | 4 | 0 | 0 |
| Future Volume (vph) | 8 | 836 | 121 | 105 | 598 | 6 | 87 | 3 | 354 | 4 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 |  |  | 5.0 | 4.0 | 4.0 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |  | 1.00 | 1.00 | 1.00 |  |  |
| Frpb, ped/bikes | 1.00 | 1.00 | 0.98 | 1.00 | 1.00 |  |  | 1.00 | 0.99 | 1.00 |  |  |
| Flpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |  | 1.00 | 1.00 | 1.00 |  |  |
| Frt | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 |  |  | 1.00 | 0.85 | 1.00 |  |  |
| Flt Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  |  | 0.95 | 1.00 | 0.95 |  |  |
| Satd. Flow (prot) | 1805 | 1729 | 1448 | 1556 | 1639 |  |  | 1531 | 1522 | 1442 |  |  |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 0.17 | 1.00 |  |  | 0.55 | 1.00 | 1.00 |  |  |
| Satd. Flow (perm) | 1805 | 1729 | 1448 | 279 | 1639 |  |  | 890 | 1522 | 1518 |  |  |
| Peak-hour factor, PHF | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Adj. Flow (vph) | 8 | 880 | 127 | 111 | 629 | 6 | 92 | 3 | 373 | 4 | 0 | 0 |
| RTOR Reduction (vph) | 0 | 0 | 29 | 0 | 0 | 0 | 0 | 0 | 96 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 8 | 880 | 98 | 111 | 635 | 0 | 0 | 95 | 277 | 4 | 0 | 0 |
| Confl. Peds. (\#/hr) |  |  | 1 | 1 |  |  |  |  | 1 | 1 |  |  |
| Confl. Bikes (\#/hr) |  |  | 2 |  |  |  |  |  |  |  |  |  |
| Heavy Vehicles (\%) | 0\% | 9\% | 9\% | 16\% | 15\% | 0\% | 19\% | 0\% | 5\% | 25\% | 0\% | 0\% |
| Bus Blockages (\#/hr) | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Turn Type | Prot | NA | Perm | pm+pt | NA |  | Perm | NA | pm+ov | Perm |  |  |
| Protected Phases | $5!$ | $2!$ |  | 1! | $6!$ |  |  | 8 ! | 1 |  | $4!$ |  |
| Permitted Phases |  |  | 2 | $6!$ |  |  | $8!$ |  | 8 | $4!$ |  |  |
| Actuated Green, G (s) | 0.6 | 43.9 | 43.9 | 49.5 | 44.9 |  |  | 7.1 | 14.8 | 2.0 |  |  |
| Effective Green, g (s) | 0.6 | 43.9 | 43.9 | 49.5 | 44.9 |  |  | 7.1 | 14.8 | 2.0 |  |  |
| Actuated g/C Ratio | 0.01 | 0.65 | 0.65 | 0.74 | 0.67 |  |  | 0.11 | 0.22 | 0.03 |  |  |
| Clearance Time (s) | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 |  |  | 5.0 | 4.0 | 4.0 |  |  |
| Vehicle Extension (s) | 1.0 | 3.5 | 3.5 | 1.0 | 3.5 |  |  | 1.0 | 1.0 | 1.0 |  |  |
| Lane Grp Cap (vph) | 16 | 1131 | 947 | 352 | 1096 |  |  | 94 | 335 | 45 |  |  |
| v/s Ratio Prot | 0.00 | c0.51 |  | 0.04 | 0.39 |  |  |  | c0.09 |  |  |  |
| v/s Ratio Perm |  |  | 0.07 | 0.20 |  |  |  | c0.11 | 0.09 | 0.00 |  |  |
| v/c Ratio | 0.50 | 0.78 | 0.10 | 0.32 | 0.58 |  |  | 1.01 | 0.83 | 0.09 |  |  |
| Uniform Delay, d1 | 33.1 | 8.2 | 4.3 | 6.5 | 6.0 |  |  | 30.0 | 24.9 | 31.7 |  |  |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |  | 1.00 | 1.00 | 1.00 |  |  |
| Incremental Delay, d2 | 8.7 | 3.5 | 0.1 | 0.2 | 0.8 |  |  | 95.7 | 14.6 | 0.3 |  |  |
| Delay (s) | 41.8 | 11.7 | 4.4 | 6.7 | 6.8 |  |  | 125.7 | 39.6 | 32.0 |  |  |
| Level of Service | D | B | A | A | A |  |  | F | D | C |  |  |
| Approach Delay (s) |  | 11.0 |  |  | 6.8 |  |  | 57.1 |  |  | 32.0 |  |
| Approach LOS |  | B |  |  | A |  |  | E |  |  | C |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 2000 Control Delay |  |  | 19.3 |  |  |  |  | B |  |  |  |  |
| HCM 2000 Volume to Capacity ratio |  |  | 0.86 | HCM 2000 Level of Service |  |  |  |  |  |  |  |  |
| Actuated Cycle Length (s) |  |  | 67.1 | Sum of lost time (s) |  |  |  |  | 14.5 |  |  |  |
| Intersection Capacity Utilization |  |  | 81.4\% | ICU Level of Service |  |  |  |  | D |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |
| ! Phase conflict between lane groups. |  |  |  |  |  |  |  |  |  |  |  |  |
| c Critical Lane Group |  |  |  |  |  |  |  |  |  |  |  |  |






|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | $\uparrow$ | F | ${ }^{*}$ | $\uparrow$ | F | \% | $\uparrow$ |  | \% | $\hat{\%}$ |  |
| Traffic Volume (vph) | 72 | 777 | 278 | 19 | 705 | 101 | 271 | 49 | 16 | 25 | 9 | 15 |
| Future Volume (vph) | 72 | 777 | 278 | 19 | 705 | 101 | 271 | 49 | 16 | 25 | ) | 15 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 | 5.5 | 4.0 | 5.0 |  | 4.0 | 5.0 |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Frpb, ped/bikes | 1.00 | 1.00 | 0.97 | 1.00 | 1.00 | 0.98 | 1.00 | 1.00 |  | 1.00 | 0.99 |  |
| Flpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Frt | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 0.96 |  | 1.00 | 0.91 |  |
| Flt Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.95 | 1.00 |  |
| Satd. Flow (prot) | 1719 | 1667 | 1499 | 1543 | 1624 | 1491 | 1656 | 1709 |  | 1043 | 1128 |  |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.95 | 1.00 |  |
| Satd. Flow (perm) | 1719 | 1667 | 1499 | 1543 | 1624 | 1491 | 1656 | 1709 |  | 1043 | 1128 |  |
| Peak-hour factor, PHF | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Adj. Flow (vph) | 80 | 863 | 309 | 21 | 783 | 112 | 301 | 54 | 18 | 28 | 10 | 17 |
| RTOR Reduction (vph) | 0 | 0 | 39 | 0 | 0 | 42 | 0 | 9 | 0 | 0 | 17 | 0 |
| Lane Group Flow (vph) | 80 | 863 | 270 | 21 | 783 | 70 | 301 | 63 | 0 | 28 | 10 | 0 |
| Confl. Peds. (\#/hr) |  |  | 1 | 1 |  |  | 1 |  |  |  |  | 1 |
| Confl. Bikes (\#/hr) |  |  | 4 |  |  | 2 |  |  |  |  |  |  |
| Heavy Vehicles (\%) | 5\% | 14\% | 5\% | 17\% | 17\% | 6\% | 9\% | 3\% | 19\% | 73\% | 0\% | 80\% |
| Turn Type | Prot | NA | Perm | Prot | NA | Perm | Prot | NA |  | Prot | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases |  |  | 2 |  |  | 6 |  |  |  |  |  |  |
| Actuated Green, G (s) | 12.4 | 95.8 | 95.8 | 4.4 | 87.8 | 87.8 | 26.9 | 25.6 |  | 5.7 | 4.4 |  |
| Effective Green, g (s) | 12.4 | 95.8 | 95.8 | 4.4 | 87.8 | 87.8 | 26.9 | 25.6 |  | 5.7 | 4.4 |  |
| Actuated g/C Ratio | 0.08 | 0.64 | 0.64 | 0.03 | 0.59 | 0.59 | 0.18 | 0.17 |  | 0.04 | 0.03 |  |
| Clearance Time (s) | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 | 5.5 | 4.0 | 5.0 |  | 4.0 | 5.0 |  |
| Vehicle Extension (s) | 1.5 | 4.0 | 4.0 | 1.5 | 4.0 | 4.0 | 1.5 | 1.5 |  | 1.5 | 1.5 |  |
| Lane Grp Cap (vph) | 142 | 1064 | 957 | 45 | 950 | 872 | 296 | 291 |  | 39 | 33 |  |
| v/s Ratio Prot | c0.05 | c0.52 |  | 0.01 | 0.48 |  | c0.18 | c0.04 |  | 0.03 | 0.01 |  |
| v/s Ratio Perm |  |  | 0.18 |  |  | 0.05 |  |  |  |  |  |  |
| v/c Ratio | 0.56 | 0.81 | 0.28 | 0.47 | 0.82 | 0.08 | 1.02 | 0.22 |  | 0.72 | 0.32 |  |
| Uniform Delay, d1 | 66.2 | 20.3 | 11.9 | 71.6 | 24.9 | 13.5 | 61.5 | 53.6 |  | 71.4 | 71.3 |  |
| Progression Factor | 1.23 | 0.52 | 0.34 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Incremental Delay, d2 | 1.8 | 4.0 | 0.4 | 2.8 | 8.1 | 0.2 | 56.7 | 0.1 |  | 40.9 | 2.0 |  |
| Delay (s) | 83.3 | 14.5 | 4.5 | 74.4 | 33.0 | 13.7 | 118.2 | 53.7 |  | 112.2 | 73.4 |  |
| Level of Service | F | B | A | E | C | B | F | D |  | F | E |  |
| Approach Delay (s) |  | 16.4 |  |  | 31.6 |  |  | 105.8 |  |  | 93.1 |  |
| Approach LOS |  | B |  |  | C |  |  | F |  |  | F |  |


| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 2000 Control Delay | 36.2 | HCM 2000 Level of Service | D |
| HCM 2000 Volume to Capacity ratio | 0.83 |  | 18.5 |
| Actuated Cycle Length (s) | 150.0 | Sum of lost time (s) | D |
| Intersection Capacity Utilization | $78.8 \%$ | ICU Level of Service |  |
| Analysis Period (min) | 15 |  |  |
| c Critical Lane Group |  |  |  |



## MOVEMENT SUMMARY

Site: 10 [SW Oregon St \& Murdock Rd]

## Year 2021 - Total Culdesac AM Peak Hour Conditions

Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | $\begin{aligned} & \text { OD } \\ & \text { Mov } \end{aligned}$ | Dema Total veh/h | $\begin{gathered} \text { =lows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | f Queue <br> Distance <br> ft | Prop. Queued | Effective Stop Rate per veh | Average Speed mph |
| South: Murdock Road mid |  |  |  |  |  |  |  |  |  |  |  |
| 3 | L2 | 91 | 1.0 | 0.615 | 13.5 | LOS B | 4.9 | 123.6 | 0.77 | 0.83 | 29.3 |
| 18 | R2 | 448 | 1.0 | 0.615 | 13.5 | LOS B | 4.9 | 123.6 | 0.77 | 0.83 | 27.9 |
| Appro |  | 539 | 1.0 | 0.615 | 13.5 | LOS B | 4.9 | 123.6 | 0.77 | 0.83 | 28.1 |
| East: Oregon St |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 98 | 14.0 | 0.255 | 5.5 | LOS A | 1.2 | 32.9 | 0.26 | 0.13 | 32.5 |
| 6 | T1 | 194 | 8.0 | 0.255 | 5.5 | LOS A | 1.2 | 32.9 | 0.26 | 0.13 | 32.1 |
| Approach |  | 292 | 10.0 | 0.255 | 5.5 | LOS A | 1.2 | 32.9 | 0.26 | 0.13 | 32.2 |
| West: Oregon St. |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T1 | 427 | 2.0 | 0.407 | 7.0 | LOS A | 2.6 | 65.0 | 0.36 | 0.20 | 32.3 |
|  | R2 | 65 | 2.0 | 0.407 | 7.0 | LOS A | 2.6 | 65.0 | 0.36 | 0.20 | 31.2 |
| Approach |  | 492 | 2.0 | 0.407 | 7.0 | LOS A | 2.6 | 65.0 | 0.36 | 0.20 | 32.2 |
| All Vehicles |  | 1322 | 3.4 | 0.615 | 9.3 | LOS A | 4.9 | 123.6 | 0.51 | 0.44 | 30.4 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: Same as Sign Control.
Vehicle movement LOS values are based on average delay and $\mathrm{v} / \mathrm{c}$ ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: US HCM 6.
HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies. Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | 4 | 「 | ${ }^{7}$ | $\uparrow$ |  |  | $\uparrow$ | 「 | ${ }^{*}$ | $\uparrow$ |  |
| Traffic Volume (vph) | 7 | 767 | 118 | 383 | 1005 | 8 | 126 | 1 | 163 | 11 | 10 | 8 |
| Future Volume (vph) | 7 | 767 | 118 | 383 | 1005 | 8 | 126 | 1 | 163 | 11 | 10 | 8 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 |  |  | 5.0 | 4.0 | 4.0 | 4.0 |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |  | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Frpb, ped/bikes | 1.00 | 1.00 | 0.98 | 1.00 | 1.00 |  |  | 1.00 | 1.00 | 1.00 | 0.99 |  |
| Flpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |  | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Frt | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 |  |  | 1.00 | 0.85 | 1.00 | 0.93 |  |
| Flt Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  |  | 0.95 | 1.00 | 0.95 | 1.00 |  |
| Satd. Flow (prot) | 1803 | 1830 | 1464 | 1770 | 1828 |  |  | 1739 | 1568 | 1805 | 1754 |  |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 0.10 | 1.00 |  |  | 0.26 | 1.00 | 0.67 | 1.00 |  |
| Satd. Flow (perm) | 1803 | 1830 | 1464 | 177 | 1828 |  |  | 466 | 1568 | 1273 | 1754 |  |
| Peak-hour factor, PHF | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Adj. Flow (vph) | 8 | 825 | 127 | 412 | 1081 | 9 | 135 | 1 | 175 | 12 | 11 | 9 |
| RTOR Reduction (vph) | 0 | 0 | 38 | 0 | 0 | 0 | 0 | 0 | 85 | 0 | 8 | 0 |
| Lane Group Flow (vph) | 8 | 825 | 89 | 412 | 1090 | 0 | 0 | 136 | 90 | 12 | 12 | 0 |
| Confl. Peds. (\#/hr) | 2 |  |  |  |  | 2 | 1 |  |  |  |  | 1 |
| Confl. Bikes (\#/hr) |  |  | 1 |  |  | 3 |  |  |  |  |  |  |
| Heavy Vehicles (\%) | 0\% | 3\% | 8\% | 2\% | 3\% | 0\% | 4\% | 0\% | 3\% | 0\% | 0\% | 0\% |
| Bus Blockages (\#/hr) | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Turn Type | Prot | NA | Perm | pm+pt | NA |  | Perm | NA | pm+ov | Perm | NA |  |
| Protected Phases | $5!$ | $2!$ |  | 1! | $6!$ |  |  | $8!$ | 1 |  | $4!$ |  |
| Permitted Phases |  |  | 2 | $6!$ |  |  | $8!$ |  | 8 | $4!$ |  |  |
| Actuated Green, G (s) | 0.7 | 48.6 | 48.6 | 62.2 | 57.5 |  |  | 15.6 | 35.7 | 6.1 | 6.1 |  |
| Effective Green, g (s) | 0.7 | 48.6 | 48.6 | 62.2 | 57.5 |  |  | 15.6 | 35.7 | 6.1 | 6.1 |  |
| Actuated g/C Ratio | 0.01 | 0.55 | 0.55 | 0.70 | 0.65 |  |  | 0.18 | 0.40 | 0.07 | 0.07 |  |
| Clearance Time (s) | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 |  |  | 5.0 | 4.0 | 4.0 | 4.0 |  |
| Vehicle Extension (s) | 1.0 | 3.5 | 3.5 | 1.0 | 3.5 |  |  | 1.0 | 1.0 | 1.0 | 1.0 |  |
| Lane Grp Cap (vph) | 14 | 1007 | 805 | 487 | 1190 |  |  | 82 | 633 | 87 | 121 |  |
| v/s Ratio Prot | 0.00 | 0.45 |  | c0.19 | c0.60 |  |  |  | 0.03 |  | 0.01 |  |
| v/s Ratio Perm |  |  | 0.06 | 0.40 |  |  |  | c0.29 | 0.03 | 0.01 |  |  |
| v/c Ratio | 0.57 | 0.82 | 0.11 | 0.85 | 0.92 |  |  | 1.66 | 0.14 | 0.14 | 0.10 |  |
| Uniform Delay, d1 | 43.7 | 16.3 | 9.5 | 24.3 | 13.3 |  |  | 36.4 | 16.6 | 38.6 | 38.5 |  |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |  | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Incremental Delay, d2 | 30.5 | 5.4 | 0.1 | 12.3 | 11.1 |  |  | 344.0 | 0.0 | 0.3 | 0.1 |  |
| Delay (s) | 74.1 | 21.7 | 9.6 | 36.6 | 24.4 |  |  | 380.3 | 16.7 | 38.9 | 38.6 |  |
| Level of Service | E | C | A | D | C |  |  | F | B | D | D |  |
| Approach Delay (s) |  | 20.5 |  |  | 27.7 |  |  | 175.7 |  |  | 38.7 |  |
| Approach LOS |  | C |  |  | C |  |  | F |  |  | D |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 41.8 |  | HCM 2000 | evel of S | ervice |  | D |  |  |  |
| HCM 2000 Volume to Capacity ratio |  |  | 1.09 |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length (s) |  |  | 88.3 |  | Sum of lost | ime (s) |  |  | 14.5 |  |  |  |
| Intersection Capacity Utilization |  |  | 87.4\% |  | CU Level of | Service |  |  | E |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |
| ! Phase conflict between lane groups. |  |  |  |  |  |  |  |  |  |  |  |  |
| c Critical Lane Group |  |  |  |  |  |  |  |  |  |  |  |  |



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| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ | F | ${ }^{7}$ | $\uparrow$ | 「 | \% | $\stackrel{\text { F }}{ }$ |  | ${ }^{7}$ | $\uparrow$ | F |
| Trafic Volume (vph) | 64 | 867 | 118 | 58 | 897 | 118 | 117 | 124 | 11 | 195 | 197 | 205 |
| Future Volume (vph) | 64 | 867 | 118 | 58 | 897 | 118 | 117 | 124 | 11 | 195 | 197 | 205 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 5.5 | 4.0 | 4.0 | 5.5 | 4.0 | 4.0 | 5.5 |  | 4.0 | 5.5 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 |
| Frpb, ped/bikes | 1.00 | 1.00 | 0.98 | 1.00 | 1.00 | 0.99 | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 |
| Flpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 |
| Fit | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 0.99 |  | 1.00 | 1.00 | 0.85 |
| FIt Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1805 | 1812 | 1427 | 1805 | 1830 | 1550 | 1752 | 1840 |  | 1735 | 1827 | 1583 |
| Flt Permitted | 0.12 | 1.00 | 1.00 | 0.15 | 1.00 | 1.00 | 0.43 | 1.00 |  | 0.35 | 1.00 | 1.00 |
| Satd. Flow (perm) | 222 | 1812 | 1427 | 281 | 1830 | 1550 | 796 | 1840 |  | 632 | 1827 | 1583 |
| Peak-hour factor, PHF | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Adj. Flow (vph) | 67 | 903 | 123 | 60 | 934 | 123 | 122 | 129 | 11 | 203 | 205 | 214 |
| RTOR Reduction (vph) | 0 | 0 | 34 | 0 | , | 28 | 0 | 2 | 0 | 0 | 0 | 137 |
| Lane Group Flow (vph) | 67 | 903 | 89 | 60 | 934 | 95 | 122 | 138 | 0 | 203 | 205 | 77 |
| Confl. Peds. (\#/hr) | 2 |  | 1 | 1 |  | 2 |  |  | 1 | 1 |  |  |
| Confl. Bikes (\#/hr) |  |  | 1 |  |  | 2 |  |  |  |  |  |  |
| Heavy Vehicles (\%) | 0\% | 4\% | 10\% | 0\% | 3\% | 2\% | 3\% | 2\% | 0\% | 4\% | 4\% | 2\% |
| Bus Blockages (\#/hr) | 0 | 2 | 2 | 0 | 2 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| Turn Type | pm+pt | NA | pm+ov | pm+pt | NA | pm+ov | pm+pt | NA |  | pm+pt | NA | pm+ov |
| Protected Phases | 5 | 2 | 3 | 1 | 6 | 7 | 3 | 8 |  | 7 | 4 | 5 |
| Permitted Phases | 2 |  | 2 | 6 |  | 6 | 8 |  |  | 4 |  | 4 |
| Actuated Green, G (s) | 99.5 | 92.9 | 103.7 | 96.5 | 91.4 | 108.0 | 27.1 | 16.3 |  | 36.9 | 22.1 | 28.7 |
| Effective Green, g (s) | 99.5 | 92.9 | 103.7 | 96.5 | 91.4 | 108.0 | 27.1 | 16.3 |  | 36.9 | 22.1 | 28.7 |
| Actuated g/C Ratio | 0.66 | 0.62 | 0.69 | 0.64 | 0.61 | 0.72 | 0.18 | 0.11 |  | 0.25 | 0.15 | 0.19 |
| Clearance Time (s) | 4.0 | 5.5 | 4.0 | 4.0 | 5.5 | 4.0 | 4.0 | 5.5 |  | 4.0 | 5.5 | 4.0 |
| Vehicle Extension (s) | 1.5 | 4.5 | 0.2 | 1.5 | 4.5 | 0.2 | 0.2 | 2.0 |  | 0.2 | 2.0 | 1.5 |
| Lane Grp Cap (vph) | 217 | 1122 | 987 | 232 | 1115 | 1116 | 212 | 200 |  | 277 | 269 | 303 |
| v/s Ratio Prot | c0.01 | 0.50 | 0.01 | 0.01 | c0.51 | 0.01 | 0.04 | 0.08 |  | c0.08 | c0.11 | 0.01 |
| v/s Ratio Perm | 0.19 |  | 0.06 | 0.16 |  | 0.05 | 0.06 |  |  | 0.10 |  | 0.04 |
| $\mathrm{v} / \mathrm{c}$ Ratio | 0.31 | 0.80 | 0.09 | 0.26 | 0.84 | 0.09 | 0.58 | 0.69 |  | 0.73 | 0.76 | 0.26 |
| Uniform Delay, d1 | 21.9 | 21.6 | 7.6 | 19.2 | 23.3 | 6.2 | 54.2 | 64.4 |  | 48.6 | 61.4 | 51.5 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | 0.3 | 4.8 | 0.0 | 0.2 | 6.1 | 0.0 | 2.3 | 8.0 |  | 8.3 | 10.9 | 0.2 |
| Delay (s) | 22.2 | 26.4 | 7.6 | 19.4 | 29.5 | 6.3 | 56.5 | 72.4 |  | 57.0 | 72.3 | 51.7 |
| Level of Service | C | C | A | B | C | A | E | E |  | E | E | D |
| Approach Delay (s) |  | 24.0 |  |  | 26.4 |  |  | 65.0 |  |  | 60.2 |  |
| Approach LOS |  | C |  |  | C |  |  | E |  |  | E |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 2000 Control Delay |  |  | 35.6 |  | HCM 2000 | Level of S | Service |  | D |  |  |  |
| HCM 2000 Volume to Capacity ratio |  |  | 0.81 |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length (s) |  |  | 149.9 | Sum of lost time (s) |  |  |  |  | 19.0 |  |  |  |
| Intersection Capacity Utilization |  |  | 84.2\% | ICU Level of Service |  |  |  |  | E |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |

C Critical Lane Group


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |



## MOVEMENT SUMMARY

Site: 10 [SW Oregon St \& Murdock Rd]

## Year 2021 - Total Culdesac PM Peak Hour Conditions

Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | $\begin{aligned} & \text { OD } \\ & \text { Mov } \end{aligned}$ | Dema Total veh/h | $\begin{gathered} \text { lows } \\ \text { HV } \\ \% \\ \hline \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance ft | Prop. Queued | Effective Stop Rate per veh | Average Speed mph |
| South: Murdock Road |  |  |  |  |  |  |  |  |  |  |  |
| 3 | L2 | 91 | 1.0 | 0.231 | 5.4 | LOS A | 1.1 | 28.6 | 0.39 | 0.26 | 32.2 |
| 18 | R2 | 162 | 4.0 | 0.231 | 5.4 | LOS A | 1.1 | 28.6 | 0.39 | 0.26 | 30.5 |
| Appro |  | 253 | 2.9 | 0.231 | 5.4 | LOS A | 1.1 | 28.6 | 0.39 | 0.26 | 31.1 |
| East: Oregon St |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 421 | 1.0 | 0.687 | 12.4 | LOS B | 7.4 | 186.4 | 0.55 | 0.30 | 29.3 |
| 6 | T1 | 434 | 1.0 | 0.687 | 12.4 | LOS B | 7.4 | 186.4 | 0.55 | 0.30 | 28.9 |
| Approach |  | 855 | 1.0 | 0.687 | 12.4 | LOS B | 7.4 | 186.4 | 0.55 | 0.30 | 29.1 |
| West: Oregon St. |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T1 | 196 | 2.0 | 0.323 | 7.7 | LOS A | 1.6 | 39.8 | 0.59 | 0.53 | 31.9 |
|  | R2 | 87 | 2.0 | 0.323 | 7.7 | LOS A | 1.6 | 39.8 | 0.59 | 0.53 | 30.8 |
| Approach |  | 283 | 2.0 | 0.323 | 7.7 | LOS A | 1.6 | 39.8 | 0.59 | 0.53 | 31.6 |
| All Vehicles |  | 1391 | 1.6 | 0.687 | 10.2 | LOS B | 7.4 | 186.4 | 0.53 | 0.34 | 29.9 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: Same as Sign Control.
Vehicle movement LOS values are based on average delay and $\mathrm{v} / \mathrm{c}$ ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: US HCM 6.
HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies. Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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|  | $\rangle$ | $\rightarrow$ |  | 7 |  |  | 4 | $\dagger$ |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | $\hat{1}$ |  | \% | $\uparrow$ |  | \% | $\uparrow$ |  | \% | $\uparrow$ |  |
| Traffic Volume (veh/h) | 11 | 5 | 10 | 22 | 14 | 16 | 0 | 225 | 3 | 2 | 327 | 44 |
| Future Volume (Veh/h) | 11 | 5 | 10 | 22 | 14 | 16 | 0 | 225 | 3 | 2 | 327 | 44 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Hourly flow rate (vph) | 11 | 5 | 10 | 23 | 15 | 17 | 0 | 234 | 3 | 2 | 341 | 46 |
| Pedestrians |  |  |  |  | 1 |  |  | 1 |  |  | 2 |  |
| Lane Width (ft) |  |  |  |  | 12.0 |  |  | 12.0 |  |  | 12.0 |  |
| Walking Speed (fts) |  |  |  |  | 3.5 |  |  | 3.5 |  |  | 3.5 |  |
| Percent Blockage |  |  |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | TWLTL |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  | 2 |  |
| Upstream signal (ft) |  |  |  |  |  |  |  |  |  |  | 978 |  |
| pX, platoon unblocked | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |  | 0.90 |  |  |  |  |  |
| vC , conflicting volume | 628 | 606 | 365 | 595 | 628 | 238 | 387 |  |  | 238 |  |  |
| $\mathrm{vC1}$, stage 1 conf vol | 368 | 368 |  | 236 | 236 |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol | 260 | 238 |  | 358 | 391 |  |  |  |  |  |  |  |
| vCu , unblocked vol | 528 | 503 | 234 | 490 | 527 | 238 | 258 |  |  | 238 |  |  |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| $\mathrm{tC}, 2$ stage (s) | 6.1 | 5.5 |  | 6.1 | 5.5 |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| po queue free \% | 98 | 99 | 99 | 96 | 97 | 98 | 100 |  |  | 100 |  |  |
| cM capacity (veh/h) | 577 | 564 | 715 | 598 | 553 | 791 | 1155 |  |  | 1310 |  |  |
| Direction, Lane \# | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 | SB 1 | SB 2 |  |  |  |  |
| Volume Total | 11 | 15 | 23 | 32 | 0 | 237 | 2 | 387 |  |  |  |  |
| Volume Left | 11 | 0 | 23 | 0 | 0 | 0 | 2 | 0 |  |  |  |  |
| Volume Right | 0 | 10 | 0 | 17 | 0 | 3 | 0 | 46 |  |  |  |  |
| CSH | 577 | 656 | 598 | 658 | 1700 | 1700 | 1310 | 1700 |  |  |  |  |
| Volume to Capacity | 0.02 | 0.02 | 0.04 | 0.05 | 0.00 | 0.14 | 0.00 | 0.23 |  |  |  |  |
| Queue Length 95th (ft) | 1 | 2 | 3 | 4 | 0 | 0 | 0 | 0 |  |  |  |  |
| Control Delay (s) | 11.4 | 10.6 | 11.3 | 10.7 | 0.0 | 0.0 | 7.8 | 0.0 |  |  |  |  |
| Lane LOS | B | B | B | B |  |  | A |  |  |  |  |  |
| Approach Delay (s) | 10.9 |  | 11.0 |  | 0.0 |  | 0.0 |  |  |  |  |  |
| Approach LOS | B |  | B |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 1.3 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 34.8\% | ICU Level of Service |  |  |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |

## Appendix G Year 2021 Total Traffic Conditions - Mitigation Worksheets




## MOVEMENT SUMMARY

## Site: 9 [SW Oregon St \& Tonquin Rd]

Year 2021 - Total Traffic AM Peak Hour Conditions
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | $\begin{aligned} & \text { OD } \\ & \text { Mov } \end{aligned}$ | Dema Total veh/h | $\begin{gathered} \text { Fows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | f Queue <br> Distance <br> ft | Prop. Queued | Effective Stop Rate per veh | Average Speed mph |
| South: Tonquin Rd mer mer min |  |  |  |  |  |  |  |  |  |  |  |
| 3a | L1 | 153 | 3.0 | 0.371 | 10.2 | LOS B | 1.6 | 44.1 | 0.65 | 0.66 | 31.2 |
| 18 | R2 | 101 | 26.0 | 0.371 | 10.2 | LOS B | 1.6 | 44.1 | 0.65 | 0.66 | 29.0 |
| Appro |  | 255 | 12.1 | 0.371 | 10.2 | LOS B | 1.6 | 44.1 | 0.65 | 0.66 | 30.3 |
| East: Oregon St |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 1 | 14.0 | 0.247 | 6.1 | LOS A | 1.1 | 30.0 | 0.44 | 0.32 | 34.6 |
| 16a | R1 | 244 | 8.0 | 0.247 | 6.1 | LOS A | 1.1 | 30.0 | 0.44 | 0.32 | 35.7 |
| Approach |  | 245 | 8.0 | 0.247 | 6.1 | LOS A | 1.1 | 30.0 | 0.44 | 0.32 | 35.7 |
| West: Oregon St. EB |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & 5 b \\ & 2 \\ & 12 \end{aligned}$ | L3 | 88 | 3.0 | 0.593 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 36.9 |
|  | T1 | 474 | 2.0 | 0.593 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 35.5 |
|  | R2 | 486 | 1.0 | 0.593 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 34.2 |
| Approach |  | 1048 | 1.6 | 0.593 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 35.0 |
| All Vehicles |  | 1548 | 4.4 | 0.593 | 2.7 | LOS A | 1.6 | 44.1 | 0.18 | 0.16 | 34.2 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: Same as Sign Control.
Vehicle movement LOS values are based on average delay and $\mathrm{v} / \mathrm{c}$ ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: US HCM 6.
HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies. Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

## Site: 9 [SW Oregon St \& Tonquin Rd]

Year 2021 - Total Traffic PM Peak Hour Conditions
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | $\begin{aligned} & \text { OD } \\ & \text { Mov } \end{aligned}$ | Dema Total veh/h | $\begin{gathered} \text { Fows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | f Queue <br> Distance | Prop. Queued | Effective Stop Rate per veh | Average Speed mph |
| South: Tonquin Rd |  |  |  |  |  |  |  |  |  |  |  |
| 3a | L1 | 391 | 1.0 | 0.493 | 9.6 | LOS A | 3.1 | 78.3 | 0.62 | 0.53 | 31.4 |
| 18 | R2 | 96 | 5.0 | 0.493 | 9.6 | LOS A | 3.1 | 78.3 | 0.62 | 0.53 | 29.3 |
| Appr |  | 487 | 1.8 | 0.493 | 9.6 | LOS A | 3.1 | 78.3 | 0.62 | 0.53 | 31.0 |
| East: Oregon St |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 1 | 12.0 | 0.814 | 25.3 | LOS D | 9.3 | 242.7 | 0.93 | 1.15 | 26.8 |
| 16a | R1 | 640 | 6.0 | 0.814 | 25.3 | LOS D | 9.3 | 242.7 | 0.93 | 1.15 | 27.4 |
| Approach |  | 641 | 6.0 | 0.814 | 25.3 | LOS D | 9.3 | 242.7 | 0.93 | 1.15 | 27.4 |
| West: Oregon St. EB |  |  |  |  |  |  |  |  |  |  |  |
| $5 b$ | L3 | 93 | 4.0 | 0.358 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 36.8 |
| 2 | T1 | 208 | 3.0 | 0.358 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 35.5 |
| 12 | R2 | 311 | 3.0 | 0.358 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 34.2 |
| Appr |  | 612 | 3.2 | 0.358 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 35.0 |
| All Ve | cles | 1740 | 3.8 | 0.814 | 12.0 | LOS B | 9.3 | 242.7 | 0.52 | 0.57 | 30.7 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: Same as Sign Control.
Vehicle movement LOS values are based on average delay and $\mathrm{v} / \mathrm{c}$ ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: US HCM 6.
HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies. Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## Appendix H Year 2025 Background

 Conditions Worksheets|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

c Critical Lane Group




Analysis Period (min) 15
c Critical Lane Group

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ＊＊ | 个4 | F | ＊＊ | ¢4 | F | \％${ }^{\text {\％}}$ | 中t |  | \％ | 中1 |  |
| Traffic Volume（vph） | 75 | 1051 | 47 | 26 | 619 | 200 | 133 | 229 | 77 | 182 | 172 | 58 |
| Future Volume（vph） | 75 | 1051 | 47 | 26 | 619 | 200 | 133 | 229 | 77 | 182 | 172 | 58 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time（s） | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 |  | 4.0 | 5.5 |  |
| Lane Util．Factor | 0.97 | 0.95 | 1.00 | 0.97 | 0.95 | 1.00 | 0.97 | 0.95 |  | 1.00 | 0.95 |  |
| Frpb，ped／bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Flpb，ped／bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Frt | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 0.96 |  | 1.00 | 0.96 |  |
| Flt Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.95 | 1.00 |  |
| Satd．Flow（prot） | 3242 | 3299 | 1252 | 2334 | 3127 | 1381 | 3155 | 3013 |  | 1612 | 3007 |  |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.27 | 1.00 |  |
| Satd．Flow（perm） | 3242 | 3299 | 1252 | 2334 | 3127 | 1381 | 3155 | 3013 |  | 454 | 3007 |  |
| Peak－hour factor，PHF | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Adj．Flow（vph） | 81 | 1130 | 51 | 28 | 666 | 215 | 143 | 246 | 83 | 196 | 185 | 62 |
| RTOR Reduction（vph） | 0 | 0 | 15 | 0 | 0 | 59 | 0 | 26 | 0 | 0 | 24 | 0 |
| Lane Group Flow（vph） | 81 | 1130 | 36 | 28 | 666 | 156 | 143 | 303 | 0 | 196 | 223 | 0 |
| Confl．Bikes（\＃hr） |  |  | 3 |  |  | 1 |  |  |  |  |  |  |
| Heavy Vehicles（\％） | 8\％ | 9\％ | 28\％ | 50\％ | 15\％ | 16\％ | 11\％ | 10\％ | 31\％ | 12\％ | 12\％ | 26\％ |
| Bus Blockages（\＃／hr） | 0 | 2 | 2 | 0 | 2 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| Turn Type | Prot | NA | pt＋ov | Prot | NA | pt＋ov | Prot | NA |  | pm＋pt | NA |  |
| Protected Phases | 5 | 2 | 23 | 1 | 6 | 67 | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases |  |  |  |  |  |  |  |  |  | 4 |  |  |
| Actuated Green，G（s） | 7.7 | 89.7 | 104.7 | 4.0 | 86.0 | 108.9 | 9.5 | 19.9 |  | 41.3 | 27.8 |  |
| Effective Green， g （s） | 7.7 | 89.7 | 104.7 | 4.0 | 86.0 | 108.9 | 9.5 | 19.9 |  | 41.3 | 27.8 |  |
| Actuated g／C Ratio | 0.05 | 0.60 | 0.70 | 0.03 | 0.57 | 0.73 | 0.06 | 0.13 |  | 0.28 | 0.19 |  |
| Clearance Time（s） | 4.0 | 5.5 |  | 4.0 | 5.5 |  | 4.0 | 5.5 |  | 4.0 | 5.5 |  |
| Vehicle Extension（s） | 1.5 | 4.5 |  | 1.5 | 4.5 |  | 0.2 | 2.0 |  | 0.2 | 2.0 |  |
| Lane Grp Cap（vph） | 166 | 1972 | 873 | 62 | 1792 | 1002 | 199 | 399 |  | 259 | 557 |  |
| v／s Ratio Prot | c0．02 | c0．34 | 0.03 | 0.01 | 0.21 | 0.11 | 0.05 | 0.10 |  | c0．09 | 0.07 |  |
| v／s Ratio Perm |  |  |  |  |  |  |  |  |  | c0．12 |  |  |
| v／c Ratio | 0.49 | 0.57 | 0.04 | 0.45 | 0.37 | 0.16 | 0.72 | 0.76 |  | 0.76 | 0.40 |  |
| Uniform Delay，d1 | 69.2 | 18.4 | 7.0 | 71.9 | 17.4 | 6.3 | 68.9 | 62.7 |  | 45.5 | 53.8 |  |
| Progression Factor | 0.95 | 1.03 | 2.74 | 1.35 | 0.49 | 1.55 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Incremental Delay，d2 | 0.8 | 1.1 | 0.0 | 1.8 | 0.6 | 0.0 | 9.9 | 7.2 |  | 10.6 | 0.2 |  |
| Delay（s） | 66.6 | 20.2 | 19.3 | 99.1 | 9.1 | 9.9 | 78.8 | 69.9 |  | 56.1 | 53.9 |  |
| Level of Service | E | C | B | F | A | A | E | E |  | E | D |  |
| Approach Delay（s） |  | 23.1 |  |  | 12.0 |  |  | 72.6 |  |  | 54.9 |  |
| Approach LOS |  | C |  |  | B |  |  | E |  |  | D |  |


| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 2000 Control Delay | 32.0 | HCM 2000 Level of Service | C |
| HCM 2000 Volume to Capacity ratio | 0.64 |  | 19.0 |
| Actuated Cycle Length（s） | 150.0 | Sum of lost time（s） | B |
| Intersection Capacity Utilization | $63.5 \%$ | ICU Level of Service |  |
| Analysis Period（min） | 15 |  |  |
| C Critical Lane Group |  |  |  |



|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |



## MOVEMENT SUMMARY

Site: 10 [SW Oregon St \& Murdock Rd]
Year 2025 - Background AM Peak Hour Conditions
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | $\begin{aligned} & \text { OD } \\ & \text { Mov } \end{aligned}$ | Dema <br> Total veh/h | $\begin{gathered} \text { =lows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | f Queue Distance ft | Prop. Queued | Effective Stop Rate per veh | Average Speed mph |
| South: Murdock Road |  |  |  |  |  |  |  |  |  |  |  |
| 3 | L2 | 95 | 1.0 | 0.646 | 14.7 | LOS B | 5.5 | 137.7 | 0.80 | 0.89 | 28.9 |
| 18 | R2 | 464 | 1.0 | 0.646 | 14.7 | LOS B | 5.5 | 137.7 | 0.80 | 0.89 | 27.5 |
| Appro |  | 559 | 1.0 | 0.646 | 14.7 | LOS B | 5.5 | 137.7 | 0.80 | 0.89 | 27.7 |
| East: Oregon St |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 101 | 14.0 | 0.267 | 5.6 | LOS A | 1.3 | 34.8 | 0.28 | 0.14 | 32.4 |
| 6 | T1 | 202 | 8.0 | 0.267 | 5.6 | LOS A | 1.3 | 34.8 | 0.28 | 0.14 | 32.0 |
| Approach |  | 304 | 10.0 | 0.267 | 5.6 | LOS A | 1.3 | 34.8 | 0.28 | 0.14 | 32.1 |
| West: Oregon St. |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T1 | 440 | 2.0 | 0.423 | 7.3 | LOS A | 2.7 | 68.7 | 0.38 | 0.22 | 32.2 |
|  | R2 | 68 | 2.0 | 0.423 | 7.3 | LOS A | 2.7 | 68.7 | 0.38 | 0.22 | 31.1 |
| Approach |  | 508 | 2.0 | 0.423 | 7.3 | LOS A | 2.7 | 68.7 | 0.38 | 0.22 | 32.1 |
| All Vehicles |  | 1371 | 3.4 | 0.646 | 9.9 | LOS A | 5.5 | 137.7 | 0.53 | 0.48 | 30.2 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: Same as Sign Control.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: US HCM 6.
HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies. Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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|  | 4 |  |  | 7 |  |  | 4 | $\dagger$ | 7 |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\uparrow$ |  | \% | $\uparrow$ |  | \% | $\uparrow$ |  | ${ }^{*}$ | $\uparrow$ |  |
| Traffic Volume (veh/h) | 23 | 10 | 20 | 4 | 4 | 2 | 0 | 414 | 24 | 16 | 217 | 12 |
| Future Volume (Veh/h) | 23 | 10 | 20 | 4 | 4 | 2 | 0 | 414 | 24 | 16 | 217 | 12 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 25 | 11 | 22 | 4 | 4 | 2 | 0 | 445 | 26 | 17 | 233 | 13 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (ft) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (ft/s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | TWLTL |  |  | TWLTL |  |
| Median storage veh) |  |  |  |  |  |  |  | 2 |  |  | 2 |  |
| Upstream signal (ft) |  |  |  |  |  |  |  |  |  |  | 1007 |  |
| pX, platoon unblocked | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |  | 0.92 |  |  |  |  |  |
| vC , conflicting volume | 722 | 744 | 240 | 752 | 738 | 458 | 246 |  |  | 471 |  |  |
| VC1, stage 1 conf vol | 274 | 274 |  | 458 | 458 |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol | 449 | 471 |  | 294 | 280 |  |  |  |  |  |  |  |
| vCu , unblocked vol | 659 | 683 | 137 | 692 | 676 | 458 | 144 |  |  | 471 |  |  |
| tC, single (s) | 7.2 | 6.7 | 6.4 | 7.2 | 6.7 | 6.4 | 4.2 |  |  | 4.2 |  |  |
| $\mathrm{tC}, 2$ stage (s) | 6.2 | 5.7 |  | 6.2 | 5.7 |  |  |  |  |  |  |  |
| tF (s) | 3.6 | 4.1 | 3.4 | 3.6 | 4.1 | 3.4 | 2.3 |  |  | 2.3 |  |  |
| p0 queue free \% | 95 | 98 | 97 | 99 | 99 | 100 | 100 |  |  | 98 |  |  |
| cM capacity (veh/h) | 495 | 475 | 812 | 489 | 489 | 577 | 1260 |  |  | 1026 |  |  |
| Direction, Lane \# | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 | SB 1 | SB 2 |  |  |  |  |
| Volume Total | 25 | 33 | 4 | 6 | 0 | 471 | 17 | 246 |  |  |  |  |
| Volume Left | 25 | 0 | 4 | 0 | 0 | 0 | 17 | 0 |  |  |  |  |
| Volume Right | 0 | 22 | 0 | 2 | 0 | 26 | 0 | 13 |  |  |  |  |
| CSH | 495 | 657 | 489 | 515 | 1700 | 1700 | 1026 | 1700 |  |  |  |  |
| Volume to Capacity | 0.05 | 0.05 | 0.01 | 0.01 | 0.00 | 0.28 | 0.02 | 0.14 |  |  |  |  |
| Queue Length 95th (ft) | 4 | 4 | 1 | 1 | 0 | 0 | 1 | 0 |  |  |  |  |
| Control Delay (s) | 12.7 | 10.8 | 12.4 | 12.1 | 0.0 | 0.0 | 8.6 | 0.0 |  |  |  |  |
| Lane LOS | B | B | B | B |  |  | A |  |  |  |  |  |
| Approach Delay (s) | 11.6 |  | 12.2 |  | 0.0 |  | 0.6 |  |  |  |  |  |
| Approach LOS | B |  | B |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 1.2 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 37.9\% | ICU Level of Service |  |  |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

c Critical Lane Group

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |



c Critical Lane Group

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

c Critical Lane Group


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


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| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |



## MOVEMENT SUMMARY

Site: 10 [SW Oregon St \& Murdock Rd]
Year 2025 - Background PM Peak Hour Conditions
Roundabout


Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: Same as Sign Control.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: US HCM 6.
HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies. Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## Appendix I Year 2025 Total Traffic Conditions Worksheets

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

C Critical Lane Group

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |



## 4: Cipole Rd \& Tualatin-Sherwood Rd



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | 中 ${ }_{\text {d }}$ |  | \% | 中 ${ }^{\text {a }}$ |  | \% | $\uparrow$ |  | \% | $\uparrow$ |  |
| Traffic Volume (vph) | 110 | 1126 | 88 | 80 | 735 | 75 | 21 | 2 | 19 | 47 | , | 30 |
| Future Volume (vph) | 110 | 1126 | 88 | 80 | 735 | 75 | 21 | 2 | 19 | 47 | 9 | 30 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |


| Total Lost time (s) | 4.0 | 5.5 |  | 4.5 | 5.5 |  | 4.5 | 4.5 | 5.0 | 5.0 |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Lane Util. Factor | 1.00 | 0.95 |  | 1.00 | 0.95 |  | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Frpb, ped/bikes | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |
| Flpb, ped/bikes | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Frt | 1.00 | 0.99 |  | 1.00 | 0.99 |  | 1.00 | 0.86 |  | 1.00 | 0.88 |
| Flt Protected | 0.95 | 1.00 |  | 0.95 | 1.00 |  | 0.95 | 1.00 | 0.95 | 1.00 |  |
| Satd. Flow (prot) | 1703 | 3277 |  | 1597 | 3088 |  | 1597 | 1452 |  | 1289 | 1331 |
| Flt Permitted | 0.32 | 1.00 |  | 0.19 | 1.00 |  | 0.73 | 1.00 |  | 0.74 | 1.00 |
| Satd. Flow (perm) | 577 | 3277 |  | 321 | 3088 |  | 1229 | 1452 |  | 1008 | 1331 |
| Peak-hour factor, PHF | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Adj. Flow (vph) | 115 | 1173 | 92 | 83 | 766 | 78 | 22 | 2 | 20 | 49 | 9 |
| RTOR Reduction (vph) | 0 | 3 | 0 | 0 | 4 | 0 | 0 | 18 | 0 | 0 | 29 |
| Lane Group Flow (vph) | 115 | 1262 | 0 | 83 | 840 | 0 | 22 | 4 | 0 | 49 | 11 |

Confl. Bikes (\#hr)
5

| Heavy Vehicles (\%) | $6 \%$ | $8 \%$ | $13 \%$ | $13 \%$ | $15 \%$ | $13 \%$ | $13 \%$ | $13 \%$ | $13 \%$ | $40 \%$ | $13 \%$ | $30 \%$ |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Bus Blockages (\#hr) | 0 | 2 | 0 | 0 | 2 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| Turn Type | pm+pt | NA | pm+pt | NA |  | Perm | NA |  | Perm | NA |  |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |


| Permitted Phases | 2 |  | 6 |  | 8 |  | 4 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Actuated Green, G (s) | 123.2 | 116.1 | 124.5 | 117.0 | 11.9 | 11.9 | 11.4 | 11.4 |
| Effective Green, g (s) | 123.2 | 116.1 | 124.5 | 117.0 | 11.9 | 11.9 | 11.4 | 11.4 |
| Actuated g/C Ratio | 0.82 | 0.77 | 0.83 | 0.78 | 0.08 | 0.08 | 0.08 | 0.08 |
| Clearance Time (s) | 4.0 | 5.5 | 4.5 | 5.5 | 4.5 | 4.5 | 5.0 | 5.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 527 | 2536 | 330 | 2408 | 97 | 115 | 76 | 101 |
| v/s Ratio Prot | 0.01 | c0.39 | c0.01 | 0.27 |  | 0.00 |  | 0.01 |
| v/s Ratio Perm | 0.17 |  | 0.20 |  | 0.02 |  | c0.05 |  |
| v/c Ratio | 0.22 | 0.50 | 0.25 | 0.35 | 0.23 | 0.03 | 0.64 | 0.11 |
| Uniform Delay, d1 | 2.6 | 6.2 | 3.3 | 5.0 | 64.7 | 63.7 | 67.3 | 64.6 |
| Progression Factor | 1.00 | 1.00 | 0.97 | 0.94 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | 0.2 | 0.7 | 0.4 | 0.4 | 1.2 | 0.1 | 17.2 | 0.5 |
| Delay (s) | 2.9 | 6.9 | 3.6 | 5.1 | 65.9 | 63.8 | 84.6 | 65.1 |
| Level of Service | A | A | A | A | E | E | F | E |
| Approach Delay (s) |  | 6.6 |  | 4.9 |  | 64.9 |  | 75.8 |
| Approach LOS |  | A |  | A |  | E |  | E |

Intersection Summary

| HCM 2000 Control Delay | 9.5 | HCM 2000 Level of Service | A |
| :--- | ---: | :--- | ---: |
| HCM 2000 Volume to Capacity ratio | 0.50 |  | 15.0 |
| Actuated Cycle Length (s) | 150.0 | Sum of lost time (s) | B |
| Intersection Capacity Utilization | $60.1 \%$ | ICU Level of Service |  |

Analysis Period (min)
15
c Critical Lane Group

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \％${ }^{*}$ | 个4 | F | ＊＊ | ¢4 | ${ }^{+}$ | \％${ }^{1 / 4}$ | 中 ${ }^{\text {a }}$ |  | ${ }^{7}$ | 中t |  |
| Traffic Volume（vph） | 79 | 1062 | 51 | 26 | 663 | 200 | 151 | 229 | 77 | 182 | 172 | 76 |
| Future Volume（vph） | 79 | 1062 | 51 | 26 | 663 | 200 | 151 | 229 | 77 | 182 | 172 | 76 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time（s） | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 |  | 4.0 | 5.5 |  |
| Lane Util．Factor | 0.97 | 0.95 | 1.00 | 0.97 | 0.95 | 1.00 | 0.97 | 0.95 |  | 1.00 | 0.95 |  |
| Frpb，ped／bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Flpb，ped／bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Frt | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 0.96 |  | 1.00 | 0.95 |  |
| Flt Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.95 | 1.00 |  |
| Satd．Flow（prot） | 3242 | 3299 | 1252 | 2334 | 3127 | 1381 | 3155 | 3013 |  | 1612 | 2961 |  |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.27 | 1.00 |  |
| Satd．Flow（perm） | 3242 | 3299 | 1252 | 2334 | 3127 | 1381 | 3155 | 3013 |  | 454 | 2961 |  |
| Peak－hour factor，PHF | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Adj．Flow（vph） | 85 | 1142 | 55 | 28 | 713 | 215 | 162 | 246 | 83 | 196 | 185 | 82 |
| RTOR Reduction（vph） | 0 | 0 | 16 | 0 | 0 | 59 | 0 | 26 | 0 | 0 | 37 | 0 |
| Lane Group Flow（vph） | 85 | 1142 | 39 | 28 | 713 | 156 | 162 | 303 | 0 | 196 | 230 | 0 |
| Confl．Bikes（\＃／hr） |  |  | 3 |  |  | 1 |  |  |  |  |  |  |
| Heavy Vehicles（\％） | 8\％ | 9\％ | 28\％ | 50\％ | 15\％ | 16\％ | 11\％ | 10\％ | 31\％ | 12\％ | 12\％ | 26\％ |
| Bus Blockages（\＃／hr） | 0 | 2 | 2 | 0 | 2 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| Turn Type | Prot | NA | pt＋ov | Prot | NA | pt＋ov | Prot | NA |  | pm＋pt | NA |  |
| Protected Phases | 5 | 2 | 23 | 1 | 6 | 67 | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases |  |  |  |  |  |  |  |  |  | 4 |  |  |
| Actuated Green，G（s） | 7.8 | 89.9 | 105.8 | 4.0 | 86.1 | 108.8 | 10.4 | 19.9 |  | 41.1 | 26.7 |  |
| Effective Green， g （s） | 7.8 | 89.9 | 105.8 | 4.0 | 86.1 | 108.8 | 10.4 | 19.9 |  | 41.1 | 26.7 |  |
| Actuated g／C Ratio | 0.05 | 0.60 | 0.71 | 0.03 | 0.57 | 0.73 | 0.07 | 0.13 |  | 0.27 | 0.18 |  |
| Clearance Time（s） | 4.0 | 5.5 |  | 4.0 | 5.5 |  | 4.0 | 5.5 |  | 4.0 | 5.5 |  |
| Vehicle Extension（s） | 1.5 | 4.5 |  | 1.5 | 4.5 |  | 0.2 | 2.0 |  | 0.2 | 2.0 |  |
| Lane Grp Cap（vph） | 168 | 1977 | 883 | 62 | 1794 | 1001 | 218 | 399 |  | 257 | 527 |  |
| v／s Ratio Prot | c0．03 | c0．35 | 0.03 | 0.01 | 0.23 | 0.11 | 0.05 | 0.10 |  | c0．09 | 0.08 |  |
| v／s Ratio Perm |  |  |  |  |  |  |  |  |  | c0．12 |  |  |
| v／c Ratio | 0.51 | 0.58 | 0.04 | 0.45 | 0.40 | 0.16 | 0.74 | 0.76 |  | 0.76 | 0.44 |  |
| Uniform Delay，d1 | 69.2 | 18.4 | 6.7 | 71.9 | 17.6 | 6.4 | 68.5 | 62.7 |  | 45.6 | 54.9 |  |
| Progression Factor | 1.21 | 0.75 | 0.30 | 1.38 | 0.35 | 0.90 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Incremental Delay，d2 | 0.8 | 1.1 | 0.0 | 1.8 | 0.6 | 0.0 | 11.3 | 7.2 |  | 11.4 | 0.2 |  |
| Delay（s） | 84.3 | 15.0 | 2.1 | 101.3 | 6.8 | 5.8 | 79.8 | 69.9 |  | 57.0 | 55.2 |  |
| Level of Service | F | B | A | F | A | A | E | E |  | E | E |  |
| Approach Delay（s） |  | 19.0 |  |  | 9.3 |  |  | 73.2 |  |  | 56.0 |  |
| Approach LOS |  | B |  |  | A |  |  | E |  |  | E |  |


| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 2000 Control Delay | 29.8 | HCM 2000 Level of Service | C |
| HCM 2000 Volume to Capacity ratio | 0.65 |  | 19.0 |
| Actuated Cycle Length（s） | 150.0 | Sum of lost time（s） | C |
| Intersection Capacity Utilization | $65.2 \%$ | ICU Level of Service |  |
| Analysis Period（min） | 15 |  |  |
| C Critical Lane Group |  |  |  |



|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


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## MOVEMENT SUMMARY

Site: 10 [SW Oregon St \& Murdock Rd]

## Year 2025 - Total Culdesac AM Peak Hour Conditions

Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | $\begin{aligned} & \text { OD } \\ & \text { Mov } \end{aligned}$ | Dema Total veh/h | $\begin{gathered} \text { =lows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | f Queue <br> Distance <br> ft | Prop. Queued | Effective Stop Rate per veh | Average Speed mph |
| South: Murdock Road |  |  |  |  |  |  |  |  |  |  |  |
| 3 | L2 | 95 | 1.0 | 0.666 | 15.5 | LOS C | 5.8 | 146.8 | 0.82 | 0.93 | 28.6 |
| 18 | R2 | 474 | 1.0 | 0.666 | 15.5 | LOS C | 5.8 | 146.8 | 0.82 | 0.93 | 27.2 |
| Appro |  | 569 | 1.0 | 0.666 | 15.5 | LOS C | 5.8 | 146.8 | 0.82 | 0.93 | 27.4 |
| East: Oregon St |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 104 | 14.0 | 0.271 | 5.7 | LOS A | 1.3 | 35.5 | 0.28 | 0.14 | 32.4 |
| 6 | T1 | 205 | 8.0 | 0.271 | 5.7 | LOS A | 1.3 | 35.5 | 0.28 | 0.14 | 32.0 |
| Approach |  | 308 | 10.0 | 0.271 | 5.7 | LOS A | 1.3 | 35.5 | 0.28 | 0.14 | 32.1 |
| West: Oregon St. |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T1 | 451 | 2.0 | 0.433 | 7.4 | LOS A | 2.8 | 71.2 | 0.39 | 0.22 | 32.1 |
|  | R2 | 68 | 2.0 | 0.433 | 7.4 | LOS A | 2.8 | 71.2 | 0.39 | 0.22 | 31.0 |
| Approach |  | 519 | 2.0 | 0.433 | 7.4 | LOS A | 2.8 | 71.2 | 0.39 | 0.22 | 32.0 |
| All Vehicles |  | 1396 | 3.4 | 0.666 | 10.3 | LOS B | 5.8 | 146.8 | 0.54 | 0.49 | 30.0 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: Same as Sign Control.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: US HCM 6.
HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies. Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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|  | $\downarrow$ |  |  | 7 |  |  |  | $\dagger$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\hat{F}$ |  | \% | $\uparrow$ |  | \% | $\uparrow$ |  | ${ }^{*}$ | $\uparrow$ |  |
| Traffic Volume (veh/h) | 23 | 10 | 20 |  | 4 | 2 | 0 | 432 | 24 | 16 | 221 | 12 |
| Future Volume (Veh/h) | 23 | 10 | 20 | 4 | 4 | 2 | 0 | 432 | 24 | 16 | 221 | 12 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 25 | 11 | 22 | 4 | 4 | 2 | 0 | 465 | 26 | 17 | 238 | 13 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (ft) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (fts) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | TWLTL |  |  | TWLTL |  |
| Median storage veh) |  |  |  |  |  |  |  | 2 |  |  | 2 |  |
| Upstream signal (ft) |  |  |  |  |  |  |  |  |  |  | 1007 |  |
| pX, platoon unblocked | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |  | 0.92 |  |  |  |  |  |
| vC , conflicting volume | 748 | 770 | 244 | 778 | 763 | 478 | 251 |  |  | 491 |  |  |
| vC 1 , stage 1 conf vol | 278 | 278 |  | 478 | 478 |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol | 469 | 491 |  | 300 | 285 |  |  |  |  |  |  |  |
| vCu , unblocked vol | 683 | 707 | 137 | 716 | 700 | 478 | 144 |  |  | 491 |  |  |
| tC , single (s) | 7.2 | 6.7 | 6.4 | 7.2 | 6.7 | 6.4 | 4.2 |  |  | 4.2 |  |  |
| $\mathrm{tC}, 2$ stage (s) | 6.2 | 5.7 |  | 6.2 | 5.7 |  |  |  |  |  |  |  |
| tF (s) | 3.6 | 4.1 | 3.4 | 3.6 | 4.1 | 3.4 | 2.3 |  |  | 2.3 |  |  |
| p0 queue free \% | 95 | 98 | 97 | 99 | 99 | 100 | 100 |  |  | 98 |  |  |
| cM capacity (veh/h) | 483 | 465 | 809 | 478 | 479 | 562 | 1255 |  |  | 1008 |  |  |
| Direction, Lane \# | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 | SB1 | SB 2 |  |  |  |  |
| Volume Total | 25 | 33 | 4 | 6 | 0 | 491 | 17 | 251 |  |  |  |  |
| Volume Left | 25 | 0 | 4 | 0 | 0 | 0 | 17 | 0 |  |  |  |  |
| Volume Right | 0 | 22 | 0 | 2 | 0 | 26 | 0 | 13 |  |  |  |  |
| CSH | 483 | 649 | 478 | 504 | 1700 | 1700 | 1008 | 1700 |  |  |  |  |
| Volume to Capacity | 0.05 | 0.05 | 0.01 | 0.01 | 0.00 | 0.29 | 0.02 | 0.15 |  |  |  |  |
| Queue Length 95th (ft) | 4 | 4 | 1 | 1 | 0 | 0 | 1 | 0 |  |  |  |  |
| Control Delay (s) | 12.9 | 10.8 | 12.6 | 12.2 | 0.0 | 0.0 | 8.6 | 0.0 |  |  |  |  |
| Lane LOS | B | B | B | B |  |  | A |  |  |  |  |  |
| Approach Delay (s) | 11.7 |  | 12.4 |  | 0.0 |  | 0.5 |  |  |  |  |  |
| Approach LOS | B |  | B |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 1.1 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 38.8\% |  | CU Level | f Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


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c Critical Lane Group




Analysis Period (min) 15
c Critical Lane Group

|  | 4 |  | $\checkmark$ | 7 |  | 4 | 4 | 4 | 7 |  | $\ddagger$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7} 1$ | 44 | 「7 | ${ }^{7}$ | 中4 | 「 | ${ }^{7}$ | 㻢 |  | ${ }^{*}$ | 㻢 |  |
| Traffic Volume（vph） | 70 | 958 | 130 | 61 | 994 | 127 | 129 | 137 | 12 | 207 | 218 | 226 |
| Future Volume（vph） | 70 | 958 | 130 | 61 | 994 | 127 | 129 | 137 | 12 | 207 | 218 | 226 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time（s） | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 |  | 4.0 | 5.5 |  |
| Lane Util．Factor | 0.97 | 0.95 | 1.00 | 0.97 | 0.95 | 1.00 | 0.97 | 0.95 |  | 1.00 | 0.95 |  |
| Frpb，ped／bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Flpb，ped／bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Frt | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 0.99 |  | 1.00 | 0.92 |  |
| Fit Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.95 | 1.00 |  |
| Satd．Flow（prot） | 3502 | 3457 | 1456 | 3502 | 3491 | 1571 | 3400 | 3497 |  | 1735 | 3238 |  |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.49 | 1.00 |  |
| Satd．Flow（perm） | 3502 | 3457 | 1456 | 3502 | 3491 | 1571 | 3400 | 3497 |  | 893 | 3238 |  |
| Peak－hour factor，PHF | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Adj．Flow（vph） | 73 | 998 | 135 | 64 | 1035 | 132 | 134 | 143 | 12 | 216 | 227 | 235 |
| RTOR Reduction（vph） | 0 | 0 | 54 | 0 | 0 | 43 | 0 | 4 | 0 | 0 | 97 | 0 |
| Lane Group Flow（vph） | 73 | 998 | 81 | 64 | 1035 | 89 | 134 | 152 | 0 | 216 | 365 | 0 |
| Confl．Peds．（\＃／hr） | 2 |  | 1 | 1 |  | 2 |  |  | 1 | 1 |  |  |
| Confl．Bikes（\＃／hr） |  |  | 1 |  |  | 2 |  |  |  |  |  |  |
| Heavy Vehicles（\％） | 0\％ | 4\％ | 10\％ | 0\％ | 3\％ | 2\％ | 3\％ | 2\％ | 0\％ | 4\％ | 4\％ | 2\％ |
| Bus Blockages（\＃／hr） | 0 | 2 | 2 | 0 | 2 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| Turn Type | Prot | NA | pt＋ov | Prot | NA | $\mathrm{pt}+\mathrm{ov}$ | Prot | NA |  | pm＋pt | NA |  |
| Protected Phases | 5 | 2 | 23 | 1 | 6 | 67 | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases |  |  |  |  |  |  |  |  |  | 4 |  |  |
| Actuated Green，G（s） | 5.2 | 44.1 | 57.2 | 4.9 | 43.8 | 64.2 | 7.6 | 11.9 |  | 30.8 | 19.2 |  |
| Effective Green，g（s） | 5.2 | 44.1 | 57.2 | 4.9 | 43.8 | 64.2 | 7.6 | 11.9 |  | 30.8 | 19.2 |  |
| Actuated g／C Ratio | 0.05 | 0.47 | 0.60 | 0.05 | 0.46 | 0.68 | 0.08 | 0.13 |  | 0.32 | 0.20 |  |
| Clearance Time（s） | 4.0 | 5.5 |  | 4.0 | 5.5 |  | 4.0 | 5.5 |  | 4.0 | 5.5 |  |
| Vehicle Extension（s） | 1.5 | 4.5 |  | 1.5 | 4.5 |  | 0.2 | 2.0 |  | 0.2 | 2.0 |  |
| Lane Grp Cap（vph） | 192 | 1608 | 878 | 181 | 1612 | 1063 | 272 | 438 |  | 422 | 655 |  |
| v／s Ratio Prot | c0．02 | 0.29 | 0.06 | 0.02 | c0．30 | 0.06 | 0.04 | 0.04 |  | c0．08 | c0．11 |  |
| v／s Ratio Perm |  |  |  |  |  |  |  |  |  | 0.09 |  |  |
| v／c Ratio | 0.38 | 0.62 | 0.09 | 0.35 | 0.64 | 0.08 | 0.49 | 0.35 |  | 0.51 | 0.56 |  |
| Uniform Delay，d1 | 43.2 | 19.1 | 7.9 | 43.4 | 19.5 | 5.2 | 41.8 | 37.9 |  | 24.8 | 34.0 |  |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Incremental Delay，d2 | 0.5 | 1.0 | 0.1 | 0.4 | 1.1 | 0.1 | 0.5 | 0.2 |  | 0.4 | 0.6 |  |
| Delay（s） | 43.7 | 20.0 | 8.0 | 43.9 | 20.6 | 5.3 | 42.3 | 38.1 |  | 25.2 | 34.6 |  |
| Level of Service | D | C | A | D | C | A | D | D |  | C | C |  |
| Approach Delay（s） |  | 20.1 |  |  | 20.2 |  |  | 40.0 |  |  | 31.6 |  |
| Approach LOS |  | C |  |  | C |  |  | D |  |  | C |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 24.1 |  | HCM 2000 | evel of | ervice |  | C |  |  |  |
| HCM 2000 Volume to Capacity ratio |  |  | 0.61 |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length（s） |  |  | 94.8 |  | Sum of lost | time（s） |  |  | 19.0 |  |  |  |
| Intersection Capacity Utilization |  |  | 61．1\％ |  | CU Level | Service |  |  | B |  |  |  |
| Analysis Period（min） |  |  | 15 |  |  |  |  |  |  |  |  |  |
| c Critical Lane Group |  |  |  |  |  |  |  |  |  |  |  |  |




|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
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## MOVEMENT SUMMARY

Site: 10 [SW Oregon St \& Murdock Rd]

## Year 2025 - Total Culdesac PM Peak Hour Conditions

Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | $\begin{aligned} & \text { OD } \\ & \text { Mov } \end{aligned}$ | Dema Total veh/h | $\begin{gathered} \text { lows } \\ \text { HV } \\ \% \\ \hline \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance ft | Prop. Queued | Effective Stop Rate per veh | Average Speed mph |
| South: Murdock Road |  |  |  |  |  |  |  |  |  |  |  |
| 3 | L2 | 97 | 1.0 | 0.249 | 5.7 | LOS A | 1.2 | 31.3 | 0.41 | 0.28 | 32.1 |
| 18 | R2 | 173 | 4.0 | 0.249 | 5.7 | LOS A | 1.2 | 31.3 | 0.41 | 0.28 | 30.4 |
| Appro |  | 269 | 2.9 | 0.249 | 5.7 | LOS A | 1.2 | 31.3 | 0.41 | 0.28 | 31.0 |
| East: Oregon St |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 446 | 1.0 | 0.731 | 14.0 | LOS B | 8.7 | 218.9 | 0.63 | 0.36 | 28.7 |
| 6 | T1 | 458 | 1.0 | 0.731 | 14.0 | LOS B | 8.7 | 218.9 | 0.63 | 0.36 | 28.3 |
| Approach |  | 904 | 1.0 | 0.731 | 14.0 | LOS B | 8.7 | 218.9 | 0.63 | 0.36 | 28.5 |
| West: Oregon St. |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T1 | 206 | 2.0 | 0.350 | 8.2 | LOS A | 1.7 | 43.6 | 0.61 | 0.57 | 31.7 |
|  | R2 | 93 | 2.0 | 0.350 | 8.2 | LOS A | 1.7 | 43.6 | 0.61 | 0.57 | 30.6 |
| Approach |  | 299 | 2.0 | 0.350 | 8.2 | LOS A | 1.7 | 43.6 | 0.61 | 0.57 | 31.3 |
| All Vehicles |  | 1473 | 1.6 | 0.731 | 11.3 | LOS B | 8.7 | 218.9 | 0.58 | 0.39 | 29.5 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: Same as Sign Control.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: US HCM 6.
HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies. Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## Appendix J Year 2025 Total Traffic Conditions - Mitigation Worksheets


c Critical Lane Group

|  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |

## MOVEMENT SUMMARY

## Site: 9 [SW Oregon St \& Tonquin Rd]

Year 2025 - Total Traffic AM Peak Hour Conditions
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | $\begin{aligned} & \text { OD } \\ & \text { Mov } \end{aligned}$ | Dema Total veh/h | $\begin{gathered} \text { =lows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | f Queue <br> Distance <br> ft | Prop. Queued | Effective Stop Rate per veh | Average Speed mph |
| South: Tonquin Rd mer min |  |  |  |  |  |  |  |  |  |  |  |
| 3a | L1 | 161 | 3.0 | 0.403 | 11.0 | LOS B | 1.8 | 50.1 | 0.67 | 0.70 | 30.8 |
| 18 | R2 | 107 | 26.0 | 0.403 | 11.0 | LOS B | 1.8 | 50.1 | 0.67 | 0.70 | 28.7 |
| Appro |  | 268 | 12.2 | 0.403 | 11.0 | LOS B | 1.8 | 50.1 | 0.67 | 0.70 | 29.9 |
| East: Oregon St |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 1 | 14.0 | 0.266 | 6.3 | LOS A | 1.2 | 32.6 | 0.45 | 0.34 | 34.4 |
| 16a | R1 | 259 | 8.0 | 0.266 | 6.3 | LOS A | 1.2 | 32.6 | 0.45 | 0.34 | 35.5 |
| Approach |  | 260 | 8.0 | 0.266 | 6.3 | LOS A | 1.2 | 32.6 | 0.45 | 0.34 | 35.5 |
| West: Oregon St. EB |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & 5 b \\ & 2 \\ & 12 \end{aligned}$ | L3 | 92 | 3.0 | 0.626 | 0.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 36.9 |
|  | T1 | 499 | 2.0 | 0.626 | 0.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 35.5 |
|  | R2 | 516 | 1.0 | 0.626 | 0.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 34.2 |
| Approach |  | 1107 | 1.6 | 0.626 | 0.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 35.0 |
| All Vehicles |  | 1635 | 4.4 | 0.626 | 2.9 | LOS A | 1.8 | 50.1 | 0.18 | 0.17 | 34.1 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: Same as Sign Control.
Vehicle movement LOS values are based on average delay and $\mathrm{v} / \mathrm{c}$ ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: US HCM 6.
HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies. Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

## $\nabla$ Site: 9 [SW Oregon St \& Tonquin Rd]

Year 2025 - Total Traffic PM Peak Hour Conditions
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | $\begin{aligned} & \text { OD } \\ & \text { Mov } \end{aligned}$ | Dema Total veh/h | $\begin{gathered} \text { =lows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | f Queue <br> Distance <br> ft | Prop. Queued | Effective Stop Rate per veh | Average Speed mph |
| South: Tonquin Rd mer min |  |  |  |  |  |  |  |  |  |  |  |
| 3a | L1 | 414 | 1.0 | 0.533 | 10.5 | LOS B | 3.7 | 92.7 | 0.66 | 0.60 | 31.0 |
| 18 | R2 | 101 | 5.0 | 0.533 | 10.5 | LOS B | 3.7 | 92.7 | 0.66 | 0.60 | 29.0 |
| Appro |  | 515 | 1.8 | 0.533 | 10.5 | LOS B | 3.7 | 92.7 | 0.66 | 0.60 | 30.6 |
| East: Oregon St |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 1 | 12.0 | 0.886 | 33.9 | LOS D | 12.6 | 329.8 | 1.00 | 1.36 | 24.3 |
| 16a | R1 | 676 | 6.0 | 0.886 | 33.9 | LOS D | 12.6 | 329.8 | 1.00 | 1.36 | 24.8 |
| Approach |  | 677 | 6.0 | 0.886 | 33.9 | LOS D | 12.6 | 329.8 | 1.00 | 1.36 | 24.8 |
| West: Oregon St. EB |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & 5 b \\ & 2 \\ & 12 \end{aligned}$ | L3 | 100 | 4.0 | 0.379 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 36.8 |
|  | T1 | 221 | 3.0 | 0.379 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 35.5 |
|  | R2 | 327 | 3.0 | 0.379 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 34.1 |
| Approach |  | 648 | 3.2 | 0.379 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 35.0 |
| All Vehicles |  | 1840 | 3.8 | 0.886 | 15.4 | LOS C | 12.6 | 329.8 | 0.55 | 0.67 | 29.4 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: Same as Sign Control.
Vehicle movement LOS values are based on average delay and $\mathrm{v} / \mathrm{c}$ ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: US HCM 6.
HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies. Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## Appendix K Year 2021 SimTraffic Queuing Worksheets

## Summary of All Intervals

| Run Number | 101 | 102 | 103 | 104 | 105 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Start Time | $7: 10$ | $7: 10$ | $7: 10$ | $7: 10$ | $7: 10$ | $7: 10$ |
| End Time | $8: 20$ | $8: 20$ | $8: 20$ | $8: 20$ | $8: 20$ | $8: 20$ |
| Total Time (min) | 70 | 70 | 70 | 70 | 70 | 70 |
| Time Recorded (min) | 60 | 60 | 60 | 60 | 60 | 60 |
| \# of Intervals | 5 | 5 | 5 | 5 | 5 | 5 |
| \# of Recorded Intervals | 4 | 4 | 4 | 4 | 4 | 4 |
| Vehs Entered | 5152 | 5171 | 5196 | 5208 | 5078 | 5162 |
| Vehs Exited | 5199 | 5157 | 5145 | 5129 | 5120 | 5152 |
| Starting Vehs | 357 | 364 | 277 | 279 | 310 | 310 |
| Ending Vehs | 310 | 378 | 328 | 358 | 268 | 323 |
| Travel Distance (mi) | 6971 | 6931 | 6954 | 6853 | 6955 | 6933 |
| Travel Time (hr) | 335.6 | 342.9 | 319.0 | 346.1 | 314.8 | 331.7 |
| Total Delay (hr) | 141.6 | 149.8 | 126.1 | 154.4 | 121.8 | 138.7 |
| Total Stops | 7924 | 8297 | 7547 | 8807 | 7261 | 7967 |
| Fuel Used (gal) | 252.6 | 252.2 | 248.6 | 252.5 | 247.0 | 250.6 |

Interval \#0 Information Seeding

| Start Time $r: 10$ |  |
| :--- | ---: |
| End Time | $7: 20$ |
| Total Time (min) | 10 |
| Volumes adjusted by Growth Factors. |  |
|  |  |
| No data recorded this interval. |  |

Interval \#1 Information Recording1

| Start Time | $7: 20$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $7: 35$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number | 101 | 102 | 103 | 104 | 105 | Avg |
| Vehs Entered | 1252 | 1211 | 1254 | 1279 | 1195 | 1236 |
| Vehs Exited | 1304 | 1284 | 1262 | 1261 | 1187 | 1260 |
| Starting Vehs | 357 | 364 | 277 | 279 | 310 | 310 |
| Ending Vehs | 305 | 291 | 269 | 297 | 318 | 293 |
| Travel Distance (mi) | 1752 | 1687 | 1663 | 1694 | 1637 | 1687 |
| Travel Time (hr) | 79.9 | 81.2 | 76.1 | 77.3 | 72.3 | 77.4 |
| Total Delay (hr) | 31.1 | 34.3 | 29.9 | 30.0 | 27.1 | 30.5 |
| Total Stops | 1859 | 1944 | 1771 | 1761 | 1603 | 1788 |
| Fuel Used (gal) | 62.4 | 61.0 | 59.7 | 60.9 | 58.4 | 60.5 |

SimTraffic Simulation Summary
Year 2021 Total Traffic - Cul-de-sac AM Peak Hour Conditions
Interval \#2 Information Recording1

| Start Time | $7: 35$ |
| :--- | ---: |
| End Time | $7: 50$ |
| Total Time (min) | 15 |
| Volumes adjusted by PHF, Growth Factors. |  |


| Run Number | 101 | 102 | 103 | 104 | 105 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1446 | 1478 | 1436 | 1475 | 1448 | 1457 |
| Vehs Exited | 1371 | 1368 | 1358 | 1381 | 1360 | 1369 |
| Starting Vehs | 305 | 291 | 269 | 297 | 318 | 293 |
| Ending Vehs | 380 | 401 | 347 | 391 | 406 | 385 |
| Travel Distance (mi) | 1804 | 1834 | 1783 | 1834 | 1848 | 1821 |
| Travel Time (hr) | 91.7 | 89.1 | 82.8 | 87.3 | 85.4 | 87.3 |
| Total Delay (hr) | 41.3 | 37.8 | 33.1 | 35.6 | 33.8 | 36.3 |
| Total Stops | 2230 | 2192 | 2098 | 2176 | 2013 | 2142 |
| Fuel Used (gal) | 66.4 | 66.2 | 64.1 | 66.9 | 66.2 | 66.0 |

Interval \#3 Information Recording1

| Start Time | $7: 50$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $8: 05$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  | 101 | 102 | 103 | 104 | 105 |
| Run Number | 1233 | 1187 | 1245 | 1232 | 1258 | 1230 |
| Vehs Entered | 1294 | 1278 | 1283 | 1271 | 1369 | 1298 |
| Vehs Exited | 380 | 401 | 347 | 391 | 406 | 385 |
| Starting Vehs | 319 | 310 | 309 | 352 | 295 | 317 |
| Ending Vehs | 1722 | 1711 | 1773 | 1673 | 1775 | 1731 |
| Travel Distance (mi) | 81.8 | 84.9 | 80.9 | 93.0 | 84.1 | 84.9 |
| Travel Time (hr) | 33.9 | 37.5 | 31.7 | 46.3 | 34.6 | 36.8 |
| Total Delay (hr) | 1887 | 1988 | 1812 | 2496 | 2075 | 2054 |
| Total Stops | 62.3 | 62.4 | 63.0 | 63.2 | 63.5 | 62.9 |

## Interval \#4 Information Recording1

| Start Time | $8: 05$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $8: 20$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number | 101 | 102 | 103 | 104 | 105 | Avg |
| Rehs Entered | 1221 | 1295 | 1261 | 1222 | 1177 | 1231 |
| Vehs Exited | 1230 | 1227 | 1242 | 1216 | 1204 | 1226 |
| Starting Vehs | 319 | 310 | 309 | 352 | 295 | 317 |
| Ending Vehs | 310 | 378 | 328 | 358 | 268 | 323 |
| Travel Distance (mi) | 1693 | 1699 | 1735 | 1651 | 1695 | 1695 |
| Travel Time (hr) | 82.2 | 87.6 | 79.3 | 88.5 | 73.0 | 82.1 |
| Total Delay (hr) | 35.2 | 40.2 | 31.3 | 42.5 | 26.2 | 35.1 |
| Total Stops | 1948 | 2173 | 1866 | 2374 | 1570 | 1986 |
| Fuel Used (gal) | 61.4 | 62.6 | 61.8 | 61.4 | 59.0 | 61.3 |

Intersection: 1: Langer Farms Pkwy \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | NB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | R | L | TR | L | T | R | L | TR |
| Maximum Queue (ft) | 105 | 934 | 170 | 198 | 453 | 151 | 177 | 137 | 92 | 134 |
| Average Queue (ft) | 11 | 359 | 80 | 58 | 161 | 65 | 74 | 56 | 28 | 51 |
| 95th Queue (ft) | 54 | 770 | 195 | 135 | 353 | 126 | 145 | 115 | 70 | 108 |
| Link Distance (ft) |  | 1478 |  |  | 5035 |  | 1246 |  | 614 |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  | 300 | 140 |  |
| Storage Bay Dist (ft) | 175 |  | 145 | 200 |  | 375 |  |  | 0 |  |
| Storage Blk Time (\%) |  | 17 | 0 | 0 | 4 |  |  |  | 0 |  |

Intersection: 2: Oregon St \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | NB | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| irections Served | L | T | R | L | TR | LT | R | L |
| Maximum Queue (ft) | 84 | 635 | 175 | 374 | 583 | 224 | 455 | 39 |
| Average Queue (ft) | 8 | 226 | 47 | 87 | 241 | 107 | 174 | 4 |
| 95th Queue (ft) | 50 | 513 | 143 | 243 | 518 | 219 | 376 | 23 |
| Link Distance (ft) |  | 5035 |  |  | 598 |  | 3282 |  |
| Upstream Blk Time (\%) |  |  |  |  | 1 |  |  |  |
| Queuing Penalty (veh) |  |  |  |  | 6 |  |  |  |
| Storage Bay Dist (ft) | 250 |  | 150 | 350 |  | 200 |  | 75 |
| Storage Blk Time (\%) |  | 12 | 0 | 0 | 6 | 5 | 6 |  |
| Queuing Penalty (veh) |  | 15 | 0 | 0 | 6 | 19 | 6 |  |

Intersection: 3: Tualatin-Sherwood Rd \& Wildrose PI

| Movement | EB | EB | WB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | R |
| Maximum Queue (ft) | 62 | 220 | 105 | 88 | 35 |
| Average Queue (ft) | 7 | 45 | 8 | 19 | 5 |
| 95th Queue (ft) | 37 | 275 | 66 | 67 | 25 |
| Link Distance (ft) |  | 598 | 1104 |  | 698 |
| Upstream Blk Time (\%) |  | 0 |  |  |  |
| Queuing Penalty (veh) |  | 1 |  |  |  |
| Storage Bay Dist (ft) | 150 |  |  |  |  |
| Storage Blk Time (\%) |  | 2 |  |  |  |
| Queuing Penalty (veh) |  | 0 |  |  |  |

Intersection: 4: Cipole Rd \& Tualatin-Sherwood Rd

| Movement | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | T | R | L | TR | L | TR |
| Maximum Queue (ft) | 384 | 963 | 154 | 314 | 100 | 78 | 106 | 193 | 94 |
| Average Queue (ft) | 92 | 436 | 67 | 72 | 9 | 22 | 21 | 71 | 20 |
| 95th Queue (ft) | 291 | 1005 | 135 | 207 | 48 | 63 | 73 | 154 | 65 |
| Link Distance (ft) |  | 1104 |  | 819 |  |  | 419 |  | 805 |
| Upstream Blk Time (\%) |  | 0 |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  | 3 |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 360 |  | 250 |  | 130 | 200 |  | 300 |  |
| Storage Blk Time (\%) |  | 11 |  | 2 | 0 |  |  | 0 |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | T | R | L | T | R | L | TR | L | T | R |
| Maximum Queue (ft) | 344 | 829 | 400 | 211 | 663 | 400 | 385 | 446 | 315 | 449 | 92 |
| Average Queue (ft) | 89 | 618 | 62 | 34 | 309 | 70 | 123 | 245 | 187 | 150 | 32 |
| 95th Queue (ft) | 291 | 972 | 277 | 120 | 532 | 242 | 254 | 394 | 319 | 343 | 72 |
| Link Distance (ft) |  | 819 |  |  | 1233 |  |  | 922 |  | 1894 | 1894 |
| Upstream BIk Time (\%) |  | 3 |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  | 30 |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 375 |  | 375 | 375 |  | 375 | 400 |  | 300 |  |  |
| Storage Blk Time (\%) | 0 | 26 | 0 |  | 4 | 0 |  | 1 | 8 | 1 |  |
| Queuing Penalty (veh) | 0 | 30 | 1 |  | 8 | 0 |  | 1 | 12 | 1 |  |

Intersection: 6: 120th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | R | L | L | R |
| Maximum Queue (ft) | 10 | 4 | 95 | 195 | 207 |
| Average Queue (ft) | 0 | 0 | 18 | 75 | 60 |
| 95th Queue (ft) | 8 | 3 | 66 | 203 | 293 |
| Link Distance (ft) | 1233 |  |  |  | 751 |
| Upstream Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |
| Storage Bay Dist (ft) |  | 130 | 230 | 260 |  |
| Storage Blk Time (\%) |  |  |  | 6 | 1 |

Intersection: 7: 115th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | R | L | L | TR | LT | R | L | TR |
| Maximum Queue (ft) | 71 | 705 | 120 | 227 | 238 | 403 | 157 | 186 | 24 | 22 |
| Average Queue (ft) | 4 | 271 | 31 | 93 | 120 | 95 | 48 | 65 | 3 | 1 |
| 95th Queue (ft) | 43 | 621 | 102 | 181 | 204 | 264 | 116 | 143 | 14 | 12 |
| Link Distance (ft) |  | 1252 |  |  |  | 957 |  | 750 | 376 |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  | 95 | 225 | 225 |  | 170 |  | 90 |
| Storage Bay Dist (ft) | 240 | 13 | 0 | 0 | 1 | 1 | 0 | 1 |  |  |
| Storage Blk Time (\%) |  | 18 | 1 | 1 | 7 | 2 | 0 | 0 |  |  |
| Queuing Penalty (veh) |  | 18 |  |  |  |  |  |  |  |  |

Intersection: 8: 112th Ave/Avery St \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | R | L | T | R | L | TR | L | TR |
| Maximum Queue (ft) | 264 | 834 | 155 | 143 | 668 | 265 | 160 | 599 | 128 | 84 |
| Average Queue (ft) | 94 | 312 | 67 | 26 | 331 | 63 | 153 | 292 | 39 | 25 |
| 95th Queue (ft) | 201 | 715 | 170 | 104 | 566 | 217 | 175 | 548 | 104 | 67 |
| Link Distance (ft) |  | 957 |  |  | 1290 |  |  | 725 | 1359 |  |
| Upstream Blk Time (\%) |  | 0 |  |  |  |  |  | 0 |  |  |
| Queuing Penalty (veh) |  | 2 |  |  |  |  |  | 0 |  |  |
| Storage Bay Dist (ft) | 240 |  | 130 | 240 |  | 240 | 135 |  | 170 |  |
| Storage BIk Time (\%) | 0 | 14 | 0 |  | 13 | 0 | 52 | 1 | 0 |  |
| Queuing Penalty (veh) | 0 | 49 | 1 |  | 16 | 0 | 34 | 2 | 0 |  |

## Intersection: 9: Tonquin Rd \& Oregon St

| Movement | EB | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | R | L | L | R |
| Maximum Queue (ft) | 9 | 135 | 96 | 129 | 114 |
| Average Queue (ft) | 0 | 11 | 32 | 54 | 47 |
| 95th Queue (ft) | 5 | 72 | 75 | 93 | 86 |
| Link Distance (ft) | 372 |  |  |  | 552 |
| Upstream Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |
| Storage Bay Dist (ft) |  | 240 | 190 | 210 |  |
| Storage Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |

Intersection: 11: 124th Ave \& Blake Road

| Movement | EB | EB | WB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | TR | L |
| Maximum Queue (ft) | 60 | 73 | 47 | 59 | 4 | 63 |
| Average Queue (ft) | 16 | 27 | 4 | 7 | 0 | 6 |
| 95th Queue (ft) | 45 | 62 | 23 | 34 | 0 | 34 |
| Link Distance (ft) |  | 807 |  | 1348 | 1018 |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  | 150 |  |  | 150 |
| Storage Bay Dist (ft) | 150 |  |  |  |  |  |

Zone Summary
Zone wide Queuing Penalty: 329

## Summary of All Intervals

| Run Number | 111 | 112 | 113 | 114 | 115 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Start Time | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ |
| End Time | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ |
| Total Time (min) | 70 | 70 | 70 | 70 | 70 | 70 |
| Time Recorded (min) | 60 | 60 | 60 | 60 | 60 | 60 |
| \# of Intervals | 5 | 5 | 5 | 5 | 5 | 5 |
| \# of Recorded Intervals | 4 | 4 | 4 | 4 | 4 | 4 |
| Vehs Entered | 5782 | 5627 | 5799 | 5788 | 5827 | 5761 |
| Vehs Exited | 5699 | 5622 | 5666 | 5763 | 5807 | 5714 |
| Starting Vehs | 312 | 346 | 330 | 378 | 338 | 341 |
| Ending Vehs | 395 | 351 | 463 | 403 | 358 | 394 |
| Travel Distance (mi) | 8059 | 7847 | 8056 | 8099 | 8131 | 8038 |
| Travel Time (hr) | 422.8 | 378.1 | 415.6 | 412.3 | 448.9 | 415.5 |
| Total Delay (hr) | 197.4 | 158.5 | 189.4 | 185.6 | 222.3 | 190.6 |
| Total Stops | 10406 | 8990 | 10397 | 10991 | 10264 | 10213 |
| Fuel Used (gal) | 296.7 | 282.3 | 293.8 | 294.6 | 306.0 | 294.7 |

## Interval \#0 Information Seeding

| Start Time $r: 4: 45$ |  |
| :--- | ---: |
| End Time $r: 55$ |  |
| Total Time (min) | 10 |
| Volumes adjusted by Growth Factors. |  |
| No data recorded this interval. |  |

Interval \#1 Information Recording1

| Start Time | $4: 55$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $5: 10$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number |  | 111 | 112 | 113 | 114 | 115 |
| Vehs Entered | 1399 | 1410 | 1405 | 1371 | 1444 | 1407 |
| Vehs Exited | 1364 | 1387 | 1408 | 1371 | 1381 | 1382 |
| Starting Vehs | 312 | 346 | 330 | 378 | 338 | 341 |
| Ending Vehs | 347 | 369 | 327 | 378 | 401 | 355 |
| Travel Distance (mi) | 1918 | 1891 | 1952 | 1907 | 1970 | 1928 |
| Travel Time (hr) | 86.5 | 83.0 | 88.1 | 88.5 | 92.5 | 87.7 |
| Total Delay (hr) | 32.9 | 29.7 | 33.1 | 34.9 | 37.6 | 33.6 |
| Total Stops | 2259 | 2041 | 2098 | 2320 | 2384 | 2216 |
| Fuel Used (gal) | 67.8 | 66.6 | 69.2 | 68.0 | 71.2 | 68.6 |

SimTraffic Simulation Summary
Year 2021 Total Traffic Culdesac PM Peak Hour Conditions
Interval \#2 Information Recording2

| Start Time | $5: 10$ |
| :--- | ---: |
| End Time | $5: 25$ |
| Total Time $(\min )$ | 15 |
| Volumes adjusted by PHF, Growth Factors. |  |


| Run Number | 111 | 112 | 113 | 114 | 115 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1558 | 1481 | 1530 | 1562 | 1548 | 1537 |
| Vehs Exited | 1453 | 1414 | 1418 | 1462 | 1500 | 1448 |
| Starting Vehs | 347 | 369 | 327 | 378 | 401 | 355 |
| Ending Vehs | 452 | 436 | 439 | 478 | 449 | 449 |
| Travel Distance (mi) | 2079 | 2038 | 2005 | 2051 | 2077 | 2050 |
| Travel Time (hr) | 111.5 | 99.2 | 103.5 | 106.1 | 114.0 | 106.9 |
| Total Delay (hr) | 53.4 | 42.3 | 47.1 | 48.8 | 56.2 | 49.5 |
| Total Stops | 2871 | 2441 | 2540 | 2908 | 2700 | 2689 |
| Fuel Used (gal) | 76.8 | 73.0 | 73.3 | 75.1 | 77.5 | 75.1 |

Interval \#3 Information Recording2

| Start Time | $5: 25$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $5: 40$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number | 111 | 112 | 113 | 114 | 115 | Avg |
| Vehs Entered | 1432 | 1396 | 1412 | 1425 | 1482 | 1430 |
| Vehs Exited | 1422 | 1442 | 1428 | 1491 | 1458 | 1447 |
| Starting Vehs | 452 | 436 | 439 | 478 | 449 | 449 |
| Ending Vehs | 462 | 390 | 423 | 412 | 473 | 431 |
| Travel Distance (mi) | 2046 | 2018 | 2028 | 2065 | 2039 | 2039 |
| Travel Time (hr) | 120.2 | 103.7 | 111.9 | 113.5 | 120.9 | 114.1 |
| Total Delay (hr) | 63.1 | 47.4 | 54.9 | 55.6 | 64.0 | 57.0 |
| Total Stops | 2892 | 2484 | 2927 | 3070 | 2566 | 2787 |
| Fuel Used (gal) | 77.8 | 74.4 | 75.3 | 76.8 | 78.8 | 76.6 |

## Interval \#4 Information Recording2

| Start Time | $5: 40$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $5: 55$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number | 111 | 112 | 113 | 114 | 115 | Avg |
| Rehs Entered | 1393 | 1340 | 1452 | 1430 | 1353 | 1387 |
| Vehs Exited | 1460 | 1379 | 1412 | 1439 | 1468 | 1430 |
| Starting Vehs | 462 | 390 | 423 | 412 | 473 | 431 |
| Ending Vehs | 395 | 351 | 463 | 403 | 358 | 394 |
| Travel Distance (mi) | 2015 | 1900 | 2071 | 2077 | 2046 | 2022 |
| Travel Time (hr) | 104.6 | 92.2 | 112.1 | 104.2 | 121.4 | 106.9 |
| Total Delay (hr) | 47.9 | 39.1 | 54.3 | 46.3 | 64.6 | 50.4 |
| Total Stops | 2384 | 2024 | 2832 | 2693 | 2614 | 2511 |
| Fuel Used (gal) | 74.3 | 68.3 | 76.0 | 74.7 | 78.5 | 74.4 |

Intersection: 1: Langer Farms Pkwy \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | NB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | R | L | TR | L | T | R | L | TR |
| Maximum Queue (ft) | 134 | 622 | 170 | 225 | 885 | 239 | 169 | 104 | 165 | 585 |
| Average Queue (ft) | 17 | 318 | 123 | 149 | 404 | 133 | 61 | 40 | 60 | 390 |
| 95th Queue (ft) | 79 | 556 | 221 | 267 | 777 | 225 | 127 | 83 | 173 | 741 |
| Link Distance (ft) |  | 1478 |  |  | 5041 |  | 1246 |  | 614 |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  | 28 |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  | 300 | 140 | 0 |
| Storage Bay Dist (ft) | 175 |  | 145 | 200 |  | 375 |  |  | 66 |  |
| Storage Blk Time (\%) |  | 21 | 0 | 2 | 16 |  |  |  | 21 |  |

Intersection: 2: Oregon St \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | R | L | TR | LT | R | L | TR |
| Maximum Queue (ft) | 38 | 606 | 175 | 375 | 606 | 256 | 189 | 46 | 55 |
| Average Queue (ft) | 7 | 249 | 56 | 228 | 324 | 105 | 68 | 12 | 15 |
| 95th Queue (ft) | 28 | 543 | 161 | 405 | 638 | 223 | 137 | 38 | 43 |
| Link Distance (ft) |  | 5041 |  |  | 594 | 3284 | 3284 |  | 369 |
| Upstream Blk Time (\%) |  |  |  |  | 2 |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  | 25 |  |  |  |  |
| Storage Bay Dist (ft) | 250 |  | 150 | 350 |  |  |  | 75 |  |
| Storage Blk Time (\%) |  | 14 | 0 | 1 | 8 |  |  | 0 | 0 |

Intersection: 3: Tualatin-Sherwood Rd \& Wildrose PI

| Movement | EB | WB | SB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | R |
| Maximum Queue (ft) | 49 | 622 | 128 | 150 |
| Average Queue (ft) | 5 | 84 | 31 | 48 |
| 95th Queue (ft) | 27 | 388 | 87 | 137 |
| Link Distance (ft) |  | 1103 |  | 698 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) | 150 |  | 200 |  |
| Storage Blk Time (\%) |  |  |  | 3 |
| Queuing Penalty (veh) |  |  |  | 0 |

## Intersection: 4: Cipole Rd \& Tualatin-Sherwood Rd

| Movement | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | T | $R$ | $L$ | TR | L | TR |
| Maximum Queue (ft) | 152 | 502 | 274 | 819 | 118 | 160 | 148 | 134 | 156 |
| Average Queue (ft) | 31 | 209 | 26 | 431 | 6 | 72 | 53 | 46 | 64 |
| 95th Queue (ft) | 70 | 419 | 118 | 805 | 46 | 138 | 113 | 102 | 128 |
| Link Distance (ft) |  | 1103 |  | 819 |  |  | 440 |  | 805 |
| Upstream Blk Time (\%) |  |  |  | 0 |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  | 5 |  |  |  |  |  |
| Storage Bay Dist (ft) | 360 |  | 250 |  | 130 | 200 |  | 300 |  |
| Storage Blk Time (\%) |  | 2 |  | 20 | 0 | 0 | 0 |  |  |
| Queuing Penalty (veh) |  | 1 |  | 7 | 0 | 0 | 0 |  |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | T | R | L | T | R | L | TR | L | T | R |
| Maximum Queue (ft) | 124 | 784 | 400 | 400 | 1131 | 400 | 183 | 204 | 304 | 311 | 260 |
| Average Queue (ft) | 59 | 380 | 93 | 80 | 633 | 124 | 91 | 105 | 160 | 156 | 121 |
| 95th Queue (ft) | 119 | 702 | 337 | 286 | 1189 | 407 | 160 | 184 | 263 | 261 | 222 |
| Link Distance (ft) |  | 819 |  |  | 1233 |  |  | 892 |  | 1894 | 1894 |
| Upstream Blk Time (\%) |  | 0 |  |  | 1 |  |  |  |  |  |  |
| Queuing Penalty (veh) |  | 1 |  |  | 7 |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 100 |  | 375 | 375 |  | 375 | 400 |  | 300 |  |  |
| Storage Blk Time (\%) | 3 | 26 | 0 |  | 21 | 0 |  |  | 0 | 1 |  |
| Queuing Penalty (veh) | 34 | 48 | 0 |  | 37 | 0 |  |  | 0 | 1 |  |

Intersection: 6: 120th Ave \& Tualatin-Sherwood Rd

| Movement | WB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | L | T | L | R |
| Maximum Queue (ft) | 24 | 275 | 117 | 47 |
| Average Queue (ft) | 2 | 30 | 38 | 8 |
| 95th Queue (ft) | 15 | 231 | 98 | 26 |
| Link Distance (ft) |  | 1252 |  | 751 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) | 230 |  | 260 |  |
| Storage Blk Time (\%) |  | 2 |  |  |
| Queuing Penalty (veh) |  | 0 |  |  |

Intersection: 7: 115th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | R | L | L | TR | LT | R | L | TR |
| Maximum Queue (ft) | 80 | 657 | 120 | 73 | 206 | 406 | 182 | 247 | 52 | 33 |
| Average Queue (ft) | 8 | 279 | 22 | 10 | 44 | 179 | 69 | 123 | 12 | 5 |
| 95th Queue (ft) | 49 | 536 | 86 | 45 | 115 | 330 | 149 | 212 | 34 | 18 |
| Link Distance (ft) |  | 1252 |  |  |  | 957 |  | 750 |  | 376 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 240 |  | 95 | 225 | 225 |  | 170 |  | 90 |  |
| Storage Blk Time (\%) |  | 19 | 0 |  | 0 | 3 | 1 | 3 | 0 |  |
| Queuing Penalty (veh) |  | 14 | 0 |  | 0 | 2 | 2 | 4 | 0 |  |

Intersection: 8: 112th Ave/Avery St \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | R | L | T | R | L | TR | L | TR |
| Maximum Queue (ft) | 191 | 716 | 155 | 26 | 447 | 68 | 159 | 334 | 171 | 146 |
| Average Queue (ft) | 34 | 316 | 96 | 4 | 228 | 12 | 134 | 99 | 81 | 72 |
| 95th Queue (ft) | 114 | 646 | 193 | 19 | 422 | 43 | 183 | 279 | 149 | 136 |
| Link Distance (ft) |  | 957 |  |  | 1290 |  |  | 725 | 1359 |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  | 170 |  |
| Storage Bay Dist (ft) | 240 |  | 130 | 240 |  | 240 | 135 | 0 | 0 |  |
| Storage Blk Time (\%) |  | 15 | 0 |  | 7 |  | 28 | 0 | 0 |  |

## Intersection: 9: Tonquin Rd \& Oregon St

| Movement | EB | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | R | L | L | R |
| Maximum Queue (ft) | 4 | 24 | 75 | 235 | 673 |
| Average Queue (ft) | 0 | 1 | 25 | 179 | 234 |
| 95th Queue (ft) | 0 | 17 | 60 | 281 | 682 |
| Link Distance (ft) | 371 |  |  |  | 808 |
| Upstream Blk Time (\%) |  |  |  |  | 3 |
| Queuing Penalty (veh) |  |  |  |  | 0 |
| Storage Bay Dist (ft) |  | 240 | 190 | 210 |  |
| Storage Blk Time (\%) |  |  |  | 35 | 0 |
| Queuing Penalty (veh) |  |  |  | 32 | 0 |

Intersection: 11: 124th Ave \& Blake Road

| Movement | EB | EB | WB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | TR | L |
| Maximum Queue (ft) | 31 | 54 | 46 | 52 | 16 | 17 |
| Average Queue (ft) | 8 | 12 | 15 | 22 | 1 | 1 |
| 95th Queue (ft) | 30 | 38 | 41 | 49 | 9 | 8 |
| Link Distance (ft) |  | 787 |  | 949 | 1716 |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  | 150 |  |  | 150 |
| Storage Bay Dist (ft) | 150 |  |  |  |  |  |

## Zone Summary

[^8]Summary of All Intervals

| Run Number | 121 | 122 | 123 | 124 | 125 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Start Time | $7: 10$ | $7: 10$ | $7: 10$ | $7: 10$ | $7: 10$ | $7: 10$ |
| End Time | $8: 20$ | $8: 20$ | $8: 20$ | $8: 20$ | $8: 20$ | $8: 20$ |
| Total Time (min) | 70 | 70 | 70 | 70 | 70 | 70 |
| Time Recorded (min) | 60 | 60 | 60 | 60 | 60 | 60 |
| \# of Intervals | 5 | 5 | 5 | 5 | 5 | 5 |
| \# of Recorded Intervals | 4 | 4 | 4 | 4 | 4 | 4 |
| Vehs Entered | 5229 | 5297 | 5181 | 5283 | 5266 | 5251 |
| Vehs Exited | 5239 | 5316 | 5171 | 5287 | 5239 | 5249 |
| Starting Vehs | 307 | 332 | 295 | 304 | 316 | 309 |
| Ending Vehs | 297 | 313 | 305 | 300 | 343 | 307 |
| Travel Distance (mi) | 6918 | 7028 | 6974 | 6899 | 6964 | 6957 |
| Travel Time (hr) | 359.3 | 346.5 | 348.7 | 311.4 | 316.6 | 336.5 |
| Total Delay (hr) | 166.6 | 151.3 | 155.2 | 119.3 | 122.7 | 143.0 |
| Total Stops | 8877 | 8216 | 8584 | 7164 | 7294 | 8028 |
| Fuel Used (gal) | 256.3 | 255.9 | 254.3 | 245.6 | 248.1 | 252.1 |

Interval \#0 Information Seeding

| Start Time | $7: 10$ |
| :--- | ---: |
| End Time | $7: 20$ |
| Total Time (min) | 10 |
| Volumes adjusted by Growth Factors. |  |
| No data recorded this interval. |  |

Interval \#1 Information Recording1

| Start Time | $7: 20$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $7: 35$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number | 121 | 122 | 123 | 124 | 125 | Avg |
| Vehs Entered | 1248 | 1252 | 1230 | 1207 | 1229 | 1228 |
| Vehs Exited | 1198 | 1265 | 1252 | 1232 | 1264 | 1242 |
| Starting Vehs | 307 | 332 | 295 | 304 | 316 | 309 |
| Ending Vehs | 357 | 319 | 273 | 279 | 281 | 303 |
| Travel Distance (mi) | 1624 | 1702 | 1718 | 1616 | 1686 | 1669 |
| Travel Time (hr) | 79.8 | 84.3 | 75.0 | 68.1 | 73.5 | 76.2 |
| Total Delay (hr) | 34.6 | 37.1 | 27.4 | 23.2 | 26.6 | 29.8 |
| Total Stops | 1939 | 2050 | 1671 | 1483 | 1676 | 1762 |
| Fuel Used (gal) | 59.4 | 62.1 | 60.1 | 56.4 | 59.4 | 59.5 |

SimTraffic Simulation Summary
Year 2021 Total Traffic - Cipole Extension AM Peak Hour Conditions
Interval \#2 Information Recording2

| Start Time | $7: 35$ |
| :--- | ---: | :--- |
| End Time | $7: 50$ |
| Total Time (min) | 15 |
| Volumes adjusted by PHF, Growth Factors. |  |


| Run Number | 121 | 122 | 123 | 124 | 125 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1481 | 1496 | 1471 | 1486 | 1479 | 1486 |
| Vehs Exited | 1400 | 1428 | 1347 | 1429 | 1390 | 1401 |
| Starting Vehs | 357 | 319 | 273 | 279 | 281 | 303 |
| Ending Vehs | 438 | 387 | 397 | 336 | 370 | 380 |
| Travel Distance (mi) | 1834 | 1867 | 1826 | 1775 | 1801 | 1821 |
| Travel Time (hr) | 101.9 | 97.4 | 93.2 | 80.6 | 84.1 | 91.4 |
| Total Delay (hr) | 50.4 | 45.2 | 42.4 | 30.9 | 33.5 | 40.5 |
| Total Stops | 2696 | 2280 | 2457 | 1905 | 2042 | 2280 |
| Fuel Used (gal) | 68.8 | 69.2 | 66.6 | 63.5 | 65.0 | 66.6 |

Interval \#3 Information Recording3

| Start Time | 7:50 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| End Time | 8:05 |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number | 121 | 122 | 123 | 124 | 125 | Avg |
| Vehs Entered | 1276 | 1262 | 1249 | 1299 | 1259 | 1266 |
| Vehs Exited | 1391 | 1338 | 1281 | 1288 | 1330 | 1324 |
| Starting Vehs | 438 | 387 | 397 | 336 | 370 | 380 |
| Ending Vehs | 323 | 311 | 365 | 347 | 299 | 328 |
| Travel Distance (mi) | 1769 | 1758 | 1748 | 1762 | 1755 | 1758 |
| Travel Time (hr) | 96.9 | 89.4 | 98.1 | 81.2 | 82.3 | 89.6 |
| Total Delay (hr) | 47.7 | 40.9 | 49.8 | 32.3 | 33.6 | 40.9 |
| Total Stops | 2382 | 2172 | 2464 | 1911 | 1875 | 2161 |
| Fuel Used (gal) | 67.1 | 64.5 | 65.9 | 63.0 | 63.2 | 64.7 |

## Interval \#4 Information Recording4

| Start Time | 8:05 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| End Time | 8:20 |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number | 121 | 122 | 123 | 124 | 125 | Avg |
| Vehs Entered | 1224 | 1287 | 1231 | 1291 | 1299 | 1266 |
| Vehs Exited | 1250 | 1285 | 1291 | 1338 | 1255 | 1284 |
| Starting Vehs | 323 | 311 | 365 | 347 | 299 | 328 |
| Ending Vehs | 297 | 313 | 305 | 300 | 343 | 307 |
| Travel Distance (mi) | 1691 | 1701 | 1681 | 1746 | 1722 | 1708 |
| Travel Time (hr) | 80.7 | 75.4 | 82.3 | 81.4 | 76.6 | 79.3 |
| Total Delay (hr) | 33.8 | 28.1 | 35.7 | 33.0 | 28.9 | 31.9 |
| Total Stops | 1860 | 1714 | 1992 | 1865 | 1701 | 1830 |
| Fuel Used (gal) | 61.1 | 60.1 | 61.6 | 62.7 | 60.5 | 61.2 |

Intersection: 1: Langer Farms Pkwy \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | NB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | R | L | TR | L | T | R | L | TR |
| Maximum Queue (ft) | 28 | 983 | 170 | 197 | 372 | 154 | 184 | 184 | 95 | 122 |
| Average Queue (ft) | 6 | 369 | 81 | 60 | 144 | 67 | 72 | 62 | 21 | 49 |
| 95th Queue (ft) | 25 | 861 | 198 | 151 | 312 | 128 | 143 | 132 | 63 | 99 |
| Link Distance (ft) |  | 1478 |  |  | 5035 |  | 1246 |  | 614 |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 175 |  | 145 | 200 |  | 375 |  | 300 | 140 |  |
| Storage Blk Time (\%) |  | 17 | 0 | 0 | 3 |  | 0 | 0 | 0 | 0 |
| Queuing Penalty (veh) |  | 35 | 1 | 0 | 2 |  | 0 | 0 | 0 | 0 |

## Intersection: 2: Oregon St \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | NB | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | R | L | TR | LT | R | L |
| Maximum Queue (ft) | 88 | 534 | 175 | 334 | 567 | 224 | 268 | 47 |
| Average Queue (ft) | 8 | 195 | 44 | 78 | 237 | 83 | 130 | 6 |
| 95th Queue (ft) | 50 | 439 | 137 | 214 | 499 | 176 | 224 | 29 |
| Link Distance (ft) |  | 5035 |  |  | 598 |  | 3282 |  |
| Upstream Blk Time (\%) |  |  |  |  | 0 |  |  |  |
| Queuing Penalty (veh) |  |  |  |  | 2 |  |  |  |
| Storage Bay Dist (ft) | 250 |  | 150 | 350 |  | 200 |  | 75 |
| Storage Blk Time (\%) |  | 9 | 0 | 0 | 4 | 1 | 2 | 0 |
| Queuing Penalty (veh) |  | 11 | 0 | 0 | 4 | 3 | 1 | 0 |

## Intersection: 3: Tualatin-Sherwood Rd \& Wildrose PI

| Movement | EB | EB | WB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | R |
| Maximum Queue (ft) | 58 | 209 | 80 | 67 | 44 |
| Average Queue (ft) | 5 | 21 | 4 | 11 | 5 |
| 95th Queue (ft) | 35 | 162 | 46 | 43 | 26 |
| Link Distance (ft) |  | 598 | 1104 |  | 698 |
| Upstream Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |
| Storage Bay Dist (ft) | 150 | 1 |  |  |  |
| Storage Blk Time (\%) |  | 1 |  |  |  |
| Queuing Penalty (veh) |  | 0 |  |  |  |

Intersection: 4: Cipole Rd \& Tualatin-Sherwood Rd

| Movement | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | T | R | L | TR | L | TR |
| Maximum Queue (ft) | 384 | 817 | 191 | 307 | 93 | 70 | 83 | 264 | 315 |
| Average Queue (ft) | 92 | 375 | 57 | 75 | 14 | 18 | 16 | 123 | 95 |
| 95th Queue (ft) | 288 | 923 | 130 | 218 | 54 | 53 | 54 | 303 | 412 |
| Link Distance (ft) |  | 1104 |  | 819 |  |  | 419 |  | 805 |
| Upstream Blk Time (\%) |  | 0 |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  | 2 |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 360 |  | 250 |  | 130 | 200 |  | 300 |  |
| Storage Blk Time (\%) | 0 | 10 |  | 2 |  |  |  | 13 | 0 |
| Queuing Penalty (veh) | 0 | 10 |  | 3 |  |  |  | 5 | 0 |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| SB |  |  |  |  |  |  |  |  |  |  |
| irections Served | L | T | R | L | T | R | L | TR | L | T |
| Raximum Queue (ft) | 399 | 828 | 400 | 117 | 695 | 400 | 362 | 526 | 318 | 554 |
| Average Queue (ft) | 115 | 636 | 57 | 33 | 310 | 94 | 113 | 275 | 217 | 233 |
| 95th Queue (ft) | 348 | 991 | 261 | 88 | 550 | 320 | 242 | 450 | 361 | 628 |
| Link Distance (ft) |  | 819 |  |  | 1233 |  |  | 922 |  | 1894 |
| Upstream Blk Time (\%) |  | 5 |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  | 49 |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 375 |  | 375 | 375 |  | 375 | 400 |  | 300 |  |
| Storage Blk Time (\%) | 0 | 26 | 0 |  | 4 | 0 |  | 3 | 17 | 1 |
| Queuing Penalty (veh) | 0 | 29 | 1 |  | 9 | 0 |  | 4 | 27 | 2 |

Intersection: 6: 120th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | R | L | L | R |
| Maximum Queue (ft) | 4 | 30 | 84 | 159 | 76 |
| Average Queue (ft) | 0 | 1 | 15 | 50 | 20 |
| 95th Queue (ft) | 3 | 16 | 56 | 164 | 60 |
| Link Distance (ft) | 1233 |  |  |  | 751 |
| Upstream Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |
| Storage Bay Dist (ft) |  | 130 | 230 | 260 |  |
| Storage Blk Time (\%) |  |  |  | 2 |  |
| Queuing Penalty (veh) |  |  |  | 0 |  |

Intersection: 7: 115th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | R | L | L | TR | LT | R | L | TR |
| Maximum Queue (ft) | 126 | 738 | 119 | 230 | 242 | 527 | 188 | 280 | 23 | 39 |
| Average Queue (ft) | 7 | 327 | 35 | 109 | 142 | 125 | 49 | 69 | 2 | 3 |
| 95th Queue (ft) | 59 | 734 | 107 | 206 | 231 | 341 | 124 | 175 | 13 | 20 |
| Link Distance (ft) |  | 1252 |  |  |  | 957 |  | 750 | 376 |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 240 | 15 | 1 | 225 | 225 |  | 170 |  | 90 |  |
| Storage Blk Time (\%) |  | 15 | 0 | 1 | 1 | 0 | 2 |  |  |  |
| Queuing Penalty (veh) |  | 20 | 6 | 1 | 11 | 3 | 0 | 1 |  |  |

Intersection: 8: 112th Ave/Avery St \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | R | L | T | R | L | TR | L | TR |
| Maximum Queue (ft) | 264 | 851 | 155 | 227 | 783 | 265 | 160 | 673 | 148 | 113 |
| Average Queue (ft) | 90 | 353 | 64 | 34 | 363 | 83 | 152 | 331 | 39 | 31 |
| 95th Queue (ft) | 202 | 740 | 164 | 126 | 620 | 253 | 174 | 629 | 105 | 80 |
| Link Distance (ft) |  | 957 |  |  | 1290 |  |  | 725 | 1359 |  |
| Upstream Blk Time (\%) |  | 0 |  |  |  |  |  | 0 |  |  |
| Queuing Penalty (veh) |  | 1 |  |  |  |  |  | 0 |  |  |
| Storage Bay Dist (ft) | 240 |  | 130 | 240 |  | 240 | 135 |  | 170 |  |
| Storage Blk Time (\%) | 0 | 16 | 0 | 0 | 15 | 0 | 51 | 2 | 0 |  |
| Queuing Penalty (veh) | 2 | 57 | 0 | 0 | 18 | 0 | 33 | 4 | 0 |  |

## Intersection: 9: Tonquin Rd \& Oregon St

| Movement | EB | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | R | L | L | R |
| Maximum Queue (ft) | 13 | 138 | 87 | 164 | 103 |
| Average Queue (ft) | 0 | 6 | 32 | 60 | 43 |
| 95th Queue (ft) | 6 | 53 | 71 | 117 | 80 |
| Link Distance (ft) | 372 |  |  |  | 552 |
| Upstream Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |
| Storage Bay Dist (ft) |  | 240 | 190 | 210 |  |
| Storage Blk Time (\%) |  |  |  | 0 |  |
| Queuing Penalty (veh) |  |  |  | 0 |  |

Intersection: 11: 124th Ave \& Blake Road

| Movement | EB | EB | WB | WB | NB | NB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | TR | L | TR | L | TR | L |
| Maximum Queue (ft) | 62 | 62 | 39 | 57 | 35 | 10 | 50 |
| Average Queue (ft) | 18 | 25 | 3 | 9 | 3 | 0 | 6 |
| 95th Queue (ft) | 47 | 56 | 20 | 35 | 17 | 8 | 27 |
| Link Distance (ft) |  | 807 |  | 1348 |  | 1018 |  |
| Upstream BIk Time (\%) |  |  |  |  |  |  |  |
| Queuing Penalty (veh) 150 |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 150 |  | 150 |  | 150 |  | 150 |
| Storage Blk Time (\%) |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |

Intersection: 12: Blake Road \& Cipole Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | L | TR | LR |
| Maximum Queue (ft) | 18 | 4 | 51 |
| Average Queue (ft) | 1 | 0 | 10 |
| 95th Queue (ft) | 8 | 3 | 34 |
| Link Distance (ft) |  | 807 | 546 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) | 150 |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Zone Summary |  |  |  |

Zone wide Queuing Penalty: 362

Summary of All Intervals

| Run Number | 131 | 132 | 133 | 134 | 135 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Start Time | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ |
| End Time | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ |
| Total Time (min) | 70 | 70 | 70 | 70 | 70 | 70 |
| Time Recorded (min) | 60 | 60 | 60 | 60 | 60 | 60 |
| \# of Intervals | 5 | 5 | 5 | 5 | 5 | 5 |
| \# of Recorded Intervals | 4 | 4 | 4 | 4 | 4 | 4 |
| Vehs Entered | 5735 | 5705 | 5786 | 5779 | 5720 | 5742 |
| Vehs Exited | 5731 | 5741 | 5705 | 5800 | 5689 | 5732 |
| Starting Vehs | 366 | 380 | 342 | 413 | 372 | 369 |
| Ending Vehs | 370 | 344 | 423 | 392 | 403 | 383 |
| Travel Distance (mi) | 8016 | 7955 | 8027 | 8168 | 7851 | 8003 |
| Travel Time (hr) | 412.4 | 372.9 | 417.6 | 415.2 | 431.4 | 409.9 |
| Total Delay (hr) | 187.4 | 149.4 | 193.0 | 187.5 | 211.5 | 185.8 |
| Total Stops | 9876 | 9246 | 10415 | 10079 | 9683 | 9860 |
| Fuel Used (gal) | 294.5 | 284.7 | 294.0 | 297.5 | 294.6 | 293.1 |

Interval \#0 Information Seeding

| Start Time | $4: 45$ |
| :--- | ---: |
| End Time | $4: 55$ |
| Total Time (min) | 10 |
| Volumes adjusted by Growth Factors. |  |
| No data recorded this interval. |  |

Interval \#1 Information Recording

| Start Time | $4: 55$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $5: 10$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number | 131 | 132 | 133 | 134 | 135 | Avg |
| Vehs Entered | 1448 | 1385 | 1356 | 1368 | 1384 | 1385 |
| Vehs Exited | 1423 | 1421 | 1362 | 1440 | 1410 | 1413 |
| Starting Vehs | 366 | 380 | 342 | 413 | 372 | 369 |
| Ending Vehs | 391 | 344 | 336 | 341 | 346 | 347 |
| Travel Distance (mi) | 2032 | 1954 | 1946 | 2035 | 1931 | 1980 |
| Travel Time (hr) | 101.8 | 89.8 | 87.2 | 94.9 | 93.7 | 93.5 |
| Total Delay (hr) | 45.0 | 34.8 | 32.6 | 38.2 | 39.6 | 38.0 |
| Total Stops | 2454 | 2291 | 2124 | 2304 | 2309 | 2299 |
| Fuel Used (gal) | 73.5 | 69.4 | 67.9 | 72.6 | 69.6 | 70.6 |

SimTraffic Simulation Summary
Year 2021 Total Traffic Cipole Extension PM Peak Hour Conditions
Interval \#2 Information Recording

| Start Time | $5: 10$ |
| :--- | ---: |
| End Time | $5: 25$ |
| Total Time $(\min )$ | 15 |
| Volumes adjusted by PHF, Growth Factors. |  |


| Run Number | 131 | 132 | 133 | 134 | 135 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1482 | 1549 | 1587 | 1592 | 1566 | 1554 |
| Vehs Exited | 1436 | 1454 | 1455 | 1464 | 1417 | 1444 |
| Starting Vehs | 391 | 344 | 336 | 341 | 346 | 347 |
| Ending Vehs | 437 | 439 | 468 | 469 | 495 | 460 |
| Travel Distance (mi) | 1966 | 2057 | 2048 | 2082 | 2019 | 2034 |
| Travel Time (hr) | 102.6 | 99.1 | 104.1 | 111.0 | 108.1 | 105.0 |
| Total Delay (hr) | 47.2 | 41.6 | 46.9 | 52.8 | 51.7 | 48.0 |
| Total Stops | 2516 | 2511 | 2791 | 2919 | 2479 | 2642 |
| Fuel Used (gal) | 72.7 | 74.2 | 74.8 | 76.8 | 75.0 | 74.7 |

Interval \#3 Information Recording

| Start Time | $5: 25$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $5: 40$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number | 131 | 132 | 133 | 134 | 135 | Avg |
| Vehs Entered | 1401 | 1386 | 1440 | 1386 | 1360 | 1390 |
| Vehs Exited | 1416 | 1461 | 1453 | 1439 | 1443 | 1443 |
| Starting Vehs | 437 | 439 | 468 | 469 | 495 | 460 |
| Ending Vehs | 422 | 364 | 455 | 416 | 412 | 416 |
| Travel Distance (mi) | 2026 | 2004 | 2032 | 2025 | 1942 | 2006 |
| Travel Time (hr) | 109.0 | 95.7 | 110.6 | 108.4 | 121.4 | 109.0 |
| Total Delay (hr) | 52.0 | 39.6 | 53.7 | 52.1 | 66.9 | 52.9 |
| Total Stops | 2669 | 2270 | 2916 | 2649 | 2796 | 2659 |
| Fuel Used (gal) | 75.5 | 72.2 | 75.3 | 74.6 | 76.2 | 74.8 |

## Interval \#4 Information Recording

| Start Time | $5: 40$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $5: 55$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| R |  |  |  |  |  |  |
| Run Number | 131 | 132 | 133 | 134 | 135 | Avg |
| Vehs Entered | 1404 | 1385 | 1403 | 1433 | 1410 | 1413 |
| Vehs Exited | 1456 | 1405 | 1435 | 1457 | 1419 | 1432 |
| Starting Vehs | 422 | 364 | 455 | 416 | 412 | 416 |
| Ending Vehs | 370 | 344 | 423 | 392 | 403 | 383 |
| Travel Distance (mi) | 1992 | 1941 | 2001 | 2026 | 1959 | 1984 |
| Travel Time (hr) | 99.0 | 88.2 | 115.6 | 101.0 | 108.2 | 102.4 |
| Total Delay (hr) | 43.2 | 33.5 | 59.7 | 44.4 | 53.3 | 46.8 |
| Total Stops | 2237 | 2174 | 2584 | 2207 | 2099 | 2259 |
| Fuel Used (gal) | 72.8 | 69.0 | 76.0 | 73.5 | 73.8 | 73.0 |

## Intersection: 1: Langer Farms Pkwy \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | NB | NB | NB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | T | R | L | TR | L | T | R | L | TR |
| Maximum Queue (ft) | 169 | 733 | 170 | 225 | 912 | 284 | 129 | 113 | 165 | 625 |
| Average Queue (ft) | 18 | 311 | 117 | 157 | 418 | 143 | 64 | 41 | 73 | 490 |
| 95th Queue (ft) | 80 | 604 | 222 | 268 | 807 | 252 | 115 | 82 | 193 | 775 |
| Link Distance (ft) |  | 1478 |  |  | 5041 |  | 1246 |  |  | 614 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  | 42 |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  | 0 |
| Storage Bay Dist (ft) | 175 |  | 145 | 200 |  | 375 |  | 300 | 140 |  |
| Storage Blk Time (\%) |  | 20 | 0 | 2 | 18 |  |  |  | 0 | 85 |
| Queuing Penalty (veh) |  | 55 | 1 | 22 | 38 |  |  |  | 0 | 27 |

## Intersection: 2: Oregon St \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | R | L | TR | LT | R | L | TR |
| Maximum Queue (ft) | 87 | 740 | 175 | 374 | 604 | 222 | 162 | 41 | 55 |
| Average Queue (ft) | 9 | 269 | 49 | 209 | 295 | 102 | 62 | 10 | 14 |
| 95th Queue (ft) | 64 | 580 | 151 | 384 | 582 | 192 | 116 | 33 | 42 |
| Link Distance (ft) |  | 5041 |  |  | 594 | 3284 | 3284 | 369 |  |
| Upstream Blk Time (\%) |  |  |  |  | 1 |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  | 19 |  |  |  |  |
| Storage Bay Dist (ft) | 250 |  | 150 | 350 |  |  |  | 75 | 0 |

## Intersection: 3: Tualatin-Sherwood Rd \& Wildrose PI

| Movement | EB | WB | SB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | R |
| Maximum Queue (ft) | 28 | 627 | 78 | 109 |
| Average Queue (ft) | 3 | 48 | 25 | 32 |
| 95th Queue (ft) | 18 | 305 | 71 | 87 |
| Link Distance (ft) |  | 1103 |  | 698 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) | 150 |  | 200 |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 4: Cipole Rd \& Tualatin-Sherwood Rd

| Movement | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | T | R | L | TR | L | TR |
| Maximum Queue (ft) | 193 | 494 | 273 | 724 | 131 | 110 | 118 | 117 | 171 |
| Average Queue (ft) | 32 | 177 | 23 | 345 | 7 | 44 | 36 | 49 | 67 |
| 95th Queue (ft) | 110 | 385 | 113 | 711 | 52 | 87 | 82 | 101 | 136 |
| Link Distance (ft) |  | 1103 |  | 819 |  |  | 440 |  | 805 |
| Upstream Blk Time (\%) |  |  |  | 0 |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  | 1 |  |  |  | 300 |  |
| Storage Bay Dist (ft) | 360 |  | 250 |  | 130 | 200 |  |  |  |
| Storage Blk Time (\%) |  | 1 |  | 15 | 0 |  |  |  |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | R | L | T | R | L | TR | L | T | R |
| Maximum Queue (ft) | 273 | 712 | 399 | 399 | 917 | 331 | 159 | 216 | 292 | 423 | 266 |
| Average Queue (ft) | 49 | 336 | 51 | 64 | 463 | 84 | 84 | 114 | 171 | 182 | 123 |
| 95th Queue (ft) | 157 | 593 | 229 | 243 | 888 | 321 | 142 | 198 | 298 | 378 | 225 |
| Link Distance (ft) |  | 819 |  |  | 1233 |  |  | 892 |  | 1894 | 1894 |
| Upstream Blk Time (\%) |  | 0 |  |  | 0 |  |  |  |  |  |  |
| Queuing Penalty (veh) |  | 0 |  |  | 0 |  |  |  | 300 |  |  |
| Storage Bay Dist (ft) | 375 |  | 375 | 375 |  | 375 | 400 |  | 7 | 1 |  |
| Storage BIk Time (\%) |  | 6 | 0 |  | 14 | 0 |  |  | 14 | 1 |  |
| Queuing Penalty (veh) |  | 9 | 0 |  | 24 | 0 |  |  |  |  |  |

Intersection: 6: 120th Ave \& Tualatin-Sherwood Rd

| Movement | WB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | L | T | L | R |
| Maximum Queue (ft) | 30 | 49 | 82 | 35 |
| Average Queue (ft) | 3 | 3 | 27 | 9 |
| 95th Queue (ft) | 16 | 44 | 77 | 25 |
| Link Distance (ft) |  | 1252 |  | 751 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) | 230 |  | 260 |  |
| Storage Blk Time (\%) |  | 0 |  |  |
| Queuing Penalty (veh) |  | 0 |  |  |

Intersection: 7: 115th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | R | L | L | TR | LT | R | L | TR |
| Maximum Queue (ft) | 97 | 569 | 120 | 51 | 219 | 382 | 187 | 234 | 60 | 34 |
| Average Queue (ft) | 11 | 263 | 17 | 10 | 48 | 164 | 72 | 118 | 13 | 6 |
| 95th Queue (ft) | 54 | 507 | 76 | 36 | 135 | 305 | 150 | 211 | 44 | 21 |
| Link Distance (ft) |  | 1252 |  |  |  | 957 |  | 750 |  | 376 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 240 |  | 95 | 225 | 225 |  | 170 |  | 90 |  |
| Storage Blk Time (\%) |  | 18 | 0 |  | 0 | 3 | 1 | 3 | 0 |  |
| Queuing Penalty (veh) |  | 13 | 0 |  | 0 | 1 | 1 | 4 | 0 |  |

Intersection: 8: 112th Ave/Avery St \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | R | L | T | R | L | TR | L | TR |
| Maximum Queue (ft) | 226 | 623 | 155 | 79 | 521 | 265 | 159 | 304 | 159 | 155 |
| Average Queue (ft) | 39 | 312 | 102 | 5 | 242 | 28 | 130 | 96 | 79 | 66 |
| 95th Queue (ft) | 130 | 598 | 202 | 44 | 442 | 138 | 181 | 269 | 143 | 127 |
| Link Distance (ft) |  | 957 |  |  | 1290 |  |  | 725 | 1359 |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  | 170 |  |
| Storage Bay Dist (ft) | 240 |  | 130 | 240 |  | 240 | 135 | 1 | 0 |  |
| Storage Blk Time (\%) |  | 16 | 0 |  | 7 | 0 | 23 |  | 0 |  |

## Intersection: 9: Tonquin Rd \& Oregon St

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | R | L | L | R |
| Maximum Queue (ft) | 23 | 67 | 235 | 550 |
| Average Queue (ft) | 1 | 22 | 172 | 172 |
| 95th Queue (ft) | 17 | 57 | 278 | 519 |
| Link Distance (ft) |  |  |  | 808 |
| Upstream Blk Time (\%) |  |  |  | 0 |
| Queuing Penalty (veh) |  |  |  | 0 |
| Storage Bay Dist (ft) | 240 | 190 | 210 |  |
| Storage Blk Time (\%) |  |  | 33 | 0 |
| Queuing Penalty (veh) |  |  | 29 | 0 |

Intersection: 11: 124th Ave \& Blake Road

| Movement | EB | EB | WB | WB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | L | TR |
| Maximum Queue (ft) | 55 | 48 | 56 | 58 | 21 | 10 | 10 |
| Average Queue (ft) | 18 | 21 | 16 | 22 | 1 | 0 | 0 |
| 95th Queue (ft) | 46 | 45 | 43 | 50 | 10 | 5 | 8 |
| Link Distance (ft) |  | 787 |  | 949 |  |  | 892 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 150 |  | 150 |  | 150 | 150 |  |
| Storage Blk Time (\%) |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |

Intersection: 12: Blake Road \& Cipole Road

| Movement | EB | SB |
| :--- | ---: | ---: |
| Directions Served | L | LR |
| Maximum Queue (ft) | 12 | 72 |
| Average Queue (ft) | 0 | 33 |
| 95th Queue (ft) | 6 | 63 |
| Link Distance (ft) |  | 300 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) | 150 |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Zone Summary |  |  |

Zone wide Queuing Penalty: 386

## Appendix L Year 2025 SimTraffic Queuing Worksheets

SimTraffic Simulation Summary
Year 2025 Total Traffic Culdesac AM Peak Hour Conditions
Summary of All Intervals

| Run Number | 101 | 102 | 103 | 104 | 105 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Start Time | $7: 10$ | $7: 10$ | $7: 10$ | $7: 10$ | $7: 10$ | $7: 10$ |
| End Time | $8: 20$ | $8: 20$ | $8: 20$ | $8: 20$ | $8: 20$ | $8: 20$ |
| Total Time (min) | 70 | 70 | 70 | 70 | 70 | 70 |
| Time Recorded (min) | 60 | 60 | 60 | 60 | 60 | 60 |
| \# of Intervals | 5 | 5 | 5 | 5 | 5 | 5 |
| \# of Recorded Intervals | 4 | 4 | 4 | 4 | 4 | 4 |
| Vehs Entered | 5523 | 5523 | 5579 | 5675 | 5487 | 5559 |
| Vehs Exited | 5558 | 5593 | 5583 | 5669 | 5458 | 5569 |
| Starting Vehs | 311 | 318 | 300 | 268 | 266 | 293 |
| Ending Vehs | 276 | 248 | 296 | 274 | 295 | 270 |
| Travel Distance (mi) | 7539 | 7605 | 7595 | 7675 | 7484 | 7580 |
| Travel Time (hr) | 304.1 | 300.7 | 301.5 | 307.9 | 302.1 | 303.2 |
| Total Delay (hr) | 95.2 | 90.0 | 91.4 | 94.9 | 95.0 | 93.3 |
| Total Stops | 6859 | 6678 | 6907 | 7094 | 6957 | 6900 |
| Fuel Used (gal) | 269.8 | 270.0 | 270.1 | 275.6 | 268.9 | 270.9 |

Interval \#0 Information Seeding

| Start Time | $7: 10$ |
| :--- | ---: |
| End Time | $7: 20$ |
| Total Time (min) | 10 |
| Volumes adjusted by Growth Factors. |  |
| No data recorded this interval. |  |

Interval \#1 Information Recording1

| Start Time | $7: 20$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $7: 35$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number |  | 101 | 102 | 103 | 104 | 105 |
| Vehs Entered | 1423 | 1387 | 1347 | 1401 | 1293 | 1367 |
| Vehs Exited | 1403 | 1407 | 1338 | 1400 | 1244 | 1361 |
| Starting Vehs | 311 | 318 | 300 | 268 | 266 | 293 |
| Ending Vehs | 331 | 298 | 309 | 269 | 315 | 303 |
| Travel Distance (mi) | 1979 | 1936 | 1850 | 1877 | 1779 | 1884 |
| Travel Time (hr) | 82.3 | 77.7 | 72.0 | 75.6 | 69.9 | 75.5 |
| Total Delay (hr) | 27.7 | 24.2 | 20.9 | 23.4 | 20.8 | 23.4 |
| Total Stops | 1875 | 1711 | 1592 | 1792 | 1640 | 1725 |
| Fuel Used (gal) | 71.5 | 68.8 | 65.4 | 67.8 | 63.2 | 67.3 |

SimTraffic Simulation Summary
Year 2025 Total Traffic Culdesac AM Peak Hour Conditions
Interval \#2 Information Recording2

| Start Time | $7: 35$ |
| :--- | ---: |
| End Time | $7: 50$ |
| Total Time (min) | 15 |
| Volumes adjusted by PHF, Growth Factors. |  |


| Run Number | 101 | 102 | 103 | 104 | 105 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1509 | 1551 | 1564 | 1584 | 1616 | 1565 |
| Vehs Exited | 1550 | 1526 | 1543 | 1503 | 1541 | 1535 |
| Starting Vehs | 331 | 298 | 309 | 269 | 315 | 303 |
| Ending Vehs | 290 | 323 | 330 | 350 | 390 | 334 |
| Travel Distance (mi) | 1985 | 2011 | 2046 | 1990 | 2036 | 2014 |
| Travel Time (hr) | 80.4 | 80.4 | 84.3 | 81.1 | 85.9 | 82.4 |
| Total Delay (hr) | 25.0 | 24.2 | 27.4 | 25.5 | 29.4 | 26.3 |
| Total Stops | 1909 | 1817 | 1970 | 1903 | 2099 | 1940 |
| Fuel Used (gal) | 71.6 | 71.8 | 73.4 | 71.6 | 73.9 | 72.4 |

Interval \#3 Information Recording3

| Start Time | $7: 50$ |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $8: 05$ |  |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |  |
| Run Number | 101 | 102 | 103 | 104 | 105 | Avg |  |
| Vehs Entered | 1278 | 1313 | 1306 | 1336 | 1277 | 1300 |  |
| Vehs Exited | 1324 | 1297 | 1348 | 1372 | 1379 | 1346 |  |
| Starting Vehs | 290 | 323 | 330 | 350 | 390 | 334 |  |
| Ending Vehs | 244 | 339 | 288 | 314 | 288 | 289 |  |
| Travel Distance (mi) | 1755 | 1860 | 1855 | 1924 | 1787 | 1836 |  |
| Travel Time (hr) | 69.3 | 72.0 | 71.9 | 76.4 | 71.2 | 72.2 |  |
| Total Delay (hr) | 20.5 | 20.7 | 20.7 | 23.5 | 21.5 | 21.4 |  |
| Total Stops | 1516 | 1536 | 1646 | 1752 | 1575 | 1605 |  |
| Fuel Used (gal) | 62.7 | 64.9 | 65.6 | 69.1 | 64.1 | 65.3 |  |

## Interval \#4 Information Recording4

| Start Time | 8:05 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| End Time | 8:20 |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number | 101 | 102 | 103 | 104 | 105 | Avg |
| Vehs Entered | 1313 | 1272 | 1362 | 1354 | 1301 | 1314 |
| Vehs Exited | 1281 | 1363 | 1354 | 1394 | 1294 | 1335 |
| Starting Vehs | 244 | 339 | 288 | 314 | 288 | 289 |
| Ending Vehs | 276 | 248 | 296 | 274 | 295 | 270 |
| Travel Distance (mi) | 1820 | 1800 | 1845 | 1884 | 1881 | 1846 |
| Travel Time (hr) | 72.0 | 70.6 | 73.3 | 74.7 | 75.1 | 73.2 |
| Total Delay (hr) | 21.9 | 20.9 | 22.4 | 22.5 | 23.3 | 22.2 |
| Total Stops | 1559 | 1614 | 1699 | 1647 | 1643 | 1632 |
| Fuel Used (gal) | 64.1 | 64.5 | 65.7 | 67.2 | 67.7 | 65.8 |

Queuing and Blocking Report
Year 2025 Total Traffic Culdesac AM Peak Hour Conditions
Intersection: 1: Langer Farms Pkwy \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | T | TR | L | T | R | L | TR |
| Maximum Queue (ft) | 29 | 254 | 267 | 187 | 196 | 209 | 167 | 218 | 126 | 74 | 114 |
| Average Queue (ft) | 7 | 138 | 135 | 47 | 55 | 75 | 73 | 80 | 54 | 26 | 53 |
| 95th Queue (ft) | 27 | 228 | 234 | 108 | 143 | 160 | 138 | 160 | 97 | 62 | 101 |
| Link Distance (ft) |  | 1478 | 1478 |  | 5034 | 5034 |  | 1246 |  |  | 602 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  | 200 |  |  | 375 |  | 300 | 140 |  |
| Storage Bay Dist (ft) | 175 |  |  |  | 0 |  |  | 0 |  |  | 0 |
| Storage Blk Time (\%) |  | 2 |  |  | 0 |  |  | 0 |  | 0 |  |

Intersection: 2: Oregon St \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | EB | WB | WB | WB | NB | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | T | R | L | T | TR | LT | R | L |
| Maximum Queue (ft) | 25 | 197 | 214 | 106 | 152 | 261 | 276 | 172 | 238 | 48 |
| Average Queue (ft) | 5 | 79 | 78 | 27 | 59 | 88 | 106 | 64 | 103 | 4 |
| 95th Queue (ft) | 19 | 161 | 168 | 74 | 120 | 211 | 228 | 132 | 184 | 26 |
| Link Distance (ft) |  | 5034 | 5034 |  |  | 600 | 600 |  | 3270 |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 250 |  |  | 200 | 350 |  |  | 200 |  | 75 |
| Storage Blk Time (\%) |  |  | 0 | 0 |  |  |  | 0 | 1 | 0 |
| Queuing Penalty (veh) |  |  | 0 | 0 |  |  |  | 1 | 1 | 0 |

Intersection: 3: Tualatin-Sherwood Rd \& Wildrose PI

| Movement | EB | WB | SB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | R |
| Maximum Queue (ft) | 33 | 4 | 66 | 48 |
| Average Queue (ft) | 5 | 0 | 10 | 5 |
| 95th Queue (ft) | 23 | 3 | 42 | 26 |
| Link Distance (ft) |  | 1102 |  | 685 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) | 150 |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 4: Cipole Rd \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | T | TR | L | TR | L | TR |
| Maximum Queue (ft) | 87 | 214 | 243 | 126 | 211 | 233 | 68 | 66 | 144 | 75 |
| Average Queue (ft) | 42 | 59 | 89 | 49 | 51 | 66 | 19 | 15 | 57 | 29 |
| 95th Queue (ft) | 75 | 140 | 186 | 101 | 142 | 159 | 53 | 46 | 126 | 65 |
| Link Distance (ft) |  | 1102 | 1102 |  | 813 | 813 |  | 401 |  | 800 | | Upstream Blk Time (\%) |  |  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Queuing Penalty (veh) |  |  |  | 250 |  |  |
| Storage Bay Dist (ft) | 360 |  |  |  | 0 |  |
| Storage Blk Time (\%) |  |  |  |  | 0 |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | EB | EB | WB | WB | WB | WB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | L | T | T | $R$ | L | L | T | T | $R$ | L | L |
| Maximum Queue (ft) | 93 | 242 | 407 | 445 | 268 | 71 | 97 | 299 | 304 | 222 | 163 | 175 |
| Average Queue (ft) | 29 | 72 | 201 | 224 | 26 | 9 | 33 | 140 | 143 | 75 | 75 | 91 |
| 95th Queue (ft) | 71 | 181 | 364 | 402 | 125 | 41 | 83 | 250 | 250 | 169 | 141 | 154 |
| Link Distance (ft) |  |  | 813 | 813 |  |  |  | 1225 | 1225 |  |  |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  | 375 | 375 | 375 |  |  | 375 | 300 | 300 |
| Storage Bay Dist (ft) | 250 | 250 |  |  | 375 |  |  |  |  |  |  |  |
| Storage Blk Time (\%) |  | 0 | 4 | 2 | 0 |  |  | 0 | 0 | 0 |  |  |
| Queuing Penalty (veh) |  | 0 | 3 | 1 | 0 |  |  | 0 | 0 | 0 |  |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | TR | L | T | TR |
| Maximum Queue (ft) | 225 | 280 | 304 | 248 | 247 |
| Average Queue (ft) | 114 | 138 | 160 | 99 | 97 |
| 95th Queue (ft) | 194 | 235 | 280 | 209 | 202 |
| Link Distance (ft) | 903 | 903 |  | 1875 | 1875 |
| Upstream Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |
| Storage Bay Dist (ft) |  |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |

Queuing and Blocking Report
Year 2025 Total Traffic Culdesac AM Peak Hour Conditions
Intersection: 6: 120th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | TR | L | L | R |
| Maximum Queue (ft) | 8 | 8 | 64 | 152 | 88 |
| Average Queue (ft) | 0 | 0 | 10 | 41 | 20 |
| 95th Queue (ft) | 0 | 4 | 43 | 119 | 67 |
| Link Distance (ft) | 1225 | 1225 |  |  | 746 |
| Upstream Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  | 230 | 260 |  |
| Storage Bay Dist (ft) |  |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |  |

Intersection: 7: 115th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | EB | WB | WB | WB | WB | NB | NB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | T | T | R | L | L | T | TR | LT | R | L | TR |
| Maximum Queue (ft) | 24 | 275 | 311 | 155 | 203 | 232 | 184 | 204 | 154 | 154 | 22 | 42 |
| Average Queue (ft) | 2 | 85 | 117 | 37 | 78 | 104 | 31 | 43 | 45 | 46 | 2 | 3 |
| 95th Queue (ft) | 15 | 206 | 253 | 119 | 168 | 191 | 105 | 133 | 110 | 110 | 11 | 20 |
| Link Distance (ft) |  | 1250 | 1250 |  |  |  | 950 | 950 |  | 738 |  | 362 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 240 |  |  | 130 | 225 | 225 |  |  | 170 |  | 90 |  |
| Storage Blk Time (\%) |  | 0 | 5 | 0 | 0 | 1 | 0 |  | 1 | 1 |  |  |
| Queuing Penalty (veh) |  | 0 | 6 | 1 | 2 | 3 | 0 |  | 1 | 0 |  |  |

Intersection: 8: 112th Ave/Avery St \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | T | R | L | T | TR | L | TR | L | TR |
| Maximum Queue (ft) | 219 | 412 | 413 | 288 | 202 | 373 | 342 | 160 | 583 | 125 | 94 |
| Average Queue (ft) | 67 | 102 | 117 | 46 | 34 | 171 | 158 | 150 | 282 | 40 | 24 |
| 95th Queue (ft) | 145 | 252 | 261 | 146 | 118 | 316 | 300 | 185 | 533 | 101 | 66 |
| Link Distance (ft) |  | 950 | 950 |  |  | 1290 | 1290 |  | 713 |  | 1359 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  | 0 |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  | 0 |  |  |
| Storage Bay Dist (ft) | 240 |  |  | 300 | 240 |  |  | 135 |  | 170 |  |
| Storage Blk Time (\%) | 1 | 1 | 1 | 0 | 0 | 4 |  | 45 | 2 | 0 |  |
| Queuing Penalty (veh) | 2 | 1 | 2 | 0 | 0 | 1 |  | 30 | 5 | 0 |  |

Queuing and Blocking Report
Year 2025 Total Traffic Culdesac AM Peak Hour Conditions
Intersection: 9: Tonquin Rd \& Oregon St

| Movement | EB | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | R | L | L | R |
| Maximum Queue (ft) | 9 | 162 | 106 | 138 | 101 |
| Average Queue (ft) | 0 | 19 | 39 | 62 | 46 |
| 95th Queue (ft) | 5 | 100 | 84 | 114 | 84 |
| Link Distance (ft) | 372 |  |  |  | 552 |
| Upstream Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |
| Storage Bay Dist (ft) |  | 240 | 190 | 210 |  |
| Storage Blk Time (\%) |  |  |  |  |  |

Intersection: 11: 124th Ave \& Blake Road

| Movement | EB | EB | WB | WB | NB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | TR | L | TR | TR | L |
| Maximum Queue (ft) | 53 | 72 | 40 | 49 | 3 | 44 |
| Average Queue (ft) | 16 | 21 | 3 | 8 | 0 | 6 |
| 95th Queue (ft) | 43 | 56 | 18 | 34 | 2 | 29 |
| Link Distance (ft) |  | 802 |  | 1330 | 1017 |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |
| Queuing Penalty (veh) 150 |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |
| Zone Summary |  |  |  |  |  |  |

Zone wide Queuing Penalty: 63

## Summary of All Intervals

| Run Number | 106 | 107 | 108 | 109 | 110 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Start Time | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ |
| End Time | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ |
| Total Time (min) | 70 | 70 | 70 | 70 | 70 | 70 |
| Time Recorded (min) | 60 | 60 | 60 | 60 | 60 | 60 |
| \# of Intervals | 5 | 5 | 5 | 5 | 5 | 5 |
| \# of Recorded Intervals | 4 | 4 | 4 | 4 | 4 | 4 |
| Vehs Entered | 6244 | 6199 | 6113 | 6260 | 6114 | 6182 |
| Vehs Exited | 6207 | 6199 | 6126 | 6229 | 6080 | 6168 |
| Starting Vehs | 346 | 373 | 349 | 353 | 344 | 348 |
| Ending Vehs | 383 | 373 | 336 | 384 | 378 | 368 |
| Travel Distance (mi) | 8840 | 8743 | 8625 | 8816 | 8696 | 8744 |
| Travel Time (hr) | 393.6 | 380.4 | 369.0 | 378.6 | 367.9 | 377.9 |
| Total Delay (hr) | 147.4 | 136.4 | 127.9 | 132.6 | 125.2 | 133.9 |
| Total Stops | 8714 | 8567 | 8682 | 8953 | 9093 | 8801 |
| Fuel Used (gal) | 321.6 | 317.3 | 310.2 | 318.9 | 313.7 | 316.4 |

Interval \#0 Information Seeding

| Start Time $r: 4: 45$ |  |
| :--- | ---: |
| End Time $r: 55$ |  |
| Total Time (min) | 10 |
| Volumes adjusted by Growth Factors. |  |
| No data recorded this interval. |  |

Interval \#1 Information Recording1

| Start Time | $4: 55$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $5: 10$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number | 106 | 107 | 108 | 109 | 110 | Avg |
| Vehs Entered | 1520 | 1474 | 1484 | 1532 | 1468 | 1488 |
| Vehs Exited | 1530 | 1536 | 1482 | 1513 | 1474 | 1505 |
| Starting Vehs | 346 | 373 | 349 | 353 | 344 | 348 |
| Ending Vehs | 336 | 311 | 351 | 372 | 338 | 337 |
| Travel Distance (mi) | 2154 | 2116 | 2130 | 2158 | 2141 | 2140 |
| Travel Time (hr) | 90.3 | 87.1 | 87.1 | 87.8 | 86.1 | 87.7 |
| Total Delay (hr) | 30.5 | 28.3 | 27.8 | 27.7 | 26.3 | 28.1 |
| Total Stops | 2269 | 2135 | 2233 | 2216 | 2213 | 2209 |
| Fuel Used (gal) | 77.1 | 76.0 | 76.1 | 77.2 | 76.3 | 76.6 |

SimTraffic Simulation Summary
Year 2025 Total Traffic Culdesac PM Peak Hour Conditions
Interval \#2 Information Recording2

| Start Time | $5: 10$ |
| :--- | ---: |
| End Time | $5: 25$ |
| Total Time $(\min )$ | 15 |
| Volumes adjusted by PHF, Growth Factors. |  |


| Run Number | 106 | 107 | 108 | 109 | 110 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1662 | 1653 | 1683 | 1671 | 1633 | 1657 |
| Vehs Exited | 1600 | 1552 | 1607 | 1644 | 1574 | 1592 |
| Starting Vehs | 336 | 311 | 351 | 372 | 338 | 337 |
| Ending Vehs | 398 | 412 | 427 | 399 | 397 | 400 |
| Travel Distance (mi) | 2318 | 2213 | 2302 | 2318 | 2257 | 2282 |
| Travel Time (hr) | 100.1 | 97.6 | 99.1 | 99.6 | 94.3 | 98.1 |
| Total Delay (hr) | 35.5 | 35.5 | 35.0 | 35.1 | 31.6 | 34.5 |
| Total Stops | 2437 | 2186 | 2301 | 2488 | 2288 | 2338 |
| Fuel Used (gal) | 84.0 | 80.6 | 82.7 | 84.3 | 81.1 | 82.6 |

Interval \#3 Information Recording3

| Start Time | $5: 25$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $5: 40$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number |  | 106 | 107 | 108 | 109 | 110 |
| Vehs Entered | 1536 | 1547 | 1489 | 1519 | 1486 | 1515 |
| Vehs Exited | 1548 | 1557 | 1567 | 1515 | 1515 | 1539 |
| Starting Vehs | 398 | 412 | 427 | 399 | 397 | 400 |
| Ending Vehs | 386 | 402 | 349 | 403 | 368 | 374 |
| Travel Distance (mi) | 2210 | 2244 | 2155 | 2199 | 2129 | 2187 |
| Travel Time (hr) | 101.3 | 101.3 | 99.4 | 96.4 | 95.4 | 98.8 |
| Total Delay (hr) | 39.9 | 38.9 | 39.0 | 35.2 | 35.8 | 37.8 |
| Total Stops | 2000 | 2065 | 2108 | 2181 | 2362 | 2143 |
| Fuel Used (gal) | 80.6 | 81.9 | 78.6 | 79.7 | 78.1 | 79.8 |

## Interval \#4 Information Recording4

| Start Time | 5:40 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| End Time | 5:55 |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number | 106 | 107 | 108 | 109 | 110 | Avg |
| Vehs Entered | 1526 | 1525 | 1457 | 1538 | 1527 | 1514 |
| Vehs Exited | 1529 | 1554 | 1470 | 1557 | 1517 | 1524 |
| Starting Vehs | 386 | 402 | 349 | 403 | 368 | 374 |
| Ending Vehs | 383 | 373 | 336 | 384 | 378 | 368 |
| Travel Distance (mi) | 2158 | 2170 | 2037 | 2142 | 2169 | 2135 |
| Travel Time (hr) | 101.9 | 94.4 | 83.4 | 94.8 | 92.0 | 93.3 |
| Total Delay (hr) | 41.5 | 33.7 | 26.1 | 34.7 | 31.5 | 33.5 |
| Total Stops | 2008 | 2181 | 2040 | 2068 | 2230 | 2105 |
| Fuel Used (gal) | 79.8 | 78.8 | 72.7 | 77.6 | 78.2 | 77.5 |

Queuing and Blocking Report
Year 2025 Total Traffic Culdesac PM Peak Hour Conditions
Intersection: 1: Langer Farms Pkwy \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | T | TR | L | T | R | L |
| Maximum Queue (ft) | 101 | 301 | 360 | 220 | 316 | 312 | 259 | 151 | 112 | 165 |
| TR |  |  |  |  |  |  |  |  |  |  |
| Average Queue (ft) | 11 | 188 | 206 | 114 | 142 | 157 | 135 | 81 | 38 | 55 |
| 95th Queue (ft) | 54 | 282 | 323 | 199 | 266 | 276 | 229 | 139 | 77 | 152 |
| Link Distance (ft) |  | 1478 | 1478 |  | 5042 | 5042 |  | 1246 |  | 513 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  | 602 |
| Queuing Penalty (veh) |  |  |  |  |  |  | 375 |  | 300 | 140 |
| Storage Bay Dist (ft) | 175 |  |  | 200 |  |  |  |  | 0 | 46 |
| Storage Blk Time (\%) |  | 8 |  | 2 | 2 |  |  |  | 0 | 15 |

Intersection: 2: Oregon St \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | T | T | R | L | T | TR | LT | R | L | TR |
| Maximum Queue (ft) | 32 | 269 | 270 | 214 | 316 | 398 | 404 | 156 | 140 | 39 | 46 |
| Average Queue (ft) | 4 | 84 | 92 | 39 | 142 | 119 | 135 | 74 | 52 | 8 | 15 |
| 95th Queue (ft) | 19 | 186 | 192 | 110 | 251 | 281 | 295 | 133 | 105 | 30 | 38 |
| Link Distance (ft) |  | 5042 | 5042 |  |  | 594 | 594 | 3272 | 3272 |  | 356 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 250 |  |  | 200 | 350 |  |  |  |  | 75 |  |
| Storage Blk Time (\%) |  | 0 | 1 | 0 | 0 | 1 |  |  |  | 0 |  |
| Queuing Penalty (veh) |  | 0 | 1 | 0 | 0 | 2 |  |  |  | 0 |  |

Intersection: 3: Tualatin-Sherwood Rd \& Wildrose PI

| Movement | EB | SB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | L | L | R |
| Maximum Queue (ft) | 31 | 53 | 50 |
| Average Queue (ft) | 2 | 12 | 13 |
| 95th Queue (ft) | 17 | 39 | 38 |
| Link Distance (ft) |  |  | 685 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) | 150 | 200 |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 4: Cipole Rd \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | T | TR | L | TR | L | TR |
| Maximum Queue (ft) | 60 | 201 | 248 | 74 | 285 | 267 | 108 | 105 | 111 | 119 |
| Average Queue (ft) | 23 | 66 | 90 | 14 | 95 | 100 | 50 | 39 | 45 | 47 |
| 95th Queue (ft) | 52 | 150 | 182 | 45 | 198 | 195 | 92 | 79 | 89 | 89 |
| Link Distance (ft) |  | 1100 | 1100 |  | 813 | 813 |  | 422 |  | 800 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  | 250 |  |  | 200 |  | 300 |  |
| Storage Bay Dist (ft) | 360 |  |  |  | 0 |  |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  | 0 |  |  |  |  |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | EB | EB | WB | WB | WB | WB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | L | T | T | R | L | L | T | T | R | L | L |
| Maximum Queue (ft) | 87 | 210 | 335 | 354 | 165 | 53 | 79 | 294 | 280 | 132 | 109 | 105 |
| Average Queue (ft) | 23 | 51 | 163 | 188 | 45 | 14 | 34 | 174 | 171 | 37 | 49 | 53 |
| 95th Queue (ft) | 62 | 126 | 290 | 308 | 123 | 42 | 67 | 267 | 266 | 94 | 91 | 95 |
| Link Distance (ft) |  |  | 813 | 813 |  |  |  | 1225 | 1225 |  |  |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  | 375 | 375 | 375 |  |  | 375 | 300 | 300 |
| Storage Bay Dist (ft) | 250 | 250 |  |  | 0 | 0 |  |  |  |  |  |  |
| Storage Blk Time (\%) |  |  | 2 | 0 | 0 |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  | 1 | 0 | 0 |  |  |  |  |  |  |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | TR | L | T | TR |
| Maximum Queue (ft) | 96 | 99 | 259 | 185 | 280 |
| Average Queue (ft) | 43 | 41 | 117 | 97 | 131 |
| 95th Queue (ft) | 80 | 87 | 202 | 169 | 228 |
| Link Distance (ft) | 873 | 873 |  | 1875 | 1875 |
| Upstream Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |
| Storage Bay Dist (ft) |  |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |

Intersection: 6: 120th Ave \& Tualatin-Sherwood Rd

| Movement | WB | NB | NB |
| :--- | ---: | ---: | ---: |
| Directions Served | L | L | R |
| Maximum Queue (ft) | 22 | 67 | 33 |
| Average Queue (ft) | 2 | 24 | 14 |
| 95th Queue (ft) | 12 | 56 | 38 |
| Link Distance (ft) |  |  | 746 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) | 230 | 260 |  |
| Storage Blk Time (\%) |  |  |  |

Intersection: 7: 115th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | EB | WB | WB | WB | WB | NB | NB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | T | T | R | L | L | T | TR | LT | R | L | TR |
| Maximum Queue (ft) | 22 | 217 | 287 | 104 | 53 | 78 | 118 | 155 | 173 | 216 | 52 | 27 |
| Average Queue (ft) | 4 | 58 | 82 | 10 | 7 | 31 | 40 | 55 | 69 | 81 | 11 | 5 |
| 95th Queue (ft) | 16 | 149 | 199 | 51 | 31 | 69 | 94 | 115 | 141 | 159 | 39 | 16 |
| Link Distance (ft) |  | 1250 | 1250 |  |  |  | 950 | 950 |  | 738 |  | 362 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 240 |  |  | 130 | 225 | 225 |  |  | 170 |  | 90 |  |
| Storage Blk Time (\%) |  | 0 | 3 | 0 |  |  |  |  | 1 | 1 | 0 |  |
| Queuing Penalty (veh) |  | 0 | 2 | 0 |  |  |  |  | 1 | 1 | 0 |  |

Intersection: 8: 112th Ave/Avery St \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | T | T | R | L | T | TR | L | TR | L | TR |
| Maximum Queue (ft) | 86 | 195 | 206 | 119 | 30 | 260 | 247 | 159 | 314 | 182 | 206 |
| Average Queue (ft) | 28 | 75 | 91 | 46 | 6 | 134 | 117 | 131 | 89 | 76 | 68 |
| 95th Queue (ft) | 67 | 168 | 182 | 92 | 23 | 231 | 221 | 180 | 267 | 145 | 147 |
| Link Distance (ft) |  | 950 | 950 |  |  | 1290 | 1290 |  | 713 |  | 1359 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 240 |  |  | 300 | 240 |  |  | 135 |  | 170 |  |
| Storage BIk Time (\%) |  | 0 |  |  |  | 0 |  | 24 |  | 1 | 1 |
| Queuing Penalty (veh) |  | 0 |  |  |  | 0 |  | 9 |  | 1 | 1 |

Queuing and Blocking Report
Year 2025 Total Traffic Culdesac PM Peak Hour Conditions
Intersection: 9: Tonquin Rd \& Oregon St

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | R | L | L | R |
| Maximum Queue (ft) | 58 | 71 | 235 | 812 |
| Average Queue (ft) | 2 | 23 | 211 | 521 |
| 95th Queue (ft) | 31 | 58 | 293 | 1036 |
| Link Distance (ft) |  |  |  | 808 |
| Upstream Blk Time (\%) |  |  |  | 34 |
| Queuing Penalty (veh) |  |  |  | 0 |
| Storage Bay Dist (ft) | 240 | 190 | 210 |  |
| Storage Blk Time (\%) |  |  | 74 | 0 |
| Queuing Penalty (veh) |  |  | 69 | 0 |

Intersection: 11: 124th Ave \& Blake Road

| Movement | EB | EB | WB | WB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | TR |
| Maximum Queue (ft) | 42 | 52 | 39 | 68 | 5 | 22 |
| Average Queue (ft) | 8 | 13 | 14 | 23 | 0 | 1 |
| 95th Queue (ft) | 31 | 39 | 35 | 52 | 4 | 10 |
| Link Distance (ft) |  | 781 |  | 930 |  | 873 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  | 150 |  | 150 |  |
| Storage Bay Dist (ft) | 150 |  |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |
| Zone Summary |  |  |  |  |  |  |

Zone wide Queuing Penalty: 117

## Summary of All Intervals

| Run Number | 116 | 117 | 118 | 119 | 120 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Start Time | $7: 10$ | $7: 10$ | $7: 10$ | $7: 10$ | $7: 10$ | $7: 10$ |
| End Time | $8: 20$ | $8: 20$ | $8: 20$ | $8: 20$ | $8: 20$ | $8: 20$ |
| Total Time (min) | 70 | 70 | 70 | 70 | 70 | 70 |
| Time Recorded (min) | 60 | 60 | 60 | 60 | 60 | 60 |
| \# of Intervals | 5 | 5 | 5 | 5 | 5 | 5 |
| \# of Recorded Intervals | 4 | 4 | 4 | 4 | 4 | 4 |
| Vehs Entered | 5332 | 5667 | 5615 | 5497 | 5509 | 5528 |
| Vehs Exited | 5331 | 5671 | 5635 | 5477 | 5513 | 5525 |
| Starting Vehs | 261 | 297 | 282 | 279 | 274 | 278 |
| Ending Vehs | 262 | 293 | 262 | 299 | 270 | 276 |
| Travel Distance (mi) | 7192 | 7571 | 7519 | 7438 | 7473 | 7438 |
| Travel Time (hr) | 284.7 | 304.4 | 299.1 | 303.1 | 300.4 | 298.3 |
| Total Delay (hr) | 85.3 | 95.0 | 90.4 | 97.1 | 93.0 | 92.1 |
| Total Stops | 6392 | 7013 | 6720 | 6801 | 6793 | 6744 |
| Fuel Used (gal) | 254.9 | 271.6 | 267.8 | 267.0 | 267.7 | 265.8 |

Interval \#0 Information Seeding

| Start Time $r: 10$ |  |
| :--- | ---: |
| End Time | $7: 20$ |
| Total Time (min) | 10 |
| Volumes adjusted by Growth Factors. |  |
|  |  |
| No data recorded this interval. |  |

Interval \#1 Information Recording1

| Start Time | $7: 20$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $7: 35$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  | 116 | 117 | 118 | 119 | 120 |
| Run Number | 1332 | 1348 | 1360 | 1313 | 1337 | 1337 |
| Vehs Entered | 1292 | 1364 | 1329 | 1300 | 1318 | 1318 |
| Vehs Exited | 261 | 297 | 282 | 279 | 274 | 278 |
| Starting Vehs | 301 | 281 | 313 | 292 | 293 | 293 |
| Ending Vehs | 1799 | 1796 | 1860 | 1807 | 1839 | 1820 |
| Travel Distance (mi) | 70.3 | 72.2 | 73.6 | 71.3 | 73.4 | 72.2 |
| Travel Time (hr) | 20.3 | 22.3 | 22.3 | 21.4 | 22.2 | 21.7 |
| Total Delay (hr) | 1593 | 1667 | 1659 | 1633 | 1640 | 1634 |
| Total Stops | 63.4 | 63.8 | 66.4 | 64.6 | 66.1 | 64.9 |

SimTraffic Simulation Summary
Year 2025 Total Traffic Cipole Extension AM Peak Hour Conditions
Interval \#2 Information Recording2

| Start Time | $7: 35$ |
| :--- | ---: | :--- |
| End Time | $7: 50$ |
| Total Time (min) | 15 |
| Volumes adjusted by PHF, Growth Factors. |  |


| Run Number | 116 | 117 | 118 | 119 | 120 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1519 | 1604 | 1651 | 1525 | 1540 | 1567 |
| Vehs Exited | 1492 | 1554 | 1633 | 1496 | 1496 | 1536 |
| Starting Vehs | 301 | 281 | 313 | 292 | 293 | 293 |
| Ending Vehs | 328 | 331 | 331 | 321 | 337 | 327 |
| Travel Distance (mi) | 1943 | 2029 | 2130 | 1964 | 2044 | 2022 |
| Travel Time (hr) | 79.9 | 82.0 | 87.3 | 82.1 | 84.4 | 83.1 |
| Total Delay (hr) | 25.6 | 25.8 | 27.9 | 27.1 | 27.7 | 26.8 |
| Total Stops | 1822 | 1907 | 2017 | 1889 | 1925 | 1909 |
| Fuel Used (gal) | 69.5 | 73.5 | 76.4 | 71.1 | 73.1 | 72.7 |

Interval \#3 Information Recording3

| Start Time | $7: 50$ |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $8: 05$ |  |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |  |
| Run Number |  | 116 | 117 | 118 | 119 | 120 | Avg |
| Vehs Entered | 1256 | 1361 | 1301 | 1322 | 1327 | 1311 |  |
| Vehs Exited | 1330 | 1413 | 1379 | 1335 | 1362 | 1362 |  |
| Starting Vehs | 328 | 331 | 331 | 321 | 337 | 327 |  |
| Ending Vehs | 254 | 279 | 253 | 308 | 302 | 275 |  |
| Travel Distance (mi) | 1758 | 1929 | 1790 | 1824 | 1848 | 1830 |  |
| Travel Time (hr) | 68.7 | 78.6 | 71.3 | 76.0 | 73.1 | 73.5 |  |
| Total Delay (hr) | 20.0 | 25.2 | 21.5 | 25.6 | 22.0 | 22.9 |  |
| Total Stops | 1532 | 1837 | 1570 | 1704 | 1628 | 1652 |  |
| Fuel Used (gal) | 61.8 | 69.8 | 63.8 | 65.7 | 66.3 | 65.5 |  |

## Interval \#4 Information Recording4

| Start Time | $8: 05$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $8: 20$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number | 116 | 117 | 118 | 119 | 120 | Avg |
| Vehs Entered | 1225 | 1354 | 1303 | 1337 | 1305 | 1302 |
| Vehs Exited | 1217 | 1340 | 1294 | 1346 | 1337 | 1304 |
| Starting Vehs | 254 | 279 | 253 | 308 | 302 | 275 |
| Ending Vehs | 262 | 293 | 262 | 299 | 270 | 276 |
| Travel Distance (mi) | 1692 | 1817 | 1739 | 1842 | 1743 | 1767 |
| Travel Time (hr) | 65.8 | 71.7 | 66.9 | 73.7 | 69.5 | 69.5 |
| Total Delay (hr) | 19.3 | 21.6 | 18.7 | 23.0 | 21.0 | 20.7 |
| Total Stops | 1445 | 1602 | 1474 | 1575 | 1600 | 1541 |
| Fuel Used (gal) | 60.1 | 64.6 | 61.1 | 65.6 | 62.1 | 62.7 |

Intersection: 1: Langer Farms Pkwy \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | T | TR | L | T | R | L | TR |
| Maximum Queue (ft) | 33 | 254 | 271 | 145 | 230 | 244 | 159 | 183 | 99 | 82 | 114 |
| Average Queue (ft) | 7 | 142 | 143 | 41 | 66 | 88 | 67 | 84 | 45 | 25 | 50 |
| 95th Queue (ft) | 28 | 229 | 239 | 95 | 168 | 189 | 124 | 155 | 80 | 66 | 95 |
| Link Distance (ft) |  | 1478 | 1478 |  | 5034 | 5034 |  | 1246 |  |  | 602 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  | 200 |  |  | 375 |  | 300 | 140 |  |
| Storage Bay Dist (ft) | 175 |  |  |  | 0 |  |  |  |  |  | 0 |
| Storage Blk Time (\%) |  | 3 |  |  | 0 |  |  |  |  |  | 0 |

## Intersection: 2: Oregon St \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | EB | WB | WB | WB | NB | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| irections Served | L | T | T | R | L | T | TR | LT | R | L |
| Maximum Queue (ft) | 29 | 168 | 163 | 83 | 134 | 298 | 344 | 157 | 209 | 27 |
| Average Queue (ft) | 6 | 65 | 68 | 27 | 52 | 77 | 99 | 65 | 93 | 2 |
| 95th Queue (ft) | 22 | 133 | 138 | 63 | 107 | 208 | 227 | 131 | 163 | 14 |
| Link Distance (ft) |  | 5034 | 5034 |  |  | 600 | 600 |  | 3270 |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 250 |  |  | 200 | 350 |  |  | 200 |  | 75 |
| Storage Blk Time (\%) |  |  | 0 |  |  | 0 |  | 0 | 0 |  |
| Queuing Penalty (veh) |  |  | 0 |  |  | 0 |  | 1 | 0 |  |

Intersection: 3: Tualatin-Sherwood Rd \& Wildrose PI

| Movement | EB | EB | SB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | L | T | L | R |
| Maximum Queue (ft) | 37 | 11 | 52 | 55 |
| Average Queue (ft) | 5 | 0 | 8 | 7 |
| 95th Queue (ft) | 24 | 8 | 37 | 33 |
| Link Distance (ft) |  | 600 |  | 685 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) | 150 |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

Intersection: 4: Cipole Rd \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | T | TR | L | TR | L | TR |
| Maximum Queue (ft) | 81 | 183 | 204 | 98 | 205 | 200 | 68 | 50 | 165 | 87 |
| Average Queue (ft) | 39 | 63 | 87 | 36 | 56 | 64 | 15 | 11 | 51 | 29 |
| 95th Queue (ft) | 71 | 141 | 174 | 77 | 157 | 156 | 45 | 37 | 120 | 66 |
| Link Distance (ft) |  | 1102 | 1102 |  | 813 | 813 |  | 401 |  | 800 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  | 250 |  |  | 200 |  | 300 |  |
| Storage Bay Dist (ft) | 360 |  |  |  | 0 |  |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  | 0 |  |  |  |  |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | EB | EB | WB | WB | WB | WB | WB | NB | NB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | L | T | T | R | L | L | T | T | R | L | L |
| Maximum Queue ( t ) | 124 | 273 | 363 | 396 | 69 | 72 | 111 | 335 | 339 | 230 | 148 | 165 |
| Average Queue (ft) | 37 | 69 | 166 | 194 | 14 | 9 | 32 | 144 | 141 | 69 | 67 | 83 |
| 95 th Queue (ft) | 88 | 166 | 306 | 337 | 51 | 41 | 84 | 272 | 274 | 162 | 126 | 140 |
| Link Distance (ft) |  |  | 813 | 813 |  |  |  | 1225 | 1225 |  |  |  |
| Upstream BIk Time (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 250 | 250 |  |  | 375 | 375 | 375 |  |  | 375 | 300 | 300 |
| Storage Blk Time (\%) |  |  | 3 | 0 |  |  |  | 0 | 0 | 0 |  |  |
| Queuing Penalty (veh) |  |  | 2 | 0 |  |  |  | 0 | 0 | 0 |  |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | TR | L | T | TR |
| Maximum Queue (ft) | 263 | 289 | 312 | 344 | 278 |
| Average Queue (ft) | 118 | 136 | 199 | 140 | 130 |
| 95th Queue (ft) | 206 | 246 | 334 | 355 | 295 |
| Link Distance (ft) | 903 | 903 |  | 1875 | 1875 |
| Upstream Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  | 300 |  |  |
| Storage Bay Dist (ft) | 0 |  | 9 | 0 |  |
| Storage Blk Time (\%) | 0 | 8 | 0 |  |  |

Queuing and Blocking Report
Year 2025 Total Traffic Cipole Extension AM Peak Hour Conditions
Intersection: 6: 120th Ave \& Tualatin-Sherwood Rd

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | TR | L | L | $R$ |
| Maximum Queue (ft) | 19 | 69 | 122 | 76 |
| Average Queue (ft) | 1 | 14 | 38 | 21 |
| 95th Queue (ft) | 10 | 52 | 98 | 64 |
| Link Distance (ft) | 1225 |  |  | 746 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) |  | 230 |  |  |
| Storage Blk Time (\%) |  |  |  |  |

Intersection: 7: 115th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | EB | WB | WB | WB | WB | NB | NB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | T | T | R | L | L | T | TR | LT | R | L | TR |
| Maximum Queue (ft) | 81 | 280 | 362 | 145 | 202 | 214 | 210 | 183 | 143 | 151 | 15 | 44 |
| Average Queue (ft) | 5 | 78 | 107 | 32 | 77 | 103 | 29 | 41 | 43 | 42 | 1 | 3 |
| 95th Queue (ft) | 44 | 189 | 246 | 107 | 162 | 178 | 111 | 116 | 105 | 103 | 9 | 20 |
| Link Distance (ft) |  | 1250 | 1250 |  |  |  | 950 | 950 |  | 738 |  | 362 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 240 |  |  | 130 | 225 | 225 |  |  | 170 |  | 90 |  |
| Storage Blk Time (\%) |  | 0 | 4 | 0 | 0 | 0 | 0 |  | 0 | 0 |  | 0 |
| Queuing Penalty (veh) |  | 0 | 5 | 0 | 1 | 2 | 0 |  | 0 | 0 |  | 0 |

Intersection: 8: 112th Ave/Avery St \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | T | R | L | T | TR | L | TR | L | TR |
| Maximum Queue (ft) | 180 | 313 | 354 | 293 | 174 | 354 | 331 | 160 | 575 | 130 | 88 |
| Average Queue (ft) | 72 | 113 | 128 | 63 | 35 | 178 | 155 | 148 | 258 | 40 | 25 |
| 95th Queue (ft) | 150 | 268 | 291 | 193 | 120 | 324 | 291 | 183 | 523 | 101 | 71 |
| Link Distance (ft) |  | 950 | 950 |  |  | 1290 | 1290 |  | 713 |  | 1359 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  | 0 |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  | 135 | 0 | 170 |  |
| Storage Bay Dist (ft) | 240 |  |  | 300 | 240 |  |  | 43 | 1 |  |  |
| Storage Blk Time (\%) | 0 | 2 | 0 | 0 |  | 3 |  | 29 | 2 |  |  |
| Queuing Penalty (veh) | 0 | 1 | 1 | 0 |  | 1 |  |  |  |  |  |

Intersection: 9: Tonquin Rd \& Oregon St

| Movement | EB | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | R | L | L | $R$ |
| Maximum Queue (ft) | 13 | 132 | 100 | 145 | 113 |
| Average Queue (ft) | 1 | 12 | 36 | 58 | 48 |
| 95th Queue (ft) | 7 | 74 | 80 | 111 | 87 |
| Link Distance (ft) | 372 |  |  |  | 552 |
| Upstream Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |
| Storage Bay Dist (ft) |  | 240 | 190 | 210 |  |
| Storage Blk Time (\%) |  |  |  | 0 |  |
| Queuing Penalty (veh) |  |  |  | 0 |  |

Intersection: 11: 124th Ave \& Blake Road

| Movement | EB | EB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | TR | L | TR |
| Maximum Queue (ft) | 52 | 71 | 29 | 52 | 26 | 24 | 45 | 427 |
| Average Queue (ft) | 15 | 25 | 2 | 6 | 3 | 0 | 5 | 15 |
| 95th Queue (ft) | 43 | 57 | 15 | 28 | 16 | 2 | 27 | 224 |
| Link Distance (ft) |  | 802 |  | 1330 |  | 1017 |  | 903 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  | 0 |
| Queuing Penalty (veh) |  |  |  |  |  |  |  | 0 |

## Intersection: 12: Blake Road \& Cipole Road

| Movement | EB | SB |
| :--- | ---: | ---: |
| Directions Served | L | LR |
| Maximum Queue (ft) | 23 | 49 |
| Average Queue (ft) | 1 | 9 |
| 95th Queue (ft) | 9 | 32 |
| Link Distance (ft) |  | 546 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) | 150 |  |
| Storage Bk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Zone Summary |  |  |
| Zone wide Queuing Penalty: 56 |  |  |

## Summary of All Intervals

| Run Number | 126 | 127 | 128 | 129 | 130 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Start Time | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ |
| End Time | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ |
| Total Time (min) | 70 | 70 | 70 | 70 | 70 | 70 |
| Time Recorded (min) | 60 | 60 | 60 | 60 | 60 | 60 |
| \# of Intervals | 5 | 5 | 5 | 5 | 5 | 5 |
| \# of Recorded Intervals | 4 | 4 | 4 | 4 | 4 | 4 |
| Vehs Entered | 6257 | 6236 | 6173 | 6323 | 6422 | 6284 |
| Vehs Exited | 6229 | 6244 | 6125 | 6309 | 6368 | 6256 |
| Starting Vehs | 333 | 354 | 309 | 345 | 338 | 335 |
| Ending Vehs | 361 | 346 | 357 | 359 | 392 | 359 |
| Travel Distance (mi) | 8870 | 8819 | 8616 | 8940 | 8943 | 8838 |
| Travel Time (hr) | 376.3 | 386.6 | 368.9 | 372.0 | 395.0 | 379.8 |
| Total Delay (hr) | 129.0 | 140.2 | 128.5 | 123.0 | 145.1 | 133.2 |
| Total Stops | 9009 | 8967 | 8337 | 9056 | 9232 | 8927 |
| Fuel Used (gal) | 319.4 | 320.3 | 309.5 | 321.9 | 326.9 | 319.6 |

## Interval \#0 Information Seeding

| Start Time | $4: 45$ |
| :--- | ---: |
| End Time | $4: 55$ |
| Total Time (min) | 10 |
| Volumes adjusted by Growth Factors. |  |
| No data recorded this interval. |  |

## Interval \#1 Information Recording1

| Start Time | 4:55 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| End Time | 5:10 |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number | 126 | 127 | 128 | 129 | 130 | Avg |
| Vehs Entered | 1557 | 1559 | 1584 | 1534 | 1617 | 1565 |
| Vehs Exited | 1504 | 1575 | 1528 | 1532 | 1563 | 1540 |
| Starting Vehs | 333 | 354 | 309 | 345 | 338 | 335 |
| Ending Vehs | 386 | 338 | 365 | 347 | 392 | 362 |
| Travel Distance (mi) | 2189 | 2235 | 2163 | 2224 | 2279 | 2218 |
| Travel Time (hr) | 90.6 | 91.4 | 88.9 | 91.0 | 94.0 | 91.2 |
| Total Delay (hr) | 29.6 | 29.5 | 28.5 | 29.2 | 30.4 | 29.4 |
| Total Stops | 2251 | 2241 | 2140 | 2261 | 2310 | 2240 |
| Fuel Used (gal) | 78.4 | 79.7 | 77.1 | 80.0 | 82.2 | 79.5 |

SimTraffic Simulation Summary
Year 2025 Total Traffic Cipole Extension PM Peak Hour Conditions
Interval \#2 Information Recording2

| Start Time | $5: 10$ |
| :--- | ---: |
| End Time | $5: 25$ |
| Total Time $(\min )$ | 15 |
| Volumes adjusted by PHF, Growth Factors. |  |


| Run Number | 126 | 127 | 128 | 129 | 130 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1651 | 1652 | 1661 | 1668 | 1693 | 1665 |
| Vehs Exited | 1623 | 1577 | 1618 | 1608 | 1672 | 1621 |
| Starting Vehs | 386 | 338 | 365 | 347 | 392 | 362 |
| Ending Vehs | 414 | 413 | 408 | 407 | 413 | 401 |
| Travel Distance (mi) | 2276 | 2284 | 2303 | 2293 | 2289 | 2289 |
| Travel Time (hr) | 99.3 | 102.6 | 97.4 | 94.6 | 101.1 | 99.0 |
| Total Delay (hr) | 35.9 | 38.5 | 33.2 | 31.2 | 36.9 | 35.1 |
| Total Stops | 2361 | 2438 | 2287 | 2268 | 2423 | 2360 |
| Fuel Used (gal) | 82.6 | 83.2 | 82.9 | 82.2 | 83.0 | 82.8 |

Interval \#3 Information Recording3

| Start Time | $5: 25$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $5: 40$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number | 126 | 127 | 128 | 129 | 130 | Avg |
| Vehs Entered | 1563 | 1545 | 1454 | 1582 | 1557 | 1540 |
| Vehs Exited | 1595 | 1610 | 1512 | 1624 | 1580 | 1583 |
| Starting Vehs | 414 | 413 | 408 | 407 | 413 | 401 |
| Ending Vehs | 382 | 348 | 350 | 365 | 390 | 358 |
| Travel Distance (mi) | 2271 | 2247 | 2070 | 2250 | 2209 | 2209 |
| Travel Time (hr) | 98.8 | 99.3 | 92.2 | 96.4 | 101.0 | 97.6 |
| Total Delay (hr) | 35.6 | 36.3 | 34.5 | 33.3 | 39.3 | 35.8 |
| Total Stops | 2240 | 2152 | 1928 | 2329 | 2339 | 2194 |
| Fuel Used (gal) | 82.5 | 81.7 | 74.8 | 81.8 | 81.8 | 80.5 |

## Interval \#4 Information Recording4

| Start Time | $5: 40$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $5: 55$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number |  | 126 | 127 | 128 | 129 | 130 |
| Vehs Entered | 1486 | 1480 | 1474 | 1539 | 1555 | Avg |
| Vehs Exited | 1507 | 1482 | 1467 | 1545 | 1553 | 1511 |
| Starting Vehs | 382 | 348 | 350 | 365 | 390 | 358 |
| Ending Vehs | 361 | 346 | 357 | 359 | 392 | 359 |
| Travel Distance (mi) | 2135 | 2053 | 2080 | 2173 | 2167 | 2121 |
| Travel Time (hr) | 87.6 | 93.3 | 90.4 | 90.0 | 98.9 | 92.0 |
| Total Delay (hr) | 27.9 | 35.9 | 32.3 | 29.4 | 38.5 | 32.8 |
| Total Stops | 2157 | 2136 | 1982 | 2198 | 2160 | 2120 |
| Fuel Used (gal) | 75.9 | 75.6 | 74.7 | 77.9 | 79.9 | 76.8 |

Intersection: 1: Langer Farms Pkwy \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | T | TR | L | T | R | L |
| Maximum Queue (ft) | 131 | 314 | 337 | 224 | 331 | 323 | 268 | 156 | 94 | 165 |
| TR |  |  |  |  |  |  |  |  |  |  |
| Average Queue (ft) | 14 | 192 | 204 | 127 | 134 | 149 | 144 | 77 | 42 | 62 |
| 95th Queue (ft) | 76 | 291 | 316 | 214 | 266 | 273 | 240 | 127 | 74 | 167 |
| Link Distance (ft) |  | 1478 | 1478 |  | 5042 | 5042 |  | 1246 |  | 582 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  | 602 |
| Queuing Penalty (veh) |  |  |  |  |  |  | 375 |  | 300 | 140 |
| Storage Bay Dist (ft) | 175 |  |  | 200 |  |  |  |  | 0 | 60 |
| Storage Blk Time (\%) |  | 9 |  | 3 | 2 |  |  |  | 0 | 20 |

## Intersection: 2: Oregon St \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | T | T | R | L | T | TR | LT | R | L | TR |
| Maximum Queue (ft) | 81 | 265 | 285 | 195 | 284 | 306 | 304 | 192 | 138 | 39 | 47 |
| Average Queue (ft) | 9 | 85 | 93 | 43 | 145 | 112 | 123 | 84 | 48 | 11 | 15 |
| 95th Queue (ft) | 50 | 185 | 207 | 127 | 254 | 244 | 249 | 155 | 102 | 35 | 40 |
| Link Distance (ft) |  | 5042 | 5042 |  |  | 594 | 594 | 3272 | 3272 |  | 356 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 250 |  |  | 200 | 350 |  |  |  |  | 75 |  |
| Storage Blk Time (\%) |  | 0 | 1 | 0 | 0 | 0 |  |  |  | 0 |  |
| Queuing Penalty (veh) |  | 0 | 1 | 0 | 1 | 0 |  |  |  | 0 |  |

Intersection: 3: Tualatin-Sherwood Rd \& Wildrose PI

| Movement | EB | SB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | L | L | R |
| Maximum Queue (ft) | 28 | 52 | 70 |
| Average Queue (ft) | 2 | 11 | 16 |
| 95th Queue (ft) | 15 | 37 | 48 |
| Link Distance (ft) |  |  | 685 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) | 150 | 200 |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

Intersection: 4: Cipole Rd \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | T | TR | L | TR | L | TR |
| Maximum Queue (ft) | 71 | 166 | 214 | 42 | 265 | 266 | 105 | 83 | 111 | 104 |
| Average Queue (ft) | 23 | 60 | 84 | 12 | 92 | 99 | 36 | 30 | 45 | 46 |
| 95th Queue (ft) | 54 | 129 | 168 | 35 | 193 | 210 | 83 | 65 | 89 | 85 |
| Link Distance (ft) |  | 1100 | 1100 |  | 813 | 813 |  | 422 |  | 800 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  | 250 |  |  | 200 |  | 300 |  |
| Storage Bay Dist (ft) | 360 |  |  |  | 0 |  |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  | 0 |  |  |  |  |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | EB | EB | WB | WB | WB | WB | WB | NB | NB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | L | T | T | R | L | L | T | T | R | L | L |
| Maximum Queue ( t ) | 75 | 233 | 338 | 374 | 131 | 66 | 92 | 307 | 326 | 114 | 96 | 115 |
| Average Queue (ft) | 21 | 49 | 170 | 191 | 35 | 13 | 35 | 169 | 164 | 35 | 45 | 50 |
| 95th Queue (ft) | 55 | 136 | 294 | 314 | 87 | 42 | 68 | 266 | 270 | 85 | 82 | 93 |
| Link Distance (ft) |  |  | 813 | 813 |  |  |  | 1225 | 1225 |  |  |  |
| Upstream BIk Time (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 250 | 250 |  |  | 375 | 375 | 375 |  |  | 375 | 300 | 300 |
| Storage Blk Time (\%) |  |  | 2 | 0 |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  | 1 | 0 |  |  |  |  |  |  |  |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | TR | L | T | TR |
| Maximum Queue (ft) | 116 | 138 | 250 | 233 | 295 |
| Average Queue (ft) | 48 | 51 | 124 | 102 | 146 |
| 95th Queue (ft) | 95 | 103 | 213 | 190 | 261 |
| Link Distance (ft) | 873 | 873 |  | 1875 | 1875 |
| Upstream Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |
| Storage Bay Dist (ft) |  |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |

Intersection: 6: 120th Ave \& Tualatin-Sherwood Rd

| Movement | WB | NB | NB |
| :--- | ---: | ---: | ---: |
| Directions Served | L | L | R |
| Maximum Queue (ft) | 22 | 63 | 42 |
| Average Queue (ft) | 2 | 26 | 13 |
| 95th Queue (ft) | 13 | 58 | 37 |
| Link Distance (ft) |  |  | 746 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) | 230 | 260 |  |
| Storage Blk Time (\%) |  |  |  |

Intersection: 7: 115th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | EB | WB | WB | WB | WB | NB | NB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | T | T | R | L | L | T | TR | LT | R | L | TR |
| Maximum Queue (ft) | 33 | 260 | 294 | 130 | 53 | 104 | 160 | 176 | 166 | 193 | 47 | 20 |
| Average Queue (ft) | 5 | 81 | 107 | 16 | 7 | 37 | 45 | 59 | 68 | 80 | 11 | 5 |
| 95th Queue (ft) | 21 | 199 | 232 | 79 | 29 | 85 | 112 | 131 | 141 | 159 | 33 | 16 |
| Link Distance (ft) |  | 1250 | 1250 |  |  |  | 950 | 950 |  | 738 |  | 362 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 240 |  |  | 130 | 225 | 225 |  |  | 170 |  | 90 |  |
| Storage Blk Time (\%) |  | 0 | 3 | 0 |  |  |  |  | 0 | 1 |  |  |
| Queuing Penalty (veh) |  | 0 | 2 | 0 |  |  |  |  | 0 | 1 |  |  |

Intersection: 8: 112th Ave/Avery St \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | T | R | L | T | TR | L | TR | L | TR |
| Maximum Queue (ft) | 102 | 192 | 203 | 129 | 29 | 317 | 301 | 160 | 357 | 160 | 170 |
| Average Queue (ft) | 29 | 72 | 88 | 45 | 4 | 136 | 117 | 130 | 99 | 77 | 61 |
| 95th Queue (ft) | 76 | 160 | 166 | 90 | 20 | 247 | 227 | 184 | 287 | 140 | 127 |
| Link Distance (ft) |  | 950 | 950 |  |  | 1290 | 1290 |  | 713 |  | 1359 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  | 135 |  | 170 |  |
| Storage Bay Dist (ft) | 240 |  |  | 300 | 240 |  |  | 25 | 0 | 1 | 0 |
| Storage Blk Time (\%) |  | 0 |  |  |  | 1 |  | 9 | 1 | 1 | 0 |

## Intersection: 9: Tonquin Rd \& Oregon St

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | R | L | L | R |
| Maximum Queue (ft) | 56 | 86 | 235 | 787 |
| Average Queue (ft) | 2 | 29 | 217 | 460 |
| 95th Queue (ft) | 30 | 72 | 285 | 936 |
| Link Distance (ft) |  |  |  | 808 |
| Upstream Blk Time (\%) |  |  |  | 19 |
| Queuing Penalty (veh) |  |  |  | 0 |
| Storage Bay Dist (ft) | 240 | 190 | 210 |  |
| Storage Blk Time (\%) |  |  | 71 | 0 |
| Queuing Penalty (veh) |  |  | 66 | 0 |

## Intersection: 11: 124th Ave \& Blake Road

| Movement | EB | EB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | TR | L | TR |
| Maximum Queue (ft) | 42 | 61 | 42 | 59 | 20 | 10 | 10 | 16 |
| Average Queue (ft) | 18 | 21 | 13 | 21 | 1 | 1 | 1 | 1 |
| 95th Queue (ft) | 44 | 48 | 35 | 48 | 10 | 10 | 7 | 12 |
| Link Distance (ft) |  | 781 |  | 930 |  | 1716 |  | 873 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  | 150 |
| Storage Bay Dist (ft) | 150 |  | 150 |  | 150 |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |  |  |  |  |

## Intersection: 12: Blake Road \& Cipole Road

| Movement | EB | SB |
| :--- | ---: | ---: |
| Directions Served | L | LR |
| Maximum Queue (ft) | 6 | 66 |
| Average Queue (ft) | 0 | 30 |
| 95th Queue (ft) | 4 | 58 |
| Link Distance (ft) |  | 300 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) | 150 |  |
| Storage Bk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Zone Summary |  |  |
| Zone wide Queuing Penalty: 125 |  |  |

# Appendix M Year 2021 Total Traffic 

 Conditions - Alternative Access Scenario|  | $\Rightarrow$ |  |  | $\checkmark$ |  |  | 4 | $\dagger$ | 7 |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}$ | $\uparrow$ | \% | \% | $\uparrow$ |  | \% | $\uparrow$ | 7 | ${ }_{7}$ | $\hat{\beta}$ |  |
| Traffic Volume (vph) | 11 | 855 | 193 | 75 | 495 | 38 | 112 | 118 | 140 | 26 | 58 | 7 |
| Future Volume (vph) | 11 | 855 | 193 | 75 | 495 | 38 | 112 | 118 | 140 | 26 | 58 | 7 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 5.5 | 4.0 | 4.0 | 5.5 |  | 4.0 | 4.5 | 4.5 | 4.0 | 4.5 |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Frpb, ped/bikes | 1.00 | 1.00 | 0.98 | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Flpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Fit | 1.00 | 1.00 | 0.85 | 1.00 | 0.99 |  | 1.00 | 1.00 | 0.85 | 1.00 | 0.98 |  |
| Flt Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  |
| Satd. Flow (prot) | 1805 | 1795 | 1538 | 1703 | 1546 |  | 1751 | 1776 | 1568 | 1504 | 1760 |  |
| Flt Permitted | 0.36 | 1.00 | 1.00 | 0.09 | 1.00 |  | 0.46 | 1.00 | 1.00 | 0.67 | 1.00 |  |
| Satd. Flow (perm) | 676 | 1795 | 1538 | 156 | 1546 |  | 844 | 1776 | 1568 | 1063 | 1760 |  |
| Peak-hour factor, PHF | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 |
| Adj. Flow (vph) | 12 | 972 | 219 | 85 | 562 | 43 | 127 | 134 | 159 | 30 | 66 | 8 |
| RTOR Reduction (vph) | 0 | 0 | 42 | 0 | 2 | 0 | 0 | 0 | 135 | 0 | 4 | 0 |
| Lane Group Flow (vph) | 13 | 972 | 177 | 85 | 604 | 0 | 127 | 134 | 24 | 30 | 70 | 0 |
| Confl. Peds. (\#/hr) |  |  | 2 | 2 |  |  | 1 |  |  |  |  | 1 |
| Confl. Bikes (\#/hr) |  |  | 1 |  |  |  |  |  |  |  |  |  |
| Heavy Vehicles (\%) | 0\% | 5\% | 2\% | 6\% | 20\% | 29\% | 3\% | 7\% | 3\% | 20\% | 5\% | 14\% |
| Bus Blockages (\#/hr) | 0 | 2 | 2 | 0 | 2 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| Turn Type | pm+pt | NA | pm+ov | pm+pt | NA |  | pm+pt | NA | Perm | pm+pt | NA |  |
| Protected Phases | 5 | 2 | 3 | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  | 2 | 6 |  |  | 8 |  | 8 | 4 |  |  |
| Actuated Green, G (s) | 62.8 | 61.1 | 72.6 | 68.6 | 64.0 |  | 22.8 | 15.2 | 15.2 | 10.9 | 7.3 |  |
| Effective Green, g (s) | 62.8 | 61.1 | 72.6 | 68.6 | 64.0 |  | 22.8 | 15.2 | 15.2 | 10.9 | 7.3 |  |
| Actuated g/C Ratio | 0.61 | 0.60 | 0.71 | 0.67 | 0.62 |  | 0.22 | 0.15 | 0.15 | 0.11 | 0.07 |  |
| Clearance Time (s) | 4.0 | 5.5 | 4.0 | 4.0 | 5.5 |  | 4.0 | 4.5 | 4.5 | 4.0 | 4.5 |  |
| Vehicle Extension (s) | 1.5 | 3.5 | 1.5 | 1.5 | 3.5 |  | 1.5 | 8.0 | 8.0 | 1.5 | 2.0 |  |
| Lane Grp Cap (vph) | 432 | 1069 | 1089 | 173 | 965 |  | 289 | 263 | 232 | 128 | 125 |  |
| v/s Ratio Prot | 0.00 | c0.54 | 0.02 | c0.02 | 0.39 |  | c0.05 | c0.08 |  | 0.01 | 0.04 |  |
| $\mathrm{v} / \mathrm{s}$ Ratio Perm | 0.02 |  | 0.10 | 0.31 |  |  | 0.05 |  | 0.02 | 0.02 |  |  |
| v/c Ratio | 0.03 | 0.91 | 0.16 | 0.49 | 0.63 |  | 0.44 | 0.51 | 0.10 | 0.23 | 0.56 |  |
| Uniform Delay, d1 | 8.4 | 18.3 | 4.9 | 18.1 | 11.9 |  | 33.5 | 40.2 | 37.7 | 41.8 | 46.1 |  |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Incremental Delay, d2 | 0.0 | 11.4 | 0.0 | 0.8 | 1.3 |  | 0.4 | 6.5 | 0.8 | 0.3 | 3.4 |  |
| Delay (s) | 8.4 | 29.6 | 5.0 | 18.9 | 13.2 |  | 33.9 | 46.7 | 38.6 | 42.1 | 49.5 |  |
| Level of Service | A | C | A | B | B |  | C | D | D | D | D |  |
| Approach Delay (s) |  | 24.9 |  |  | 13.9 |  |  | 39.7 |  |  | 47.3 |  |
| Approach LOS |  | C |  |  | B |  |  | D |  |  | D |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 25.3 |  | HCM 2000 | Level of S | Service |  | C |  |  |  |
| HCM 2000 Volume to Capacity ratio |  |  | 0.81 |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length (s) |  |  | 102.5 |  | Sum of lost | time (s) |  |  | 18.0 |  |  |  |
| Intersection Capacity Utilization |  |  | 73.7\% |  | CU Level | f Service |  |  | D |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |

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| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | F |  | \% | $\uparrow$ | F | \% | $\uparrow$ |  | \% | $\uparrow$ |  |
| Traffic Volume (vph) | 110 | 1005 | 68 | 62 | 655 | 75 | 15 | 2 | 11 | 47 | 9 | 30 |
| Future Volume (vph) | 110 | 1005 | 68 | 62 | 655 | 75 | 15 | 2 | 11 | 47 |  | 30 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 5.5 |  | 4.5 | 5.5 | 5.5 | 4.5 | 4.5 |  | 5.0 | 5.0 |  |
| Lane Util. Factor | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Frpb, ped/bikes | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Flpb, ped/bikes | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Fit | 1.00 | 0.99 |  | 1.00 | 1.00 | 0.85 | 1.00 | 0.87 |  | 1.00 | 0.88 |  |
| Flt Protected | 0.95 | 1.00 |  | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.95 | 1.00 |  |
| Satd. Flow (prot) | 1703 | 1721 |  | 1597 | 1639 | 1418 | 1597 | 1468 |  | 1289 | 1331 |  |
| Flt Permitted | 0.35 | 1.00 |  | 0.17 | 1.00 | 1.00 | 0.73 | 1.00 |  | 0.75 | 1.00 |  |
| Satd. Flow (perm) | 632 | 1721 |  | 279 | 1639 | 1418 | 1229 | 1468 |  | 1017 | 1331 |  |
| Peak-hour factor, PHF | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Adj. Flow (vph) | 115 | 1047 | 71 | 65 | 682 | 78 | 16 | 2 | 11 | 49 | 9 | 31 |
| RTOR Reduction (vph) | 0 | 1 | 0 | 0 | 0 | 14 | 0 | 10 | 0 | 0 | 29 | 0 |
| Lane Group Flow (vph) | 115 | 1117 | 0 | 65 | 682 | 64 | 16 | 3 | 0 | 49 | 11 | 0 |
| Confl. Bikes (\#/hr) |  |  | 5 |  |  |  |  |  |  |  |  |  |
| Heavy Vehicles (\%) | 6\% | 8\% | 13\% | 13\% | 15\% | 13\% | 13\% | 13\% | 13\% | 40\% | 13\% | 30\% |
| Bus Blockages (\#/hr) | 0 | 2 | 0 | 0 | 2 | 2 | 0 | 0 | , | 0 | 0 | 0 |
| Turn Type | pm+pt | NA |  | pm+pt | NA | Prot | Perm | NA |  | Perm | NA |  |
| Protected Phases | 5 | , |  | 1 | 6 | 6 |  | 8 |  |  | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |  |
| Actuated Green, G (s) | 125.3 | 118.5 |  | 122.6 | 117.4 | 117.4 | 11.8 | 11.8 |  | 11.3 | 11.3 |  |
| Effective Green, g (s) | 125.3 | 118.5 |  | 122.6 | 117.4 | 117.4 | 11.8 | 11.8 |  | 11.3 | 11.3 |  |
| Actuated g/C Ratio | 0.84 | 0.79 |  | 0.82 | 0.78 | 0.78 | 0.08 | 0.08 |  | 0.08 | 0.08 |  |
| Clearance Time (s) | 4.0 | 5.5 |  | 4.5 | 5.5 | 5.5 | 4.5 | 4.5 |  | 5.0 | 5.0 |  |
| Vehicle Extension (s) | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |  | 3.0 | 3.0 |  |
| Lane Grp Cap (vph) | 576 | 1359 |  | 273 | 1282 | 1109 | 96 | 115 |  | 76 | 100 |  |
| v/s Ratio Prot | c0.01 | c0.65 |  | 0.01 | 0.42 | 0.05 |  | 0.00 |  |  | 0.01 |  |
| v/s Ratio Perm | 0.16 |  |  | 0.19 |  |  | 0.01 |  |  | c0.05 |  |  |
| v/c Ratio | 0.20 | 0.82 |  | 0.24 | 0.53 | 0.06 | 0.17 | 0.02 |  | 0.64 | 0.11 |  |
| Uniform Delay, d1 | 3.1 | 9.4 |  | 10.5 | 6.1 | 3.7 | 64.5 | 63.8 |  | 67.4 | 64.7 |  |
| Progression Factor | 1.00 | 1.00 |  | 0.44 | 0.39 | 0.12 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Incremental Delay, d2 | 0.2 | 5.7 |  | 0.3 | 1.2 | 0.1 | 0.8 | 0.1 |  | 17.2 | 0.5 |  |
| Delay (s) | 3.3 | 15.1 |  | 4.9 | 3.6 | 0.5 | 65.3 | 63.9 |  | 84.6 | 65.2 |  |
| Level of Service | A | B |  | A | A | A | E | E |  | F | E |  |
| Approach Delay (s) |  | 14.0 |  |  | 3.4 |  |  | 64.7 |  |  | 75.9 |  |
| Approach LOS |  | B |  |  | A |  |  | E |  |  | E |  |


| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 2000 Control Delay | 13.2 | HCM 2000 Level of Service | B |
| HCM 2000 Volume to Capacity ratio | 0.78 |  | 15.0 |
| Actuated Cycle Length (s) | 150.0 | Sum of lost time (s) | E |
| Intersection Capacity Utilization | $83.0 \%$ | ICU Level of Service |  |
| Analysis Period (min) | 15 |  |  |
| c Critical Lane Group |  |  |  |


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | $\uparrow$ | 「 | \% ${ }^{*}$ | $\uparrow$ |  |  | $\uparrow$ | F | ${ }^{*}$ | $\hat{\dagger}$ |  |
| Traffic Volume (vph) | 5 | 1031 | 129 | 197 | 784 | 10 | 40 | 0 | 84 | 3 | 1 | 2 |
| Future Volume (vph) | , | 1031 | 129 | 197 | 784 | 10 | 40 | 0 | 84 | 3 | 1 | 2 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 |  |  | 4.5 | 4.5 | 4.5 | 4.5 |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 0.97 | 1.00 |  |  | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Frpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |  | 1.00 | 1.00 | 1.00 | 0.98 |  |
| Flpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |  | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Frt | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 |  |  | 1.00 | 0.85 | 1.00 | 0.90 |  |
| Flt Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  |  | 0.95 | 1.00 | 0.95 | 1.00 |  |
| Satd. Flow (prot) | 1805 | 1743 | 1242 | 2918 | 1676 |  |  | 1250 | 1145 | 1805 | 1010 |  |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  |  | 0.76 | 1.00 | 0.73 | 1.00 |  |
| Satd. Flow (perm) | 1805 | 1743 | 1242 | 2918 | 1676 |  |  | 995 | 1145 | 1385 | 1010 |  |
| Peak-hour factor, PHF | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Adj. Flow (vph) | 5 | 1109 | 139 | 212 | 843 | 11 | 43 | 0 | 90 | 3 | 1 | 2 |
| RTOR Reduction (vph) | 0 | 0 | 19 | 0 | 0 | 0 | 0 | 0 | 84 | 0 | 2 | 0 |
| Lane Group Flow (vph) | 5 | 1109 | 120 | 212 | 854 | 0 | 0 | 43 | 6 | 3 | 1 | 0 |
| Confl. Peds. (\#/hr) |  |  |  |  |  |  | 1 |  |  |  |  | 1 |
| Confl. Bikes (\#/hr) |  |  | 4 |  |  | 1 |  |  |  |  |  |  |
| Heavy Vehicles (\%) | 0\% | 9\% | 30\% | 20\% | 13\% | 20\% | 44\% | 0\% | 41\% | 0\% | 100\% | 50\% |
| Turn Type | Prot | NA | Prot | Prot | NA |  | Perm | NA | Perm | Perm | NA |  |
| Protected Phases | 5 | 2 | 2 | 1 | 6 |  |  | 8 |  |  | 4 |  |



| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 2000 Control Delay | 21.0 | HCM 2000 Level of Service | C |
| HCM 2000 Volume to Capacity ratio | 0.83 |  | 14.0 |
| Actuated Cycle Length (s) | 150.0 | Sum of lost time (s) | D |
| Intersection Capacity Utilization | $80.4 \%$ | ICU Level of Service |  |
| Analysis Period (min) | 15 |  |  |
| c Critical Lane Group |  |  |  |




## MOVEMENT SUMMARY

Site: 10 [SW Oregon St \& Murdock Rd]
Year 2021 - Total Cipole Extension AM Peak Hour Conditions
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | $\begin{aligned} & \text { OD } \\ & \text { Mov } \end{aligned}$ | Dema Total veh/h | $\begin{gathered} \text { Fows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | f Queue <br> Distance | Prop. Queued | Effective Stop Rate per veh | Average Speed mph |
| South: Murdock Road |  |  |  |  |  |  |  |  |  |  |  |
| 3 | L2 | 91 | 1.0 | 0.615 | 13.5 | LOS B | 4.9 | 123.6 | 0.77 | 0.83 | 29.3 |
| 18 | R2 | 448 | 1.0 | 0.615 | 13.5 | LOS B | 4.9 | 123.6 | 0.77 | 0.83 | 27.9 |
| Appr |  | 539 | 1.0 | 0.615 | 13.5 | LOS B | 4.9 | 123.6 | 0.77 | 0.83 | 28.1 |
| East: Oregon St |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 98 | 14.0 | 0.255 | 5.5 | LOS A | 1.2 | 32.9 | 0.26 | 0.13 | 32.5 |
| 6 | T1 | 194 | 8.0 | 0.255 | 5.5 | LOS A | 1.2 | 32.9 | 0.26 | 0.13 | 32.1 |
| Appr |  | 292 | 10.0 | 0.255 | 5.5 | LOS A | 1.2 | 32.9 | 0.26 | 0.13 | 32.2 |
| West: Oregon St. |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T1 | 427 | 2.0 | 0.407 | 7.0 | LOS A | 2.6 | 65.0 | 0.36 | 0.20 | 32.3 |
| 12 | R2 | 65 | 2.0 | 0.407 | 7.0 | LOS A | 2.6 | 65.0 | 0.36 | 0.20 | 31.2 |
| Approach |  | 492 | 2.0 | 0.407 | 7.0 | LOS A | 2.6 | 65.0 | 0.36 | 0.20 | 32.2 |
| All Ve | cles | 1322 | 3.4 | 0.615 | 9.3 | LOS A | 4.9 | 123.6 | 0.51 | 0.44 | 30.4 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: Same as Sign Control.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: US HCM 6.
HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies. Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: KITTELSON AND ASSOCIATES INC | Processed: Wednesday, March 13, 2019 2:54:21 PM
Project: H:\23|23278-Orr Property Corporate ParklsynchrolDec 2019 TIA analysis|SidralFuture 2021123278_Total Cipole Extension 2021 AM.sip7




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|  | 4 |  |  |  |  |  | 4 | 4 | 7 |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\uparrow$ |  | \% | $\uparrow$ | 「 | ${ }_{1}$ | $\uparrow$ |  | ${ }^{*}$ | $\hat{F}$ |  |
| Traffic Volume (vph) | 37 | 909 | 18 | 17 | 1183 | 14 | 60 | - | 45 | 63 | 2 | 124 |
| Future Volume (vph) | 37 | 909 | 18 | 17 | 1183 | 14 | 60 | 9 | 45 | 63 | 2 | 124 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 5.5 |  | 4.0 | 5.5 | 5.5 | 5.0 | 5.0 |  | 5.0 | 5.0 |  |
| Lane Util. Factor | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Frpb, ped/bikes | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Flpb, ped/bikes | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Fit | 1.00 | 1.00 |  | 1.00 | 1.00 | 0.85 | 1.00 | 0.88 |  | 1.00 | 0.85 |  |
| Flt Protected | 0.95 | 1.00 |  | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.95 | 1.00 |  |
| Satd. Flow (prot) | 1752 | 1805 |  | 1671 | 1830 | 1405 | 1671 | 1540 |  | 1703 | 1586 |  |
| Flt Permitted | 0.04 | 1.00 |  | 0.20 | 1.00 | 1.00 | 0.46 | 1.00 |  | 0.72 | 1.00 |  |
| Satd. Flow (perm) | 77 | 1805 |  | 352 | 1830 | 1405 | 815 | 1540 |  | 1288 | 1586 |  |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 40 | 988 | 20 | 18 | 1286 | 15 | 65 | 10 | 49 | 68 | 2 | 135 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 44 | 0 | 0 | 120 | 0 |
| Lane Group Flow (vph) | 40 | 1008 | 0 | 18 | 1286 | 11 | 65 | 15 | 0 | 68 | 17 | 0 |
| Confl. Peds. (\#/hr) | 2 |  |  |  |  | 2 |  |  |  |  |  |  |
| Confl. Bikes (\#/hr) |  |  | 1 |  |  | 4 |  |  |  |  |  |  |
| Heavy Vehicles (\%) | 3\% | 4\% | 8\% | 8\% | 3\% | 14\% | 8\% | 8\% | 8\% | 6\% | 8\% | 2\% |
| Bus Blockages (\#/hr) | 0 | 2 | 0 | 0 | 2 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| Turn Type | pm+pt | NA |  | pm+pt | NA | Prot | Perm | NA |  | Perm | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 | 6 |  | 8 |  |  | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |  |
| Actuated Green, G (s) | 101.1 | 95.9 |  | 95.3 | 93.0 | 93.0 | 13.9 | 13.9 |  | 13.9 | 13.9 |  |
| Effective Green, g (s) | 101.1 | 95.9 |  | 95.3 | 93.0 | 93.0 | 13.9 | 13.9 |  | 13.9 | 13.9 |  |
| Actuated g/C Ratio | 0.80 | 0.76 |  | 0.75 | 0.73 | 0.73 | 0.11 | 0.11 |  | 0.11 | 0.11 |  |
| Clearance Time (s) | 4.0 | 5.5 |  | 4.0 | 5.5 | 5.5 | 5.0 | 5.0 |  | 5.0 | 5.0 |  |
| Vehicle Extension (s) | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |  | 3.0 | 3.0 |  |
| Lane Grp Cap (vph) | 130 | 1367 |  | 288 | 1344 | 1032 | 89 | 169 |  | 141 | 174 |  |
| v/s Ratio Prot | c0.01 | 0.56 |  | 0.00 | c0.70 | 0.01 |  | 0.01 |  |  | 0.01 |  |
| $\mathrm{v} / \mathrm{s}$ Ratio Perm | 0.23 |  |  | 0.05 |  |  | c0.08 |  |  | 0.05 |  |  |
| v/c Ratio | 0.31 | 0.74 |  | 0.06 | 0.96 | 0.01 | 0.73 | 0.09 |  | 0.48 | 0.10 |  |
| Uniform Delay, d1 | 31.1 | 8.4 |  | 7.8 | 15.0 | 4.5 | 54.5 | 50.7 |  | 53.0 | 50.7 |  |
| Progression Factor | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Incremental Delay, d2 | 1.3 | 2.1 |  | 0.1 | 15.3 | 0.0 | 26.2 | 0.2 |  | 2.6 | 0.2 |  |
| Delay (s) | 32.4 | 10.5 |  | 7.9 | 30.3 | 4.5 | 80.8 | 50.9 |  | 55.6 | 50.9 |  |
| Level of Service | C | B |  | A | C | A | F | D |  | E | D |  |
| Approach Delay (s) |  | 11.4 |  |  | 29.7 |  |  | 66.5 |  |  | 52.5 |  |
| Approach LOS |  | B |  |  | C |  |  | E |  |  | D |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 26.0 |  | HCM 2000 | Level of S | ervice |  | C |  |  |  |
| HCM 2000 Volume to Capacity ratio |  |  | 0.90 |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length (s) |  |  | 126.6 |  | Sum of los | time (s) |  |  | 14.5 |  |  |  |
| Intersection Capacity Utilization |  |  | 87.1\% |  | ICU Level | f Service |  |  | E |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |

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c Critical Lane Group


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |




## MOVEMENT SUMMARY

Site: 10 [SW Oregon St \& Murdock Rd]
Year 2021 - Total Cipole Extension PM Peak Hour Conditions
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | $\begin{aligned} & \text { OD } \\ & \text { Mov } \end{aligned}$ | Dema Total veh/h | $\begin{gathered} \text { lows } \\ \text { HV } \\ \% \\ \hline \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance ft | Prop. Queued | Effective Stop Rate per veh | Average Speed mph |
| South: Murdock Road |  |  |  |  |  |  |  |  |  |  |  |
| 3 | L2 | 91 | 1.0 | 0.231 | 5.4 | LOS A | 1.1 | 28.6 | 0.39 | 0.26 | 32.2 |
| 18 | R2 | 162 | 4.0 | 0.231 | 5.4 | LOS A | 1.1 | 28.6 | 0.39 | 0.26 | 30.5 |
| Appro |  | 253 | 2.9 | 0.231 | 5.4 | LOS A | 1.1 | 28.6 | 0.39 | 0.26 | 31.1 |
| East: Oregon St |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 421 | 1.0 | 0.687 | 12.4 | LOS B | 7.4 | 186.4 | 0.55 | 0.30 | 29.3 |
| 6 | T1 | 434 | 1.0 | 0.687 | 12.4 | LOS B | 7.4 | 186.4 | 0.55 | 0.30 | 28.9 |
| Approach |  | 855 | 1.0 | 0.687 | 12.4 | LOS B | 7.4 | 186.4 | 0.55 | 0.30 | 29.1 |
| West: Oregon St. |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T1 | 196 | 2.0 | 0.323 | 7.7 | LOS A | 1.6 | 39.8 | 0.59 | 0.53 | 31.9 |
|  | R2 | 87 | 2.0 | 0.323 | 7.7 | LOS A | 1.6 | 39.8 | 0.59 | 0.53 | 30.8 |
| Approach |  | 283 | 2.0 | 0.323 | 7.7 | LOS A | 1.6 | 39.8 | 0.59 | 0.53 | 31.6 |
| All Vehicles |  | 1391 | 1.6 | 0.687 | 10.2 | LOS B | 7.4 | 186.4 | 0.53 | 0.34 | 29.9 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: Same as Sign Control.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: US HCM 6.
HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies. Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## Appendix N Year 2025 Total Traffic Conditions - Alternative Access Scenario

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

c Critical Lane Group

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |




| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \％${ }^{1}$ | 个4 | 「 | ＊＊ | 个4 | 「 | \％${ }^{1 / 1}$ | 性 |  | \％ | 个 ${ }_{\text {d }}$ |  |
| Traffic Volume（vph） | 78 | 1059 | 47 | 26 | 663 | 200 | 133 | 230 | 80 | 182 | 172 | 76 |
| Future Volume（vph） | 78 | 1059 | 47 | 26 | 663 | 200 | 133 | 230 | 80 | 182 | 172 | 76 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time（s） | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 |  | 4.0 | 5.5 |  |
| Lane Util．Factor | 0.97 | 0.95 | 1.00 | 0.97 | 0.95 | 1.00 | 0.97 | 0.95 |  | 1.00 | 0.95 |  |
| Frpb，ped／bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Flpb，ped／bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Fit | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 0.96 |  | 1.00 | 0.95 |  |
| Flt Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.95 | 1.00 |  |
| Satd．Flow（prot） | 3242 | 3299 | 1252 | 2334 | 3127 | 1381 | 3155 | 3006 |  | 1612 | 2961 |  |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.26 | 1.00 |  |
| Satd．Flow（perm） | 3242 | 3299 | 1252 | 2334 | 3127 | 1381 | 3155 | 3006 |  | 449 | 2961 |  |
| Peak－hour factor，PHF | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Adj．Flow（vph） | 84 | 1139 | 51 | 28 | 713 | 215 | 143 | 247 | 86 | 196 | 185 | 82 |
| RTOR Reduction（vph） | 0 | 0 | 15 | 0 | 0 | 59 | 0 | 27 | 0 | 0 | 37 | 0 |
| Lane Group Flow（vph） | 84 | 1139 | 36 | 28 | 713 | 156 | 143 | 306 | 0 | 196 | 230 | 0 |
| Confl．Bikes（\＃／hr） |  |  | 3 |  |  | 1 |  |  |  |  |  |  |
| Heavy Vehicles（\％） | 8\％ | 9\％ | 28\％ | 50\％ | 15\％ | 16\％ | 11\％ | 10\％ | 31\％ | 12\％ | 12\％ | 26\％ |
| Bus Blockages（\＃／hr） | 0 | 2 | 2 | 0 | 2 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| Turn Type | Prot | NA | pt＋ov | Prot | NA | pt＋ov | Prot | NA |  | pm＋pt | NA |  |
| Protected Phases | 5 | 2 | 23 | 1 | 6 | 67 | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases |  |  |  |  |  |  |  |  |  | 4 |  |  |
| Actuated Green，G（s） | 7.8 | 89.6 | 104.6 | 4.0 | 85.8 | 108.6 | 9.5 | 20.1 |  | 41.4 | 27.9 |  |
| Effective Green，g（s） | 7.8 | 89.6 | 104.6 | 4.0 | 85.8 | 108.6 | 9.5 | 20.1 |  | 41.4 | 27.9 |  |
| Actuated g／C Ratio | 0.05 | 0.60 | 0.70 | 0.03 | 0.57 | 0.72 | 0.06 | 0.13 |  | 0.28 | 0.19 |  |
| Clearance Time（s） | 4.0 | 5.5 |  | 4.0 | 5.5 |  | 4.0 | 5.5 |  | 4.0 | 5.5 |  |
| Vehicle Extension（s） | 1.5 | 4.5 |  | 1.5 | 4.5 |  | 0.2 | 2.0 |  | 0.2 | 2.0 |  |
| Lane Grp Cap（vph） | 168 | 1970 | 873 | 62 | 1788 | 999 | 199 | 402 |  | 258 | 550 |  |
| v／s Ratio Prot | c0．03 | c0．35 | 0.03 | 0.01 | 0.23 | 0.11 | 0.05 | 0.10 |  | c0．09 | 0.08 |  |
| v／s Ratio Perm |  |  |  |  |  |  |  |  |  | c0．12 |  |  |
| v／c Ratio | 0.50 | 0.58 | 0.04 | 0.45 | 0.40 | 0.16 | 0.72 | 0.76 |  | 0.76 | 0.42 |  |
| Uniform Delay，d1 | 69.2 | 18.6 | 7.1 | 71.9 | 17.8 | 6.4 | 68.9 | 62.6 |  | 45.4 | 53.9 |  |
| Progression Factor | 1.14 | 0.78 | 0.47 | 1.38 | 0.35 | 0.93 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Incremental Delay，d2 | 0.8 | 1.1 | 0.0 | 1.8 | 0.6 | 0.0 | 9.9 | 7.5 |  | 10.8 | 0.2 |  |
| Delay（s） | 80.0 | 15.7 | 3.3 | 101.1 | 6.9 | 6.0 | 78.8 | 70.1 |  | 56.2 | 54.1 |  |
| Level of Service | E | B | A | F | A | A | E | E |  | E | D |  |
| Approach Delay（s） |  | 19.4 |  |  | 9.4 |  |  | 72.7 |  |  | 55.0 |  |
| Approach LOS |  | B |  |  | A |  |  | E |  |  | D |  |


| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 2000 Control Delay | 29.6 | HCM 2000 Level of Service | C |
| HCM 2000 Volume to Capacity ratio | 0.65 |  | 19.0 |
| Actuated Cycle Length（s） | 150.0 | Sum of lost time（s） | C |
| Intersection Capacity Utilization | $64.9 \%$ | ICU Level of Service |  |
| Analysis Period（min） | 15 |  |  |
| C Critical Lane Group |  |  |  |



|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |



## MOVEMENT SUMMARY

Site: 10 [SW Oregon St \& Murdock Rd]
Year 2025 - Total Cipole Extension AM Peak Hour Conditions
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | $\begin{aligned} & \text { OD } \\ & \text { Mov } \end{aligned}$ | Dema <br> Total veh/h | $\begin{array}{r} \text { =lows } \\ \text { HV } \\ \% \\ \hline \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | f Queue Distance ft | Prop. Queued | Effective Stop Rate per veh | Average Speed mph |
| South: Murdock Road |  |  |  |  |  |  |  |  |  |  |  |
| 3 | L2 | 95 | 1.0 | 0.666 | 15.5 | LOS C | 5.8 | 146.8 | 0.82 | 0.93 | 28.6 |
| 18 | R2 | 474 | 1.0 | 0.666 | 15.5 | LOS C | 5.8 | 146.8 | 0.82 | 0.93 | 27.2 |
| Appro |  | 569 | 1.0 | 0.666 | 15.5 | LOS C | 5.8 | 146.8 | 0.82 | 0.93 | 27.4 |
| East: Oregon St |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 104 | 14.0 | 0.271 | 5.7 | LOS A | 1.3 | 35.5 | 0.28 | 0.14 | 32.4 |
| 6 | T1 | 205 | 8.0 | 0.271 | 5.7 | LOS A | 1.3 | 35.5 | 0.28 | 0.14 | 32.0 |
| Approach |  | 308 | 10.0 | 0.271 | 5.7 | LOS A | 1.3 | 35.5 | 0.28 | 0.14 | 32.1 |
| West: Oregon St. |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T1 | 451 | 2.0 | 0.433 | 7.4 | LOS A | 2.8 | 71.2 | 0.39 | 0.22 | 32.1 |
| 12 | R2 | 68 | 2.0 | 0.433 | 7.4 | LOS A | 2.8 | 71.2 | 0.39 | 0.22 | 31.0 |
| Approach |  | 519 | 2.0 | 0.433 | 7.4 | LOS A | 2.8 | 71.2 | 0.39 | 0.22 | 32.0 |
| All Vehicles |  | 1396 | 3.4 | 0.666 | 10.3 | LOS B | 5.8 | 146.8 | 0.54 | 0.49 | 30.0 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: Same as Sign Control.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: US HCM 6.
HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies. Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Extension 2025 AM.sip7




Citical

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |



c Critical Lane Group


Citical


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{7}$ | 个4 | 「 | ${ }^{7 *}$ | 性 |  |  | $\uparrow$ | F＇ | ${ }^{7}$ | F |  |
| Traffic Volume（vph） | 12 | 1097 | 60 | 52 | 1003 | 16 | 111 | 0 | 229 | 16 | 1 | 15 |
| Future Volume（vph） | 12 | 1097 | 60 | 52 | 1003 | 16 | 111 | 0 | 229 | 16 | 1 | 15 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time（s） | 4.5 | 5.5 | 5.5 | 4.0 | 5.5 |  |  | 4.5 | 4.5 | 4.5 | 4.5 |  |
| Lane Util．Factor | 1.00 | 0.95 | 1.00 | 0.97 | 0.95 |  |  | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Frpb，ped／bikes | 1.00 | 1.00 | 0.99 | 1.00 | 1.00 |  |  | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Flpb，ped／bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |  | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Fit | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 |  |  | 1.00 | 0.85 | 1.00 | 0.86 |  |
| Flt Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  |  | 0.95 | 1.00 | 0.95 | 1.00 |  |
| Satd．Flow（prot） | 1805 | 3471 | 1533 | 2894 | 3497 |  |  | 1770 | 1568 | 1805 | 1632 |  |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  |  | 0.75 | 1.00 | 0.54 | 1.00 |  |
| Satd．Flow（perm） | 1805 | 3471 | 1533 | 2894 | 3497 |  |  | 1390 | 1568 | 1030 | 1632 |  |
| Peak－hour factor，PHF | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Adj．Flow（vph） | 13 | 1155 | 63 | 55 | 1056 | 17 | 117 | 0 | 241 | 17 | 1 | 16 |
| RTOR Reduction（vph） | 0 | 0 | 15 | 0 | 1 | 0 | 0 | 0 | 213 | 0 | 14 | 0 |
| Lane Group Flow（vph） | 13 | 1155 | 48 | 55 | 1072 | 0 | 0 | 117 | 28 | 17 | 3 | 0 |
| Confl．Peds．（\＃／hr） | 2 |  |  |  |  | 2 |  |  |  |  |  |  |
| Confl．Bikes（\＃／hr） |  |  | 2 |  |  |  |  |  |  |  |  |  |
| Heavy Vehicles（\％） | 0\％ | 4\％ | 4\％ | 21\％ | 3\％ | 0\％ | 2\％ | 0\％ | 3\％ | 0\％ | 0\％ | 0\％ |
| Turn Type | Prot | NA | Perm | Prot | NA |  | Perm | NA | Perm | Perm | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |



| Intersection Summary |  |  | B |
| :--- | ---: | :--- | ---: |
| HCM 2000 Control Delay | 14.0 | HCM 2000 Level of Service |  |
| HCM 2000 Volume to Capacity ratio | 0.49 |  | 14.5 |
| Actuated Cycle Length（s） | 120.0 | Sum of lost time（s） | B |
| Intersection Capacity Utilization | $60.8 \%$ | ICU Level of Service |  |
| Analysis Period（min） | 15 |  |  |
| C Critical Lane Group |  |  |  |




## MOVEMENT SUMMARY

Site: 10 [SW Oregon St \& Murdock Rd]
Year 2025 - Total Cipole Extension PM Peak Hour Conditions
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | $\begin{aligned} & \text { OD } \\ & \text { Mov } \end{aligned}$ | Dema Total veh/h | $\begin{gathered} \text { lows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | f Queue <br> Distance | Prop. Queued | Effective Stop Rate per veh | Average Speed mph |
| South: Murdock Road |  |  |  |  |  |  |  |  |  |  |  |
| 3 | L2 | 97 | 1.0 | 0.249 | 5.7 | LOS A | 1.2 | 31.3 | 0.41 | 0.28 | 32.1 |
| 18 | R2 | 173 | 4.0 | 0.249 | 5.7 | LOS A | 1.2 | 31.3 | 0.41 | 0.28 | 30.4 |
| Appr |  | 269 | 2.9 | 0.249 | 5.7 | LOS A | 1.2 | 31.3 | 0.41 | 0.28 | 31.0 |
| East: Oregon St |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 446 | 1.0 | 0.731 | 14.0 | LOS B | 8.7 | 218.9 | 0.63 | 0.36 | 28.7 |
| 6 | T1 | 458 | 1.0 | 0.731 | 14.0 | LOS B | 8.7 | 218.9 | 0.63 | 0.36 | 28.3 |
| Approach |  | 904 | 1.0 | 0.731 | 14.0 | LOS B | 8.7 | 218.9 | 0.63 | 0.36 | 28.5 |
| West: Oregon St. |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T1 | 206 | 2.0 | 0.350 | 8.2 | LOS A | 1.7 | 43.6 | 0.61 | 0.57 | 31.7 |
|  | R2 | 93 | 2.0 | 0.350 | 8.2 | LOS A | 1.7 | 43.6 | 0.61 | 0.57 | 30.6 |
| Approach |  | 299 | 2.0 | 0.350 | 8.2 | LOS A | 1.7 | 43.6 | 0.61 | 0.57 | 31.3 |
| All Vehicles |  | 1473 | 1.6 | 0.731 | 11.3 | LOS B | 8.7 | 218.9 | 0.58 | 0.39 | 29.5 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: Same as Sign Control.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: US HCM 6.
HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies. Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Extension 2025 PM.sip7

|  | $\rangle$ |  |  | 7 |  |  | 4 | $\uparrow$ |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\hat{1}$ |  | \% | $\uparrow$ |  | \% | $\uparrow$ |  | \% | $\hat{\beta}$ |  |
| Traffic Volume (veh/h) | 26 | 5 | 27 | 22 | 14 | 16 | 5 | 246 | 3 | 2 | 346 | 44 |
| Future Volume (Veh/h) | 26 | 5 | 27 | 22 | 14 | 16 | 5 | 246 | 3 | 2 | 346 | 44 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Hourly flow rate (vph) | 27 | 5 | 28 | 23 | 15 | 17 | 5 | 256 | 3 | 2 | 360 | 46 |
| Pedestrians |  |  |  |  | 1 |  |  | 1 |  |  | 2 |  |
| Lane Width (ft) |  |  |  |  | 12.0 |  |  | 12.0 |  |  | 12.0 |  |
| Walking Speed (fts) |  |  |  |  | 3.5 |  |  | 3.5 |  |  | 3.5 |  |
| Percent Blockage |  |  |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | TWLTL |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  | 2 |  |
| Upstream signal (ft) |  |  |  |  |  |  |  |  |  |  | 978 |  |
| pX, platoon unblocked | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 |  | 0.89 |  |  |  |  |  |
| vC , conflicting volume | 680 | 657 | 384 | 664 | 678 | 260 | 406 |  |  | 260 |  |  |
| $\mathrm{vC1}$, stage 1 conf vol | 387 | 387 |  | 268 | 268 |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol | 292 | 270 |  | 396 | 410 |  |  |  |  |  |  |  |
| vCu , unblocked vol | 582 | 557 | 252 | 565 | 581 | 260 | 277 |  |  | 260 |  |  |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| $\mathrm{tC}, 2$ stage (s) | 6.1 | 5.5 |  | 6.1 | 5.5 |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| po queue free \% | 95 | 99 | 96 | 96 | 97 | 98 | 100 |  |  | 100 |  |  |
| cM capacity (veh/h) | 550 | 543 | 696 | 548 | 531 | 769 | 1135 |  |  | 1286 |  |  |
| Direction, Lane \# | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 | SB 1 | SB 2 |  |  |  |  |
| Volume Total | 27 | 33 | 23 | 32 | 5 | 259 | 2 | 406 |  |  |  |  |
| Volume Left | 27 | 0 | 23 | 0 | 5 | 0 | 2 | 0 |  |  |  |  |
| Volume Right | 0 | 28 | 0 | 17 | 0 | 3 | 0 | 46 |  |  |  |  |
| CSH | 550 | 668 | 548 | 635 | 1135 | 1700 | 1286 | 1700 |  |  |  |  |
| Volume to Capacity | 0.05 | 0.05 | 0.04 | 0.05 | 0.00 | 0.15 | 0.00 | 0.24 |  |  |  |  |
| Queue Length 95th (ft) | 4 | 4 | 3 | 4 | 0 | 0 | 0 | 0 |  |  |  |  |
| Control Delay (s) | 11.9 | 10.7 | 11.9 | 11.0 | 8.2 | 0.0 | 7.8 | 0.0 |  |  |  |  |
| Lane LOS | B | B | B | B | A |  | A |  |  |  |  |  |
| Approach Delay (s) | 11.2 |  | 11.3 |  | 0.2 |  | 0.0 |  |  |  |  |  |
| Approach LOS | B |  | B |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 1.7 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 35.8\% | ICU Level of Service |  |  |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |



January 9, 2020
Jeff Shoemaker, P.E. DOW
720 SW Washington Street, Suite 750
Portland, OR 97205

## RE: DESIGN EXCEPTION

T-S CORPORATE PARK PROPERTY CITY OF SHERWOOD

The following is in response to your November 25, 2019 Request for Exception to Section 220 of the Washington County Road Design \& Construction Standards, subsection 340.070 for one proposed access on the south side of SW Cipole Road on SW Tualatin-Sherwood Road, an arterial, for the subject development in the city of Sherwood.

XX Your request is approved with the following conditions.

1. Construct the access on SW Tualatin-Sherwood Road with two outbound lanes (one left lane and one shared through/right-turn lane) and one inbound lane. Coordinate with Washington County, design and construct the new added traffic signal for the proposed access and modify the existing intersection traffic signal as needed.

Please be advised that all exceptions granted to the W.C.R.D.C.S. are considered unique and are not uniformly applicable.

NOTE: See Section 220.020.4 for appeal procedures should you wish to appeal.
Sincerely,
Stony a Shatter
Stacy Shetler, P.E.
County Engineer
Reviewed By: Jinde Zhu


Approved By: John Fasana


SS:JZ:tf
c: John Fasana
Jinde Zhu
Naomi Vogel
Plan Review File
c/File

## $D \square W L$

November 25, 2019
Mr. Stacy Shetler, County Engineer
Engineering, Traffic and Survey Division
Washington County
1400 SW Walnut Street, MS 17 VIA Email
Hillsboro, Oregon 97123

## RE: T-S Corporate Park Property (Lot 2S128D001100) <br> Request for Exception to County Road Design and Construction Standards For Local Access to SW Tualatin-Sherwood Rd., a County Arterial

Please accept this letter with attachments as a formal request for approval of a Design Exception to the Washington County Community Development Code Section 501-8.5(B)(4) pertaining to the subject property, a 46.5 -acre parcel at 12822 SW Tualatin-Sherwood Road (TS Rd.). Specifically, we are requesting a Design Exception for direct access to an arterial, T-S Rd., that is not from a collector or other arterial street.

## Background

Trammell Crow Company (TCC), the developer and Applicant, is planning to develop a multibuilding industrial park containing approximately 535,000 square feet on the subject site located at the southwest corner of T-S Rd. and SW $124^{\text {th }}$ Avenue ( $124^{\text {th }}$ ). The southern boundary of the site is the future SW Blake Road (Blake). The site, currently in Washington County, is being annexed to the City of Sherwood. Annexation of the site is expected to become effective in early 2020.

The proposed industrial park development at the site supports Metro, County, and City objectives for economic development, existing business support, employment opportunity, and land development. The guiding documents for site development and vehicular access - to occur after it is annexed to the City of Sherwood - are as follows:

TEA Implementation Plan, the Tonquin Employment Area Market Analysis, Business Recruitment Strategy, and Implementation Plan

- Project partners were Metro, Washington County, and the City of Sherwood
- Sherwood City Council accepted on June 16, 2015 (Resolution 2015-051)
- Washington County Board of Commissioners acknowledged as part of the Washington County Industrial Sites Assessment and Implementation Project on June 23, 2015 (Resolution \& Order 15-48)

TSPV1, the Sherwood Transportation System Plan, Volume 1

- Sherwood City Council adopted on June 17, 2014 (Ordinance 2014-012)

TSPV2, the Sherwood Transportation System Plan, Volume 2

- Sherwood City Council adopted on June 17, 2014 (Ordinance 2014-012)
- City of Sherwood updated in April 2018

These Transportation Plans call for one vehicular access point for the site from/to T-S Rd. at the Cipole intersection. Vehicular access to the site is not permitted from/to $124^{\text {th }}$. The Transportation Plans do not specify a roadway of any classification to be extended into the site. The TEA Implementation Plan (page 27) assumes that "an internal drive will be located here." Mackenzie, a consultant team member of the TEA Implementation Plan, provided a summary review of the Transportation Plans associated with the property in its Letter dated August 29, 2019, attached hereto as Exhibit A.

## Developer Traffic Studies

## Preliminary Findings Memorandum dated April 2019

Sherwood city officials are familiar with TCC's consideration to subdivide the site into multiple lots to support the multi-building development. The developer may submit a subdivision application to the City upon annexation of the property to Sherwood. Since each of the future lots resulting from a subdivision must front and be accessible from a public street, in 2018, the City of Sherwood asked the developer to analyze two possible conditions of an access road from T-S Rd. at the Cipole intersection (aka, the Cipole extension):

1) The access road terminates on-site in a cul-de-sac, and
2) The access road connects through the site to Blake

In response to this request from Sherwood, Kittelson and Associates, Inc. (KAI) produced a Preliminary Findings Memorandum dated April 2019, a copy of which is included in Exhibit B. This technical document found the following:
a) Traffic Operations: All studied intersections in the area are anticipated to meet the regional mobility standard in either scenario. There appears to be no significant system-wide benefit whether the road terminates in a cul-de-sac or whether it extends and connects to Blake.
b) Traffic Safety: If the access road is extended through the site to Blake, this new intersection would introduce an unprotected left-turn movement on the system, thereby reducing traffic safety, particularly with large trucks.

Support Memo for Washington County DLUT Design Exception dated November 15, 2019
Following guidance of Washington County in preparation of this Request, KAI produced a subsequent technical document, the Support Memorandum for Washington County DLUT Design Exception dated November 15, 2019, attached hereto as Exhibit B. This report analyzed the proposed access road for 2021 and 2025 operations and left-turn queuing conditions along T-S Rd. at the Cipole intersection. The findings of this report are summarized below.

## Reasons For Request

Washington County Community Development Code Section 501-8.5(B)(4) states that "direct access to arterial roads shall be from a collector or other arterial streets." Since the Transportation Plans do not call for an arterial or collector at the site's sole access point to T-S Rd., an arterial road, a design exception is requested.

The County applies the approval criteria of Section 220.020.1 of the Design and Construction Standards (DCS) to determine whether to allow a requested design exception. These standards
are provided below with a response regarding how this request satisfies the applicable criteria. Per DCS 220.020.1, the exception can be granted when one of three conditions is met.

## A. The specification or standard does not apply in the particular application

Response: This condition is met because the Transportation Plans do not present a planned arterial or collector connection through the subject property at the Cipole intersection of T-S Rd. as presented in the following reference material:

- TSPV1 Figure 17, Street Functional Classification, shows no proposed roadway of any classification at this access point,
- TSPV2 Section D, Project Options Technical Report, Figure 1, Motor Vehicle Projects (updated 10/30/17), shows no proposed roadway of any classification at this access point,
- TEA Implementation Plan Figure 18, Implementation Plan, shows only "Anticipated Access" and no proposed roadway of any classification at this access point,
- TEA Implementation Plan specifically notes that this connection is assumed to be "an internal drive" (page 27),
- Washington County TSP Functional Classification Urban Area Map 6 does not illustrate an existing or proposed Arterial, Collector, or Neighborhood Route south of T-S Rd. between Oregon Street and 124th.

The Transportation Plans do not call for any roadway, much less one classified as an arterial or collector to serve the subject site at the T-S Rd.-Cipole intersection. Therefore, the standard in this particular application does not apply since it is inconsistent with the Transportation Plans.
B. Topography, right-of-way or other geographic conditions impose an economic hardship on the applicant and an equivalent alternative is available which can accomplish the same design objective

Response: This condition is met as evidenced by the TEA Implementation Plan, which considered numerous factors, such as those listed in this condition, and concluded that no arterial or collector roadway should be located south of the Cipole intersection with T-S Rd. The Mackenzie letter (Exhibit A) reinforces this conclusion.

Recently, the planned alignment of Blake has shifted north by approximately 600 feet, compared to the Transportation Plans. This new alignment exacerbates the conditions, such as topography, considered in the TEA Implementation Plan.

As referenced in the KAI Memo (Exhibit B), access into the site via an arterial or collector is "impracticable" because a roadway of either classification would need to have limited curvature and connect to Blake. A roadway connected to Blake would have curvature to avoid the wetlands present on the site, and it would have a steep trajectory at a grade of approximately $12 \%$, which would be incompatible with the City of Sherwood Employment Industrial (EI) zoning applicable to this site and neighboring properties. Future uses at the site will require large trucks and the steep grade would be nonfunctional and dangerous. See Exhibit C for a site plan that illustrates a local access road terminating in a cul-de-sac and the steep grade from the cul-desac south to Blake.

In addition to being impracticable, nonfunctional and dangerous, an arterial or collector roadway constructed between T-S Rd. and Blake would impose an economic hardship and create side slopes that would drastically reduce the amount of building area on the site. Reduced building area limits the economic development, existing business support, and employment opportunity objectives of the governmental stakeholders.

As an abutting arterial, $124^{\text {th }}$ provides the alternative north-south through-put on the transportation system for the immediate area, and in fact is superior because it does not suffer from the grade differential challenges that a Cipole connection to Blake would experience.
C. A minor change to a specification or standard is required to address a specific design or construction problem which if not allowed will result in an undue economic hardship

Response: This criterion is not applicable to this design exception request.

## Comparison (Existing Standard v. Proposed)

Table 1. Comparison - Existing Standard v. Proposed Access

|  | Standard | Proposed |
| :--- | :--- | :--- |
| CDC 501-8.5(B)(4) | Access to arterial roads must <br> come from Arterial or <br> Collector | No access provided by an Arterial or <br> Collector |

## Public Safety

No adverse impacts to traffic operations or public safety are anticipated. See the KAI Memo that addresses the anticipated intersection operations and left turn queues at key intersections around the future TCC development site, both under a 2021 condition and a 2025 condition. These conditions are provided in the memorandum to represent the condition of T-S Rd. before and after the planned widening of the corridor ${ }^{1}$ and intersection improvements anticipated at $124^{\text {th }}$, where dual eastbound left turn lanes are planned with the county's upcoming T-S Rd. widening project.

## 2021 and 2025 Operations

The intersections adjacent to the site will operate during the weekday AM and PM peak hours at levels which meet the governing regional or City operating standard under year 2021 and 2025 conditions with the site fully developed.

As noted in the KAI Memo the following queuing conditions are anticipated under the 2021 and 2025 conditions.

[^9]
## 2021 Left Turn Queue Condition (between Cipole and $124^{\text {th }}$ )

Under the peak hour queueing scenarios modeled in the 2021 condition, eastbound left turn lane $95^{\text {th }}$ percentile queues on T-S Rd. at $124^{\text {th }}$ could reach 400 feet in the AM peak hour and 125 feet in the PM peak hour. As noted in the KAI Memo, the existing queue storage and taper space can accommodate this queue.

The westbound left turn queue for vehicles entering the TCC site is anticipated to reach a $95^{\text {th }}$ percentile queue of up to 175 feet during the AM peak hour and 150 feet in the PM peak hour. These queues can be contained within the approximately 250 -feet of turn bay storage at Cipole.

As a consequence, no left-turn conflicts are anticipated in the back-to-back turn lanes between the proposed site access at Cipole and the $124^{\text {th }}$ intersection under the AM and PM peak hour scenarios in the 2021 condition of T-S Rd. including the buildout of the TCC development.

## 2025 Left Turn Queue Condition (between Cipole and $124^{\text {th }}$ )

The KAI Memo also addresses queueing conditions that are expected in 2025 after completion of the county's T-S Rd. widening project. As noted on page 14 of the KAI Memo, it is expected that the $95^{\text {th }}$ percentile queue lengths at the eastbound left turn approach at $124^{\text {th }}$ would reach 150 feet in the AM peak hour and 125 feet during the PM peak hour, which can be accommodated by the future design storage.

Additionally, the turn-bay storage for the westbound left turn onto the new access into the TCC site provides 250 feet of storage, significantly more than needed to accommodate the anticipated $95^{\text {th }}$ percentile queues of 100 feet in the AM peak hour and 75 feet in the PM peak hour.

As discussed in the findings on pages 14 and 15 of the KAI Memo, eastbound and westbound left turn queues will have adequate storage bays for anticipated peak hour queues under the 2025 (after construction) condition of T-S Rd. including the buildout of the TCC development.

## Performance

See the KAI Memo and KAl's Preliminary Findings Memorandum dated April 2019, a part of the KAI Memo, for further detail regarding the performance of the affected intersections upon completion of the industrial park project and the extension of the southern leg of Cipole into the site, terminating in a cul-de-sac. As noted in the KAI documentation, the proposed Cipole intersection and $124^{\text {th }}$ intersections are projected to operate acceptably under both 2021 and 2025 conditions.

## Conclusion

As demonstrated in this letter and attached materials, the proposed Design Exception Request satisfies the approval criteria of DCS Section 220.020.1 and will not result in any safety or operational performance deficiencies on the adjacent and regional transportation system. The requested design exception is also consistent with the long-term vision of the City's TSP and associated TEA Implementation Study. Therefore, we respectfully request your approval of this

Mr. Stacy Shetler
Washington County
November 25, 2019
Page 6 of 6
request. If you have any further questions or need any further supplemental information, please do not hesitate to contact me at 971-280-8646 or at jshoemaker@dowl.com

Sincerely,
DOWL
fll/lJlC
Jeff Shoemaker, P.E.
Senior Project Manager

Attachment(s): As stated
cc: Kirk Olsen, Trammell Crow Company


EXPIRATION DATE: 06/30/2020

DESIGN DRIVEN I CLIENT FOCUSED

## August 19, 2019 (Revised August 29, 2019)

Trammell Crow Company
Attention: Kirk Olsen
1300 SW 5th Avenue, Suite 3050
Portland, OR 97201
Re: TCC - Sherwood Corporate Park
LTR-Kirk Olsen
Project Number: $\mathbf{2 1 8 0 4 5 9 . 0 0}$
Dear Kirk:
At your request, Mackenzie has reviewed prior transportation planning documents and the current Sherwood Industrial Park site plan concept in relation to a potential extension of Cipole Road south of Tualatin-Sherwood Road to the future Blake Street.

## BACKGROUND

The Sherwood Industrial Park development is proposed at the southwest corner of Tualatin-Sherwood Road and SW 124th Avenue. Both roadways are classified as Washington County Arterials, which are intended to serve through traffic and have limited access to adjacent land uses. The future Blake Street to the south is classified as a collector roadway and parallel to Tualatin-Sherwood Road to provide access to adjacent parcels.

Washington County does not typically allow private driveways at signalized intersections, so a public cul-de-sac has been proposed to allow access to Tualatin-Sherwood Road at the existing signalized intersection with Cipole Road.

The site plan with the proposed cul-de-sac is enclosed.

## TRANSPORTATION PLANNING

A number of documents have been prepared over the years as development has been considered for the area. A concept plan for the Tonquin employment area was initially prepared in 2010 which provided an overview of general roadway alignments and development potential of the area. The current City of Sherwood Transportation Plan (TSP) from 2014 shows Local Street Connectivity in Figure 18, with "proposed roadway" alignments shown with a dashed line and arrows identifying "conceptual street connection" locations. The south approach of the Cipole Road intersection with TualatinSherwood Road is shown with an arrow, indicating it is a conceptual street connection, not a proposed roadway. Blake Street, as well as SW 124th Avenue, are shown with dashed lines, indicating they are proposed roadways. On page 59 of the TSP, local street connectivity as shown in Figure 18 is described as "It specifies the general location where new local streets could potentially be installed as nearby areas are developed or as the opportunity arises. The conceptual locations shown consider block length and access spacing requirements but do not necessarily reflect develop-ability due to topographic, environmental or manmade constraints. Locations identified are conceptual..." A copy of Figure 18 is enclosed.

P 503.224.9560 . F 503.228.1285 . W MCKNZE.COM • RiverEast Center, 1515 SE Water Avenue, \#100, Portland, OR 97214
architecture • interiors • structural engineering • civil engineering • land use planning - transportation planning . landscape architecture Portland, Oregon • Vancouver, Washington • Seattle, Washington

Trammell Crow Company
TCC - Sherwood Corporate Park
Project Number: 2180459.00
August 19, 2019 (Revised August 29, 2019)
Page 2
The Tonquin Employment Area (TEA) Plan prepared by Mackenzie in 2015 built off of the earlier documents and looked at development potential of the area in more detail, considering site layouts, circulation needs and physical constrains such as grades, wetlands, utilities and property boundaries. The plan presents recommended road alignments with the intent to serve all parcels and development sites with those roads. The arrows show anticipated access locations to serve each development node. An alignment of Blake Street is shown similar to the City's Plan and in conformance with Washington County's plan for an intersection on the new SW 124th Avenue alignment. It was not intended that additional roadways would be needed, nor access provided on Tualatin-Sherwood Road "except opposite the Cipole Road signalized intersection" as noted on page 26. Page 27 of the TEA document, a copy of which is attached and highlighted, notes "based on this update, we are assuming an internal driveway will be located here instead" of an extension of Cipole Road south of Tualatin-Sherwood Road. A copy of the relevant pages and Figure 17 from the TEA document are enclosed.

The area shown between Tualatin-Sherwood Road and the original Blake Street alignment was originally one large parcel/development site, which has since been divided for development of the subject project and the Willamette Water Supply facility. When the site was one large parcel, it was intended to simply provide access locations on both Blake Street and Tualatin Sherwood Road, due to the distance between roadways, significant grades across the site and to take advantage of the existing Cipole Road intersection. There was no need for a public street connection to serve the site.

## CURRENT PROPOSAL

City staff had originally agreed to the cul-de-sac concept for the site layout with no other driveways on public streets. As noted above, access to both SW 124th Avenue and Tualatin-Sherwood Road is limited, and a public street connection at Cipole Road would provide a protected access location for the site. It is understood the cul-de-sac needs a variance from the 250 ft minimum standard and Washington County must still approve the new approach to the signal.

The current cul-de-sac design results in about a $4 \%$ grade south of Tualatin-Sherwood Road. Extension of the road as a public street south to Blake Street would not only require additional ROW, but would necessitate slope easements and/or retaining walls, both of which will severely reduce the available development area for a site already impacted by grades and wetlands. The new location of Blake Street makes providing a public street connection difficult due to the grades, especially for an industrial use. A concept alignment has been prepared that shows grades of about $12 \%$, which is acceptable for residential development, but not the intended industrial use of the zone. The proposed Blake alignment is currently about 20 feet above the finished floor elevation of Building E, a distance of approximately 20-30 ft to the fire access road around the building.

Trammell Crow Company
TCC - Sherwood Corporate Park
Project Number: 2180459.00
August 19, 2019 (Revised August 29, 2019)
Page 3
In summary, the City's Transportation Plan does not dictate a public street connection, and more recent and thorough planning for the area in the Tonquin Employment Area specifically notes a public street connection is not needed. Further, there is no benefit or need for a public street connection through a site. Even if a public street had been envisioned at one point, the new location of Blake Street makes providing such a connection unnecessary and impractical.

Sincerely,


Brent Ahrend, PE
Associate Principal | Traffic Engineer

Enclosures: Sherwood Industrial Park Site Plan
City of Sherwood TSP - Figure 18 Local Street Connectivity
Tonquin Employment Area - Pages 26-27, Figure 17 Conceptual Road Layout
Cipole Road Concept Plan and Profile
Willamette Water Supply Plan
Partition Plat
c: Gabriela Frask, Scott Moore - Mackenzie



SHERWOOD INDUSTRIAL PARK - OPTION 1

Employment Industrial Zone
Following adoption of the 2010 Preferred Concept Plan, the site was designated Employment Industrial (EI) in the Comprehensive Plan and a new El zone was incorporated into the Development Code. Properties within the TEA that annex into the City would be zoned EI. The EI zone was created specifically for the Tonquin Employment Area to
 Areas and with the vision outlined in the Concept Plan.
The EI zone is intended to complement the City's EOA by targeting preferred industry sectors including Clean Technology, Technology and Advanced Manufacturing, and Outdoor Gear and Active Wear. The permitted uses within the El zone are more restrictive than the uses allowed in the City's Light Industrial or General Industrial zones. Furthermore, to provide sufficient space for the target industries, the El zone requires new sites to have a minimum


 rather than near Oregon Street or 124th Avenue.

## Transportation Review

[^10]
## Tualatin-Sherwood Road

This County Arterial is currently three lanes wide adjacent to the site. Widening is anticipated to a five-lane section in the near future, but no funds are currently identified. We have assumed no driveway access will be allowed for development in the TEA except opposite the Cipole Road signalized intersection, as all development areas would have access to lower classification roadways.

## 124th Avenue

## Blake Street

This road, which is identified as a need in the 2010 TEA Concept Plan, would serve as an east-west collector through the area, providing an alternate to Tualatin-Sherwood Road between 124th Avenue and Oregon Street in Sherwood

 the wetland and cross the power line easements perpendicularly. From that point, the road would turn 90 degrees along the west side of the power line easements to a roundabout intersection with Oregon Street. At the 90 degree bend, future extensions to the south and west could be accommodated.

## Tonquin Road

This two-lane County arterial does not have bike lanes or sidewalks, and is not currently planned for improvements as it is primarily outside the city limits. No access is proposed to Tonquin Road for the TEA as it is located at the bottom of a steep slope.
Oregon Street
This roadway is classified as a three-lane arterial and is built to its planned width. Sidewalks do not exist for most of the south frontage and will need to be provided with development.

## Local Street Connections

Tri-Met serves downtown Sherwood with routes 12 and 94. TriMet's Southwest Service Enhancement Plan is anticipated to provide service along Tualatin-Sherwood Road and 124th Avenue.

## Access Spacing Standards

> The following spacing standards generally apply to new driveway and roadway access points:

- Local streets -10 feet from the point of curvature or 25 feet if no radius exists
- Neighborhood routes - 50 feet
- Collectors - 100 feet
- Arterials - 600 feet


## Transit Service

Additional access restrictions apply to Tualatin-Sherwood Road (which would prohibit new driveways except
opposite Cipole Road) and 124th Avenue (which would prohibit all driveways and only allow access at Blake Road).
Infrastructure Review
The consultant team reviewed documentation of the existing infrastructure conditions, as well as proposed improvements for water distribution, sewer collection and treatment, and storm drainage systems. In addition to location and sizes of the proposed improvements, the team reviewed the assumptions used to determine the
 system corridor alignments. The Development Nodes and phases referenced in the discussion below are illustrated on Figure 18 in Chapter 5.

| SHERWOOD TONQUIN |  |
| :---: | :---: |
| EMPLOYMENT AREA |  |
| AND SW TUALATIN |  |
| CONCEPTUAL |  |
| ROAD LAYOUT |  |
| Washington County, OR |  |
| FIGURE 17 |  |
| LEGEND: |  |
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| development noie | A |




MACKENZIE.


CIPOLE RD CONNECTION TO BLAKE RD TEST FIT (1 OF 2)






Date:

Project:
Subject:

To: Kirk Olsen - Trammel Crow Company

From: Brian J. Dunn, PE, Kristine Connolly, PE, \& Claire Dougherty
November 15, 2019

Sherwood Industrial Park
Support Memorandum for Washington County DLUT Design Exception

This memorandum supplements our comprehensive transportation impact analysis documented in the April 2019 Preliminary Findings Memorandum (See Appendix " A ") for the proposed Sherwood Industrial Park development located in the southwest quadrant of the SW Tualatin-Sherwood Road and SW $124^{\text {th }}$ Avenue intersection in Sherwood, Oregon. It is intended to serve as the technical analysis necessary to support an exception to Washington County's access standards to allow a local access on the south side of the signalized SW Tualatin-Sherwood Road/SW Cipole Road intersection, which is under the jurisdiction of the Washington County Department of Land Use \& Transportation (DLUT).

As summarized herein, the requested design exception can be approved as it will not result in adverse intersection operation or queuing-related impacts at the following 3 key study intersections immediately adjacent to the site:

- SW Tualatin-Sherwood Road / SW Cipole Road (site access);
- SW Tualatin-Sherwood Road / SW $124^{\text {th }}$ Avenue; and,
- SW Blake Road / SW $124^{\text {th }}$ Avenue (future intersection).

A final comprehensive Transportation Impact Analysis (TIA) report, inclusive of all study intersections identified through a scoping process with City of Sherwood staff in December 2018, will be provided at a later time to support a site development application.

## PROPOSED DEVELOPMENT

The Applicant, Trammell Crow Company, is in the process of preparing an application to develop up to 547,200 square feet of industrial buildings on the subject property. The site is currently vacant and is bordered by the recent extension of SW $124^{\text {th }}$ Avenue to the east, SW Tualatin-Sherwood Road to the north, future industrial land uses to the west and a future east-west collector, Blake Road, to the south. A site vicinity map is shown on Figure 1, with two alternative site plan exhibits provided in the Preliminary Findings Memorandum. As shown in those exhibits, no site access is envisioned along the SW $124^{\text {th }}$

Avenue site frontage. Rather, access to the site is proposed at a single location on the south side of SW Tualatin Sherwood Road, at the signalized SW Cipole Road intersection.

## Need for Design Exception Request

SW Tualatin-Sherwood Road is under the jurisdiction of the Washington County Department of Land Use and Transportation (DLUT), which has access permitting authority. According Washington County's Functional Classification System Map (Reference 1), SW Tualatin-Sherwood Road is designated as an Arterial roadway. Therefore, a new access to this roadway must conform to Washington County Community Development Code (CDC, Reference 2) Section 501-8.5(B)(4), which states:
"Direct access to arterial roads shall be from collector or other arterial streets. Exceptions for local streets and private accesses may be allowed through a Type II process when collector access is found to be unavailable and impracticable by the Director. New Arterial Street alignments identified in the TSP may be adjusted within the subject property, as approved by the County Engineer."

Based on the CDC standards above, a design exception is necessary, as the proposed site access to SW Tualatin-Sherwood Road is not from an Arterial or Collector street, but by a local access connection that serves only localized traffic associated with this development. As described in this memorandum, the requested design exception is expected to result in no adverse impacts to intersection operations, safety, and vehicle queuing conditions along the SW Tualatin-Sherwood Road corridor. Therefore, it is our professional opinion that Washington County DLUT should grant the requested design exception and allow a local access to SW Tualatin-Sherwood Road.

The following are additional supportive reasons to justify the requested design exception:

1. Access to SW Tualatin-Sherwood Road via a public street designed to an Arterial or Collector standards is "unavailable" because higher-functioning roadways of these classifications are not envisioned either in the adopted City of Sherwood Transportation System Plan (TSP, Reference 3) or in the adopted Tonquin Employment Area (TEA) Implementation Plan (Reference 4). Both studies identify only a localized access to SW Tualatin-Sherwood Road across from SW Cipole Road.

- Figure 17 of the Sherwood TSP does not contemplate or designate a public road access into the site.
- See Conceptual Road Layout (Figure 17) of the TEA Implementation Plan.
- This figure identifies an unclassified access point for the subject property (Development Node E) to Tualatin-Sherwood Road at the SW Cipole Rd intersection. No road of any type is contemplated connecting the subject property to Tualatin-Sherwood Road.
- Page 27 of the TEA Implementation Plan states that the property's access point is assumed to be an "internal drive."

2. Access to SW Tualatin-Sherwood Road via public Arterial or Collector is also "impracticable" because a roadway of either classification would need to be continuous, of limited curvature, and connect through the site along steep grades in order to connect with the future Blake Road collector street. If this connection were made, it has been demonstrated by the applicant's civil engineer (DOWL) that roadway grading would reach approximately $11.7 \%$, which is not conducive for truck travel on the roadway or to/from the future industrial uses planned for this site.
3. If a continuous street connection built to Collector or Arterial standards were to be constructed through the site, it could elicit undesirable vehicular cut-through patterns through the industrial park complex. SW $124^{\text {th }}$ Avenue should be the primary north-south arterial in the immediate area.


KITTELSON
\& ASSOCIATES

## TRAFFIC ANALYSIS SCOPE AND METHODOLOGY

As the purpose of this memorandum is to inform the design exception process with Washington County, Kittelson has performed additional technical analyses beyond what is already provided in the attached Preliminary Findings Assessment, with a revised focus on the following scenarios:

- Existing Year 2019 Traffic Conditions
- No site development with SW Tualatin-Sherwood Road in current condition as 3-lane cross section
- Year 2021 Total Traffic Conditions
- SW Cipole Road as a local access cul-de-sac and SW Tualatin-Sherwood Road remains as 3-lane cross section
- SW Cipole Road as a local access extension to Blake Road and SW Tualatin-Sherwood Road remains as 3-lane cross section
- Year 2025 Total Traffic Conditions
- SW Cipole Road as a local access cul-de-sac and SW Tualatin-Sherwood Road is widened to 5 -lane cross section
- SW Cipole Road as a local access extension to Blake Road and SW Tualatin-Sherwood Road is widened to 5 -lane cross section

It should be emphasized that the existing and future intersection operation and queueing analyses in this memorandum are different than the results documented in the Preliminary Findings Memorandum as they reflect the following recent updates:

- Signal timing changes were recently implemented during the PM peak hour at the SW TualatinSherwood Road/SW Cipole Road and SW Tualatin-Sherwood Road/SW 124 ${ }^{\text {th }}$ Avenue intersections, such that both intersections now operate as fully-actuated, uncoordinated signals, with AutoMax enabled during the PM peak hour, and,
- Revisions to the SW Tualatin Sherwood Road widening project now indicate a build-out year of 2025, with dual left-turn lanes planned at the SW $124^{\text {th }}$ Avenue intersection, as published on the Washington County project website in August 2019.

The methodologies and assumptions used to update the existing and future traffic conditions at the 3 adjacent study intersections integral to the design exception discussion are consistent with those documented in the attached Preliminary Findings Memorandum.

## EXISTING TRAFFIC CONDITIONS

Table 1 summarizes the updated operational analysis for the existing study intersections under current traffic conditions for the weekday AM and PM peak hours, considering the recent signal timing changes. As shown, the two existing intersections along SW Tualatin-Sherwood Road currently operate at acceptable levels and meet the mobility standards of the governing agency. However, as observed in the field, and reported within the queuing outputs in the Synchro worksheets, vehicle queueing is prevalent east-west along the SW Tualatin-Sherwood Road corridor during both AM and PM peak hours.

Appendix " $B$ " contains the year 2019 existing traffic level-of-service and queuing worksheets.
Table 1: Existing Conditions Operational Analysis Results

| \# | Intersection | LOS $^{1}$ |  | $\mathrm{v} / \mathrm{c}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operational Standard | Standard Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 4 | SW Cipole Road/SW TualatinSherwood Road | A (7.3) | B (15.0) | 0.67 | 0.82 | Regional | V/C of 0.99 | Yes |
| 5 | SW $124^{\text {th }}$ Avenue/SW TualatinSherwood Road | D (35.9) | C (27.7) | 0.88 | 0.71 | Regional | V/C of 0.99 | Yes |
| 11 | SW 124 ${ }^{\text {th }}$ Avenue/Blake Road | N/A | N/A | N/A | N/A | N/A | N/A | N/A |

${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or critical movement delay at two-way stop control (TWSC).
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio (signalized). For TWSC intersections, the critical movement is shown;
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan.
N/A = Not applicable. Intersection does not yet exist.

## YEAR 2021 BACKGROUND TRAFFIC CONDITIONS

The year 2021 background traffic conditions analysis assumes the same traffic attributed to general growth in the region (application of a 1.5 percent annual growth rate), and in-process trips as documented in Preliminary Findings Memorandum. Additionally, it was still assumed that Blake Road would be in place by 2021 from SW Oregon Street to SW 124th Avenue, with some re-distribution of trips at the SW Oregon Street/SW Tualatin-Sherwood Road and SW 124th Avenue/SW Tualatin-Sherwood Road intersections.

Table 2 summarizes the updated operational analysis for the study intersections under the weekday AM and PM peak hour background 2021 traffic conditions, considering the recent PM signal timing changes. As indicated in Table 2, all study intersections are forecast to operate at levels which meet the mobility standards of the governing agency during both weekday AM and PM peak hours.

Appendix "C" contains the year 2021 background traffic level-of-service worksheets.

Table 2: Year 2021 Background Conditions Operational Analysis Results

| \# | Intersection | LOS $^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operating <br> Standard | Standard Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 4 | SW Cipole Road/SW TualatinSherwood Road | A (8.2) | B (19.4) | 0.71 | 0.89 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 5 | SW $124^{\text {th }}$ Avenue/SW TualatinSherwood Road | E (58.1) | C (34.6) | 0.98 | 0.79 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 11 | SW $124{ }^{\text {th }}$ Avenue/Blake Road | B (12.2) | B (11.2) | 0.05 (EB) | 0.04 (EB) | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |

${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or critical movement delay (TWSC).
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio (signalized). For TWSC intersections, the critical movement is shown;
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan.

## YEAR 2021 TOTAL TRAFFIC ANALYSIS

The year 2021 total traffic conditions analysis results are shown below for the two alternative site development and access scenarios. Both scenarios continue to assume that Blake Road would be in place from SW Oregon Street to SW $124^{\text {th }}$ Avenue, with limited re-distribution of system trips from the SW Oregon Street/SW Tualatin-Sherwood Road and SW 124 ${ }^{\text {th }}$ Avenue/SW Tualatin-Sherwood Road intersections. Consistent with the Preliminary Findings Memorandum, the total traffic scenarios assume full build-out of the 547,220 SF Industrial Park.

## SW Cipole Road Cul-De-Sac

Updated year 2021 total traffic conditions are presented in Table 3 for the scenario in which SW Cipole Road is a local access cul-de-sac street. Appendix "D" contains the updated year 2021 Total Traffic Cul-de-sac Site Plan level-of-service worksheets.

Table 3: Year 2021 Total Traffic Conditions - SW Cipole Road Cul-De-Sac Operational Analysis Results

| \# | Intersection | LOS $^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operating <br> Standard | Standard Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 4 | SW Cipole Road/SW TualatinSherwood Road | B (14.7) | C (33.3) | 0.81 | 0.92 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 5 | SW $124^{\text {th }}$ Avenue/SW TualatinSherwood Road | E (57.5) | D (35.6) | 0.99 | 0.81 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 11 | SW $124^{\text {th }}$ Avenue/Blake Road | B (12.4) | B (11.4) | 0.05(EB) | 0.02 (EB) | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 12 | SW Cipole Road/Blake Road | Not Applicable to SW Cipole Road Cul-de-sac Scenario |  |  |  |  |  |  |

${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or critical movement delay (TWSC).
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio (signalized). For TWSC intersections, the critical movement is shown;
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan.

## SW Cipole Road Extension to Blake Road

Updated year 2021 total traffic conditions are presented in Table 4 for the scenario in which SW Cipole Road extends to Blake Road as a local access street. Appendix " $E$ " contains the updated year 2021 Total Traffic - SW Cipole Road Extension Site Plan level-of-service worksheets.

Table 4: Year 2021 Total Traffic Conditions - SW Cipole Road Extension Operational Analysis Results

| \# | Intersection | LOS $^{1}$ |  | $v / C^{2}$ |  | Jurisdiction ${ }^{3}$ | Operating <br> Standard | Standard <br> Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 4 | SW Cipole Road/SW TualatinSherwood Road | B (13.2) | C (26.0) | 0.78 | 0.90 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 5 | SW $124^{\text {th }}$ Avenue/SW TualatinSherwood Road | E (58.3) | D (36.2) | 0.99 | 0.81 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 11 | SW $124{ }^{\text {th }}$ Avenue/Blake Road | B (12.7) | B (11.5) | 0.06 (EB) | 0.05 (EB) | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 12 | SW Cipole Road/Blake Road | A (9.1) | B (9.2) | 0.02 (SB) | 0.07 (SB) | City of Sherwood | $\begin{gathered} \text { LOS "E" } \\ \text { or V/C of } \\ 0.90 \end{gathered}$ | Yes |

${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or critical movement delay (TWSC).
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio (signalized). For TWSC intersections, the critical movement is shown;
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan.
As indicated in Tables 3 and 4, the adjacent study intersections around the site are anticipated to meet the applicable regional or City of Sherwood operating standards with site development, considering both scenarios for the termination of SW Cipole Road as a local access cul-de-sac street and the potential SW Cipole Road extension to Blake Road.

## YEAR 2025 BACKGROUND TRAFFIC CONDITIONS

The year 2025 background traffic conditions analysis identifies how the study area's transportation system will operate without the proposed development. Similar to the year 2021 background analysis, the year 2025 analysis includes trips from traffic attributed to general growth in the region (application of a 1.5 percent annual growth rate), trips from the in-process and some re-distribution of trips, assuming the connection of Blake Road from SW Oregon Street to SW $124^{\text {th }}$ Avenue.

Additionally, the 2025 background analysis includes the planned widening of SW Tualatin-Sherwood Road to five lanes, as defined by Project \#318 in the Washington County MSTIP 3e (Reference 9). Volumes on SW Tualatin-Sherwood Road and SW $124^{\text {th }}$ Avenue were increased an additional 5 percent on top of regional growth, to account for increased future demand.

Assumed lane configurations for SW Tualatin Sherwood Road signalized intersections to be widened as part of the planned project were updated to match the preliminary design layouts posted on the Washington County website for Project \#318 in August 2019 (Reference 10). Additionally, as dual leftturn lanes are now planned for the eastbound, westbound and northbound approaches to the SW $124^{\text {th }}$ Avenue intersection, it was assumed that these movements would become controlled by protected-only left turn phasing.

Table 5 summarizes the operational analysis for the study intersections under the weekday AM and PM peak hour 2025 background traffic conditions and shows that all study intersections are forecast to operate at levels which meet the mobility standards of the governing agency during both weekday AM and PM peak hours. Appendix "F" contains the year 2025 background traffic level-of-service worksheets.

Table 5: Year 2025 Background Conditions Operational Analysis Results

| \# | Intersection | LOS $^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operating <br> Standard | Standard Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 4 | SW Cipole Road/SW TualatinSherwood Road | A (5.6) | A (9.5) | 0.43 | 0.62 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 5 | SW $124^{\text {th }}$ Avenue/SW TualatinSherwood Road | C (32.0) | C (23.6) | 0.64 | 0.60 | Regional | $\begin{gathered} \mathrm{V} / \mathrm{C} \text { of } \\ 0.99 \end{gathered}$ | Yes |
| 11 | SW $124{ }^{\text {th }}$ Avenue/Blake Road | B (12.7) | B (11.6) | $\begin{aligned} & 0.05 \\ & \text { (EB) } \end{aligned}$ | 0.02 (EB) | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |

${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or critical movement delay (TWSC).
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio (signalized). For TWSC intersections, the critical movement is shown;
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan.

## YEAR 2025 TOTAL TRAFFIC CONDITIONS

The year 2025 total traffic conditions analysis results are shown below for the two alternative site access scenarios. Both scenarios continue to assume that Blake Road would be in place from SW Oregon Street to SW $124^{\text {th }}$ Avenue, and assume the planned widening of SW Tualatin-Sherwood Road and associated intersection modifications. Consistent with the Preliminary Findings Memorandum, the total traffic scenarios assume full build-out of the 547,220 SF Industrial Park.

## SW Cipole Road Cul-De-Sac

Updated year 2025 total traffic conditions are reported in Table 6 for the scenario in which SW Cipole Road is a local access cul-de-sac street. Appendix " $G$ " contains the year 2025 Total Traffic Cul-de-sac Site Plan level-of-service worksheets.

Table 6: Year 2025 Total Traffic Conditions - SW Cipole Road Cul-De-Sac Operational Analysis Results

| \# | Intersection | LOS ${ }^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operating <br> Standard | Standard Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 4 | SW Cipole Road/SW TualatinSherwood Road | A (9.5) | B (14.3) | 0.50 | 0.62 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 5 | SW $124^{\text {th }}$ Avenue/SW TualatinSherwood Road | C (29.8) | C (24.1) | 0.65 | 0.61 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 11 | SW $124^{\text {th }}$ Avenue/Blake Road | B (12.9) | B (11.7) | 0.05 (EB) | 0.02 (EB) | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 12 | SW Cipole Road/Blake Road | Not Applicable to Cipole Road Cul-de-sac Scenario |  |  |  |  |  |  |

[^11]
## SW Cipole Road Extension to Blake Road

Updated year 2025 total traffic conditions are presented in Table 7 for the scenario in which SW Cipole Road extends to Blake Road as a local access street. Appendix "H" contains the year 2025 Total Traffic Cipole Road Extension Site Plan level-of-service worksheets.

Table 7: Year 2025 Total Traffic Conditions - SW Cipole Road Extension Operational Analysis Results

| \# | Intersection | LOS $^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operating <br> Standard | Standard Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 4 | SW Cipole Road/SW TualatinSherwood Road | A (8.6) | B (12.5) | 0.48 | 0.60 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 5 | SW $124^{\text {th }}$ Avenue/SW TualatinSherwood Road | C (29.6) | C (24.1) | 0.65 | 0.61 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 11 | SW 124 ${ }^{\text {th }}$ Avenue/Blake Road | B (13.2) | B (11.9) | 0.06 (EB) | 0.05 (EB) | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 12 | SW Cipole Road/Blake Road | A (9.1) | A (9.2) | 0.02 (SB) | 0.07 (SB) | City of Sherwood | $\begin{aligned} & \text { LOS "E" } \\ & \text { or v/c of } \\ & 0.90 \end{aligned}$ | Yes |

${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or critical movement delay (TWSC.
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio (signalized). For TWSC intersections, the critical movement is shown;
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan.
As indicated in Tables 6 and 7, the three or four site-adjacent study intersections are anticipated to meet the applicable regional or City of Sherwood operating standards with the site development place, considering both the termination of SW Cipole Road as a cul-de-sac and the potential SW Cipole Road extension to Blake Road.

## VEHICLE QUEUING ANALYSIS

An updated $95^{\text {th }}$-percentile vehicle queuing analysis was completed for two site access scenarios under future build-out years 2021 and 2025. Consistent with the methodology applied in the Preliminary Assessment Report, for the SimTraffic analysis, four 15-minute periods were recorded, with the second period representative of the peak 15-minute period, with the report results averaging five runs. Appendix "I" contains the updated Year 2021 Total Traffic SimTraffic worksheets and Appendix "J" contains the Year 2025 Total Traffic Simtraffic worksheets.

As shown in Tables 8, under year 2021 total traffic conditions, most $95^{\text {th }}$ percentile queues can generally be accommodated by the existing or assumed lane storage capacities, considering both site access scenarios. In the few instances where demand in the striped turn bay storage is exceeded, as measured by the length of the white gore stripe, additional queue storage is available in the adjacent striped median or two-way left-turn lane (TWLTL) area, with the exception of the eastbound right-turn lane at the SW Tualatin Sherwood Road/SW $124^{\text {th }}$ Avenue intersection during the AM peak hour. The eastbound rightturn lane $95^{\text {th }}$ percentile queue is estimated 400 feet, whereas the striped turn bay storage, as measured by the length of the white gore stripe, is 350 feet. Inclusive of the taper length, there is adequate storage to accommodate a 400-foot-long queue before potentially impacting the adjacent bike lane or eastbound through lane. Additionally, eastbound SW Tualatin-Sherwood Road through lane queues may extend to
adjacent intersections during the AM peak hour and westbound through lane queues may extend to adjacent intersections during the PM peak hour.

Table 8: Year 2021 Total Traffic Conditions - SimTraffic $95^{\text {th }}$ Percentile Queue Summary

| Intersection | Scenario |  | Eastbound |  |  | Westbound |  |  | Northbound |  |  | Southbound |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right |
| SW Cipole <br> Road / <br> SW <br> Tualatin- <br> Sherwood <br> Road |  | Storage (feet) | $360^{1}$ | 1100 | - | 250 | 790 | 125 | 200 | 200 | - | 300 | 725 | - |
|  | SW Cipole <br> Road Cul-de-Sac | AM <br> Queue | 325 | 1225 | - | 175 | 225 | 75 | 75 | 75 | - | 250 | 225 | - |
|  |  | PM <br> Queue | 125 | 600 | - | 150 | 775 | 50 | 150 | 125 | - | 125 | 150 |  |
|  | SW Cipole <br> Road <br> Extension | AM Queue | 375 | 1225 | - | 125 | 175 | 100 | 75 | 75 | - | 175 | 75 | - |
|  |  | PM <br> Queue | 100 | 400 | - | 75 | 653 | 75 | 125 | 100 | - | 100 | 150 | - |
| SW $124^{\text {th }}$ <br> Avenue / <br> SW <br> Tualatin- <br> Sherwood <br> Road |  | Storage (feet) | $360^{1}$ | 790 | 350 | 375 | 1180 | 375 | 460 | 1000 | - | $240^{3}$ | 730 | - |
|  | SW Cipole <br> Road Cul-de-Sac | $\begin{gathered} \text { AM } \\ \text { Queue } \end{gathered}$ | 300 | 975 | 275 | 125 | 600 | 300 | 325 | 600 | - | 325 | 325 | - |
|  |  | PM <br> Queue | 125 | 750 | 400 | 275 | 1225 | 375 | 175 | 200 | - | 325 | 350 | - |
|  | SW Cipole <br> Road <br> Extension | AM <br> Queue | 400 | 975 | - | 175 | 475 | 350 | 350 | 525 | - | 275 | 200 | - |
|  |  | PM <br> Queue | 125 | 575 | 275 | 300 | 1000 | 350 | 150 | 200 | - | 275 | 275 | - |
| SW 124th <br> Avenue / <br> Blake Road |  | Storage (feet) ${ }^{4}$ | 150 | 800 | - | 150 | - | - | 150 | 1000 | - | 150 | 1000 | - |
|  | SW Cipole Road Cul-de-Sac | AM <br> Queue | 50 | 75 | - | 25 | - | - | - | 75 | - | 25 | - | - |
|  |  | PM Queue | 50 | 50 | - | 50 | - | - | - | 25 | - | - | - | - |
|  | SW Cipole <br> Road <br> Extension | AM Queue | 50 | 75 | - | 25 | - | - | - | - | - | 25 | - | - |
|  |  | PM Queue | 50 | 50 | - | 75 | - | - | 25 | 25 | - | 25 | - | - |

Notes:
$95^{\text {th }}$ percentile queue lengths are reported in feet and have been rounded up to the nearest car length, assuming one vehicle equals 25 feet;
Bold and highlighted cells indicate $95^{\text {th }}$ percentile queue lengths greater than the storage length;
${ }^{1}$ Storage measured as the length of white gore stripe for turn lane, additional queue storage available in striped median;
${ }^{2}$ Storage for future intersection left-turn lanes assumed to be 150 feet;
${ }^{3}$ Storage measured as the length of white gore stripe for turn lane, additional queue storage available in left-most southbound through lane, as only the right southbound through lane continues through the intersection.
${ }^{4}$ Storage for future intersection eastbound left-turn lanes assumed to be 150 feet.
As detailed in Table 9, under year 2025 total traffic conditions, all $95^{\text {th }}$ percentile queues can be accommodated by the planned lane configuration storage capacity, considering both site access scenarios, except for the southbound left-turn movement at the SW Tualatin Sherwood Road/SW 124 ${ }^{\text {th }}$ Avenue intersection during the AM peak hour.

Table 9: Year 2025 Total Traffic Conditions AM Peak Hour - SimTraffic 95 ${ }^{\text {th }}$ Percentile Queue Summary

| Intersection | Scenario |  | Eastbound |  |  | Westbound |  |  | Northbound |  |  | Southbound |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right |
| SW Cipole <br> Road / <br> SW <br> Tualatin- <br> Sherwood <br> Road |  | Storage (feet) | 360 | 1100 | - | 250 | 790 | - | 200 | 200 | - | 300 | 725 | - |
|  | SW Cipole Road Cul-de-Sac | AM Queue | 100 | 175 | - | 100 | 175 | - | 75 | 50 | - | 150 | - | - |
|  |  | PM Queue | 75 | 150 | - | 50 | - | - | 125 | 75 | - | 100 | - | - |
|  | SW Cipole <br> Road <br> Extension | AM Queue | 100 | 175 | - | 100 | 175 |  | 50 | 50 | - | 50 | 50 | - |
|  |  | PM Queue | 50 | 150 | - | 75 | 225 | - | 100 | 75 | - | 100 | 100 | - |
| SW $124^{\text {th }}$ <br> Avenue / SW <br> TualatinSherwood <br> Road |  | Storage (feet) | $250^{1}$ | 790 | 350 | $375{ }^{1}$ | 1180 | 375 | $300{ }^{1}$ | 1000 | - | 240 | 730 | - |
|  | SW Cipole Road Cul-de-Sac | AM Queue | 150 | 350 | 150 | 100 | 275 | 150 | 175 | 225 | - | 325 | - | - |
|  |  | PM Queue | 100 | 300 | 125 | 75 | 175 | 75 | 100 | 100 | - | 225 | - | - |
|  | SW Cipole <br> Road <br> Extension | AM Queue | 150 | 400 | 100 | 100 | 275 | 175 | 150 | 225 | - | 275 | 225 | - |
|  |  | PM Queue | 125 | 325 | 150 | 50 | 300 | 100 | 100 | 100 | - | 225 | 200 | - |
| SW 124th <br> Avenue / <br> Blake Road |  | Storage (feet) ${ }^{2}$ | 150 | 800 | - | 150 | - | - | 150 | 1000 | - | 150 | 1000 | - |
|  | SW Cipole Road Cul-de-Sac | AM Queue | 50 | 100 | - | 25 | - | - | - | 25 | - | 50 | - | - |
|  |  | PM Queue | 50 | 50 | - | 50 | - | - | - | 25 | - | - | - | - |
|  | SW Cipole <br> Road <br> Extension | AM Queue | 50 | 75 | - | 25 | - | - | 25 | - | - | 25 | - | - |
|  |  | PM Queue | 50 | 50 | - | 50 | - | - | 25 | - | - | 25 | - | - |

Notes:
$95^{\text {th }}$ percentile queue lengths are reported in feet and have been rounded up to the nearest car length, assuming one vehicle equals 25 feet; Bold and highlighted cells indicate $95^{\text {th }}$ percentile queue lengths greater than the storage length;
${ }^{1}$ Storage measured as the length of white gore stripe for turn lane, assuming dual left-turn lane as published on Washington County widening project website. Northbound dual left-storage capacity estimated, additional storage available in striped median to the south;
${ }^{2}$ Storage for future intersection left-turn lanes assumed to be 150 feet.

## Vehicle Queueing Impact Considerations

## Year 2021 Total Traffic Conditions - Back-to-Back Left-turn Lanes

Looking specifically at the potential eastbound and westbound back-to-back left-turn queue interaction along SW Tualatin-Sherwood Road between SW 124 ${ }^{\text {th }}$ Avenue and SW Cipole Road, the existing eastbound approach to the SW $124^{\text {th }}$ Avenue / SW Tualatin- Sherwood Road intersection includes 360 feet of turn bay storage and an approximately 150 -foot-long taper, as detailed in Figure 2 . The $95^{\text {th }}$ percentile eastbound left-turn movement peak hour queues in the worst-case scenario are estimated at 400 feet during the AM peak hour and 125 feet during the PM peak hour, both of which can be accommodated by the existing queue storage and taper space provided. Additionally, the westbound left-turn lane at the SW Cipole Road / SW Tualatin-Sherwood Road intersection is assumed to have turn bay storage length of approximately 250 feet. A turn lane of this length can accommodate the $95^{\text {th }}$
percentile queues estimated at 175 feet, approximately 7 vehicle lengths, during the AM peak hour and 150 feet, approximately 6 vehicles lengths, during the PM peak hour.

Figure 2. Existing Year 2019 SW 124 ${ }^{\text {th }}$ Ave Eastbound Left-Turn Lane Configuration


## Year 2025 Total Traffic Conditions - Back-to-Back Left-turn Lanes

Updated information available on the Washington County SW Tualatin-Sherwood Road Widening project now indicates the use of dual-left turn lanes on most approaches to the SW Tualatin-Sherwood Road / SW $124^{\text {th }}$ Avenue intersection, as reproduced below as Figure 3.

Figure 3. Future Year 2025 SW 124 ${ }^{\text {th }}$ Ave Eastbound Left-Turn Lane Configuration


With the widening of SW Tualatin-Sherwood Road, the northbound, eastbound and westbound intersection approaches are shown as converted to dual-left turn lanes. The eastbound approach includes 250 feet of dual left-turn bay storage and an approximately 190-foot-long taper. With this revised eastbound approach configuration, there is approximately 310 feet of space that is sufficient for the left-turn movement at the westbound approach to the SW Cipole Road / SW Tualatin-Sherwood Road intersection. The 95th percentile eastbound left-turn movement peak hour queues in the worst-case
scenario are estimated at 150 feet during the AM peak hour and 125 feet during the PM peak hour, both of which can be accommodated by the existing queue storage and taper space provided. Additionally, a future westbound left-turn lane at the SW Cipole Road / SW Tualatin-Sherwood Road intersection can have a single lane left-turn bay with storage of approximately 250 feet, which would accommodate the 95th percentile queues estimated at 100 feet (or approximately 4 vehicle lengths) during the AM peak hour and 75 feet (or approximately 3 vehicles lengths) during the PM peak hour.

## SUMMARY OF FINDINGS

The findings of our updated intersection operations and vehicle queueing analyses indicate there is no significant difference between impacts of the two site access options contemplated for this development. Our findings relative to impacts on intersection operations and vehicle queueing are as follows:

- The three study intersections adjacent to the site will operate during the weekday AM and PM peak hours at levels which meet the governing regional operating standard under all scenarios studied, including:
- Year 2019 Existing Traffic Conditions
- Year 2021 Background Traffic Conditions
- Year 2021 Total Traffic Conditions
- With SW Cipole Road as a local access cul-de-sac, or
- With SW Cipole Road as a local access street connecting to Blake Road.
- Year 2025 Background Traffic Conditions
- Year 2025 Total Traffic Conditions
- With SW Cipole Road as a local access cul-de-sac, or
- With SW Cipole Road as a local access street connecting to Blake Road.
- Under current traffic conditions, the eastbound left-turn lane at the SW Tualatin-Sherwood Road / SW $124^{\text {th }}$ Avenue intersection includes 360 feet of turn bay storage and an approximately 150-foot-long taper. The 275 feet of additional distance to the west before the SW Cipole Road intersection could be utilized as a single westbound left-turn lane to the planned extension of SW Cipole Road into the site without creating any operational or safety deficiencies.
- Estimated year 2021 total traffic queues within the back-to-back eastbound and westbound leftturn lanes along SW Tualatin-Sherwood Road, between SW Cipole Road and SW $124^{\text {th }}$ Avenue, can be adequately accommodated under either SW Cipole Road site access scenario without creating any operational or safety deficiencies.
- The planned widening along SW Tualatin-Sherwood Road by the year 2025 includes dual eastbound left-turn lanes on the approach to SW $124^{\text {th }}$ Avenue with 250 feet of striped storage and an approximately 190 -foot-long taper. This revised configuration would leave approximately 310 feet of additional space to the west available for a single left-turn lane on the westbound approach to the SW Cipole Road / SW 124th Avenue intersection.
- Estimated year 2025 total traffic queues within the back-to-back eastbound and westbound leftturn lanes along SW Tualatin-Sherwood Road, between SW Cipole Road and SW $124^{\text {th }}$ Avenue, can be adequately accommodated under either SW Cipole Road site access scenario without creating any operational or safety deficiencies.

Aside from the minor differences between technical findings for the two site access scenarios, it is our professional opinion that the termination of SW Cipole Road as a local access cul-de-sac would be the best option. Besides the design challenges associated with steep grades throughout the site and the need to facilitate safe truck movements, a cul-de-sac ending will also eliminate the potential for longterm cut-through traffic through the site, thus, establishing a finite limit to the queues that could result within the associated turn lanes at the SW Tualatin-Sherwood Road/SW Cipole Road intersection.

## CONCLUSIONS

In summary, the requested design exception associated with the proposed Sherwood Industrial Park can be approved by Washington County DLUT while maintaining acceptable levels of mobility and safety at the adjacent study intersections under either site access scenario at SW Cipole Road, assuming agencyplanned improvements are in place.

If you have any questions, please give us a call at (503) 228-5230.

## REFERENCES

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9) Washington County. MSTIP 3e Adopted Funding Program and Project List. 2016.
10) Washington County. Tualatin Sherwood Road (Teton Avenue to Langer Farms Parkway). https://www.co.washington.or.us/LUT/TransportationProjects/tualatinsherwoodroad.cfm

## ATTACHMENTS

Appendix A: Preliminary Findings Memorandum - April 2019
Appendix B: Year 2019 Existing Operations Worksheets
Appendix C: Year 2021 Background Operations Worksheets
Appendix D: Year 2021 Total Traffic - Cipole Road Cul-de-sac Operations Worksheets
Appendix E: Year 2021 Total Traffic - Cipole Road Extension Operations Worksheets
Appendix F: Year 2025 Background Operations Worksheets
Appendix G: Year 2025 Total Traffic - Cipole Road Cul-de-sac Operations Worksheets
Appendix H: Year 2025 Total Traffic - Cipole Road Extension Operations Worksheets
Appendix I: Year 2021 Total Traffic - SimTraffic Worksheets
Appendix J: Year 2025 Total Traffic - SimTraffic Worksheets

## Appendix A Preliminary <br> Findings <br> Memorandum <br> (April 2019)

## MEMORANDUM

Date:<br>April 19, 2019<br>To: Kirk Olsen - Trammel Crow Company<br>From: $\quad$ Brian J. Dunn, PE, Kristine Connolly, PE, \& Claire Dougherty<br>Project: Sherwood Industrial Park<br>Subject: $\quad$ Traffic Impact Study - Preliminary Findings Memorandum - Revision 1

Project \#: 23278

This memorandum presents the preliminary transportation impact analysis findings for the proposed Sherwood Industrial Park development, to be located the southwest quadrant of the SW TualatinSherwood Road and SW $124^{\text {th }}$ Avenue intersection in Sherwood, Oregon. This interim memorandum was revised in response to City comments on the memorandum dated March 15, 2019. It is provided to facilitate discussions regarding the operational impacts of site development to the surrounding street network under two scenarios:

- Limiting SW Cipole Road to a cul-de-sac ending within the site; and,
- Extension of SW Cipole Road through the site to Blake Road (future east-west collector).

A comparison of these two potential scenarios led to the following findings which support limiting SW Cipole Road to a cul-de-sac ending, rather than extending it through the site to Blake Road:

- Traffic Operations: Regardless of whether or not SW Cipole Road is extended through the site, the adjacent study intersections are all anticipated to meet the regional mobility standard. While the extension of SW Cipole Road results in slightly improved operations at the SW Cipole Road / SW Tualatin-Sherwood Road intersection, operations remain the same or slightly deteriorate at the SW $124^{\text {th }}$ Avenue / SW Tualatin-Sherwood Road, SW Cipole Road/Blake Road and SW $124^{\text {th }}$ Avenue / Blake Road intersections. Therefore, there appears to be no significant system-wide benefit to extending SW Cipole Road through the site to connect with the future Blake Road.
- Traffic Safety: A connection to Blake Road would add an access point to the roadway network, introducing conflict. Limiting SW Cipole Road to a cul-de-sac ending would result in fewer unprotected left-turn conflict points on the surrounding roadway network, especially those involving large trucks.

Based on the results of the preliminary transportation impact analysis detailed herein, and considering the planned widening of SW Tualatin-Sherwood Road, the proposed Sherwood Industrial Park can be developed while maintaining acceptable levels of mobility at the study intersections, with the exception
of the SW Oregon Street / SW Tonquin Road intersection. This two-way stop-controlled intersection is anticipated to exceed the regional operating standard during the PM peak hour with site development, regardless of whether or not SW Cipole Road is extended through the site. The SW Oregon Street / SW Tonquin Road intersection can meet the regional operating standards with either a signal or a roundabout installed.

## INTRODUCTION

## Proposed Development

The Applicant, Trammell Crow Company, is in the process of preparing an application to develop 547,220 square feet of industrial buildings on the subject property. The site is currently vacant and is bordered by the recent extension of SW $124^{\text {th }}$ Avenue to the east, SW Tualatin-Sherwood Road to the north, future industrial land uses to the west and a future east-west collector, Blake Road, to the south.

Figure 1 displays a site vicinity map and Figures 2 and 3 display two site plan alternatives. The site plan as shown in Figure 2 details a possible extension of Cipole Road into the site terminating as a private cul-de-sac, whereas Figure 3 shows Cipole Road bisecting the site as a public street, extending to intersect with the future Blake Road. As shown in both site plans, no site access driveways are planned on SW $124^{\text {th }}$ Avenue.




Scope of Study
The following study intersections were identified in a scoping memorandum submitted to the City of Sherwood and Washington County Department of Land Use and Transportation (DLUT) for review:

- SW Tualatin-Sherwood Road/SW Oregon Street;
- SW Tualatin-Sherwood Road/SW Wildrose Place;
- SW Tualatin-Sherwood Road/SW Cipole Road;
- SW Tualatin-Sherwood Road/SW 124 ${ }^{\text {th }}$ Avenue;
- SW Tualatin-Sherwood Road/SW 120 ${ }^{\text {th }}$ Avenue;
- SW Tualatin-Sherwood Road/SW 115 ${ }^{\text {th }}$ Avenue; and,
- SW Tualatin-Sherwood Road/SW $112{ }^{\text {th }}$ Avenue-SW Avery Street.

After further discussions with the City of Sherwood, the following intersections were added for analysis:

- SW Tualatin-Sherwood Road/SW Langer Farms Parkway;
- SW Oregon Street/SW Tonquin Road;
- SW Oregon Street/SW Murdock Road;
- Blake Road / SW Cipole Road (future year scenarios only); and,
- Blake Road / SW $124^{\text {th }}$ Avenue (future year scenarios only).


## Analysis Scenarios

This preliminary study evaluated transportation conditions for the following scenarios:

- Year 2019 existing traffic within the study area during the weekday AM and PM peak hours;
- Year 2021 background traffic conditions (without the proposed development) during the weekday AM and PM peak hours, assuming that the Blake Road connection from SW Oregon Street to SW $124^{\text {th }}$ Avenue is in place;
- Year 2021 total traffic operations (with full build-out of the proposed development, assuming SW Cipole Road terminates within the site) during the weekday AM and PM peak hours;
- Year 2021 total traffic operations (with full build-out of the proposed development, assuming SW Cipole Road bisects the site to connect to Blake Road) during the weekday AM and PM peak hours;
- Year 2023 background traffic conditions (without the proposed development) during the weekday AM and PM peak hours, assuming the Blake Road connection from SW Oregon Street to SW $124^{\text {th }}$ Avenue is in place, that SW Tualatin-Sherwood Road has been widened to five lanes;
- Year 2023 total traffic operations (with full build-out of the proposed development, assuming Cipole Road terminates within the site) during the weekday AM and PM peak hours;
- Year 2023 total traffic operations (with full build-out of the proposed development, assuming Cipole Road bisects the site to connect to Blake Road) during the weekday AM and PM peak hours.


## TRAFFIC ANALYSIS

The site vicinity was visited and inventoried in February 2019. At that time, site conditions, adjacent land uses, existing traffic operations, and transportation facilities in the study area were collected.

## Analysis Methodology

All level-of-service analyses of signalized and stop-controlled intersections described in this report were performed in accordance with the procedures stated in the 2000 Highway Capacity Manual (HCM, Reference 1). The peak 15 -minute flow rates were used in the evaluation of all intersection level-ofservice (LOS) and volume-to-capacity (V/C) ratios. For this reason, the analyses reflect conditions that are only likely to occur for 15 minutes out of each average peak hour. Traffic conditions during typical weekday hours are expected to operate with lower levels of delay than those described in this report. Operational analyses for the signalized and stop-controlled intersections presented in this report were completed using Synchro 10 software. The roundabout intersection operations analyses were completed using SIDRA 7 software, based on the procedures stated in the Highway Capacity Manual $6^{\text {th }}$ Edition (HCM 6 ${ }^{\text {th }}$ Ed., Reference 2).

## Operating Standards

Per Section 8 of Sherwood's 2014 Transportation System Plan (TSP, Reference 3), "The City target for signalized, all way stop (AWSC), or roundabout intersections is level of service D or volume to capacity ratio equal to or less than 0.85 . The target for unsignalized two way stop control (TWSC) intersections is level of service E or a volume to capacity ratio equal to or less than 0.90 ." For the future year analysis assuming the extension of SW Cipole Road to Blake Road, the assumed future TWSC intersection of SW Cipole Road and Blake Road will be compared to the City of Sherwood unsignalized two way stop control (TWSC) intersection standards, under the assumption properties west of SW $124^{\text {th }}$ Avenue are brought into the City limits of Sherwood as planned.

For streets owned by Washington County or city-owned streets on the Arterial and Throughway Network Map within the 2014 Regional Transportation Plan (Reference 4), the Regional 0.99 volume to capacity (V/C) operating standard will be used. The Arterial and Throughway Network Map identifies SW Tualatin-Sherwood Road as a Major Arterial and SW Oregon Street as a Minor Arterial. As all existing study intersections are along SW Tualatin-Sherwood Road or SW Oregon Street, the 0.99 V/C operating standard will be used. Additionally, as SW $124^{\text {th }}$ Avenue extension is also identified as a Minor Arterial on the Arterial and Throughway Network, the $0.99 \mathrm{~V} / \mathrm{C}$ standard will also be used for the assumed future TWSC intersection of Blake Road and SW $124^{\text {th }}$ Avenue.

## Existing Traffic Operations

Intersection turning-movement counts were conducted at the study intersections when local area schools were in session in February 2019. All the weekday counts were conducted on a typical midweek day during the morning (7:00 to 9:00 AM) and evening (4:00 to 6:00 PM) peak time periods. From the counts, the weekday AM peak hour was found to occur from 7:20 to 8:20 AM and the PM peak hour occurs from 4:45 to 5:55 PM. Appendix "A" contains the February 2019 traffic count worksheets.

Table 1 summarizes the operational analysis for the study intersections under existing traffic conditions for the weekday AM and PM peak hours. As shown, all of the study intersections currently operate at acceptable levels and meet the mobility standards of the governing agency. However, as observed in the field, and reported within the queuing outputs in the Synchro worksheets, vehicle queueing is prevalent east-west along the SW Tualatin-Sherwood Road corridor during both AM and PM peak hours.

Appendix " $B$ " contains the year 2019 existing traffic level-of-service and queuing worksheets.

Table 1: Existing Conditions Operational Analysis Results

| \# | Intersection | LOS ${ }^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operational Standard | Standard Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 1 | SW Langer Farms Parkway/SW Tualatin-Sherwood Road | C (21.2) | C (26.1) | 0.72 | 0.82 | Regional | V/C of 0.99 | Yes |
| 2 | SW Oregon Street/SW Tualatin Sherwood Road | B (14.8) | C (28.2) | 0.77 | 0.96 | Regional | V/C of 0.99 | Yes |
| 3 | SW Wildrose Place/SW TualatinSherwood Road | D (25.5) | E (43.5) | 0.03 (SB) | 0.17 (SB) | Regional | V/C of 0.99 | Yes |
| 4 | SW Cipole Road/SW TualatinSherwood Road | A (7.3) | B (15.8) | 0.67 | 0.82 | Regional | V/C of 0.99 | Yes |
| 5 | SW 124 ${ }^{\text {th }}$ Avenue/SW TualatinSherwood Road | D (35.9) | C (27.3) | 0.88 | 0.72 | Regional | V/C of 0.99 | Yes |
| 6 | SW $120^{\text {th }}$ Avenue/SW TualatinSherwood Road | D (26.5) | C (19.5) | 0.10 (NB) | 0.10 (NB) | Regional | V/C of 0.99 | Yes |
| 7 | SW 115 ${ }^{\text {th }}$ Avenue/SW TualatinSherwood Road | B (15.9) | B (13.5) | 0.71 | 0.62 | Regional | V/C of 0.99 | Yes |
| 8 | SW 112 ${ }^{\text {th }}$ Avenue-SW Avery Street/SW Tualatin-Sherwood Road | C (24.6) | B (19.5) | 0.74 | 0.61 | Regional | V/C of 0.99 | Yes |
| 9 | SW Oregon Street/ SW Tonquin Road | B (14.2) | E (46.2) | 0.26 (NB) | 0.85 (NB) | Regional | V/C of 0.99 | Yes |
| 10 | SW Oregon Street/ SW Murdock Road | A (8.0) | A (8.7) | 0.53 | 0.62 | Regional | V/C of 0.99 | Yes |

${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or critical movement delay (TWSC), HCM 6 ${ }^{\text {th }}$ Ed. Level-of-Service and average delay per vehicle in seconds (roundabout);
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio (signalized) or HCM 6 th Ed. Volume-to-Capacity ratio (roundabout). For TWSC intersections, the critical movement is shown;
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan.

## Year 2021 Background Traffic Conditions

The year 2021 background traffic conditions analysis identifies how the study area's transportation system will operate without the proposed development. This analysis includes trips from traffic attributed to general growth in the region (application of a 1.5 percent annual growth rate), but does not include traffic from the proposed development. In-process trips from the following developments were including in the background traffic volumes:

- Parkway Village South (SW Langer Farms Parkway)
- Spring Creek Industrial
- Four-S Corporate Warehouse
- IPT Tualatin
- Majestic SW $115^{\text {th }}$ Avenue Industrial Park
- Hedges C Building
- Tualatin Business Park

Additionally, it was assumed that Blake Road would be in place from SW Oregon Street to SW $124^{\text {th }}$ Avenue, with some re-distribution of trips from the SW Oregon Street / SW Tualatin-Sherwood Road and SW $124^{\text {th }}$ Avenue / SW Tualatin-Sherwood Road intersections.

As Washington County plans to update the timing of signals along SW Tualatin-Sherwood Road later in 2019 to account for the recent opening of the SW $124^{\text {th }}$ Avenue extension, the future year analysis assumes the re-coordination of the traffic signals in the corridor at the SW Cipole Road, SW $124^{\text {th }}$ Avenue, SW $115^{\text {th }}$ Avenue and SW $112^{\text {th }}$ Avenue /SW Avery Street intersections. While the existing signal timing parameters show that during the AM peak hour, the SW Cipole Road and SW $124^{\text {th }}$ Avenue signals operate with a coordinated 120 second cycle length and the SW $115^{\text {th }}$ and SW $112^{\text {th }} / \mathrm{SW}$ Avery Street signals operate with a coordinated 140 second cycle, the future years analysis assumed that all four signals would be coordinated with 150 second cycle length during the AM peak, accounting for the addition of the northbound approach at the SW $124^{\text {th }}$ Avenue intersection and regional growth. No cycle length changes were assumed in the future year PM peak hour analysis, as the existing signal timing parameters show that all four signals operate with a coordinated 120 second cycle length, though the coordination offset was optimized to account for future traffic patterns.

Table 2 summarizes the operational analysis for the study intersections under the weekday AM and PM peak hour background 2021 traffic conditions. As indicated in Table 2, all study intersections except for the SW Oregon Street / SW Tualatin- Sherwood Road intersection are forecast to operate at levels which meet the mobility standards of the governing agency during both weekday AM and PM peak hours.

Appendix "C" contains the year 2021 background traffic level-of-service worksheets.

Table 2: Year 2021 Background Conditions Operational Analysis Results

| \# | Intersection | LOS $^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operating <br> Standard | Standard Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 1 | SW Langer Farms Parkway/SW Tualatin-Sherwood Road | C (24.1) | C (32.1) | 0.78 | 0.92 | Regional | $\begin{gathered} \mathrm{V} / \mathrm{C} \text { of } \\ 0.99 \end{gathered}$ | Yes |
| 2 | SW Oregon Street/SW Tualatin Sherwood Road | B (16.4) | D (35.5) | 0.84 | 1.01 | Regional | $\begin{gathered} \mathrm{V} / \mathrm{C} \text { of } \\ 0.99 \end{gathered}$ | No |
| 3 | SW Wildrose Place/SW TualatinSherwood Road | D(30.1) | F (75.7) | 0.04 (SB) | 0.27 (SB) | Regional | $\begin{gathered} \hline \mathrm{V} / \mathrm{C} \text { of } \\ 0.99 \end{gathered}$ | Yes |
| 4 | SW Cipole Road/SW TualatinSherwood Road | A (8.1) | C (24.9) | 0.72 | 0.90 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 5 | SW $124^{\text {th }}$ Avenue/SW TualatinSherwood Road | E (56.8) | D (36.4) | 0.98 | 0.82 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 6 | SW $120^{\text {th }}$ Avenue/SW TualatinSherwood Road | D (32.6) | C (22.8) | 0.11 (NB) | 0.13 (NB) | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 7 | SW $115^{\text {th }}$ Avenue/SW TualatinSherwood Road | B (18.1) | B (15.9) | 0.82 | 0.74 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 8 | SW 112 ${ }^{\text {th }}$ Avenue-SW Avery Street/SW Tualatin-Sherwood Road | C (31.9) | C (27.5) | 0.82 | 0.77 | Regional | $\begin{gathered} \hline \mathrm{V} / \mathrm{C} \text { of } \\ 0.99 \end{gathered}$ | Yes |
| 9 | SW Oregon Street/ SW Tonquin Road | C (15.2) | F (72.1) | 0.30 (NB) | 0.98 (NB) | Regional | $\begin{gathered} \hline \mathrm{V} / \mathrm{C} \text { of } \\ 0.99 \end{gathered}$ | Yes |
| 10 | SW Oregon Street/ SW Murdock Road | A (9.0) | B (10.3) | 0.60 | 0.68 | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 11 | Blake Road / SW 124 ${ }^{\text {th }}$ Avenue | B (11.8) | B (11.1) | 0.01 (WB) | 0.04 (WB) | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 12 | SW Cipole Road / Blake Road | Future Access (Cipole Extension Scenario) |  |  |  |  |  |  |

${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or critical movement delay (TWSC), HCM 6 ${ }^{\text {th }}$ Ed. Level-of-Service and average delay per vehicle in seconds (roundabout);
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio (signalized) or HCM 6 ${ }^{\text {th }}$ Ed. Volume-to-Capacity ratio (roundabout). For TWSC intersections, the critical movement is shown;
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan.

## Trip Generation

A preliminary trip generation estimate for the proposed development was prepared based on the Institute of Transportation Engineers' (ITE) Trip Generation Manual, 10 ${ }^{\text {th }}$ Edition (Reference 5). Table 3 displays the preliminary trip generation for the proposed site.

Table 3. Preliminary Trip Generation Estimate

| Land Use Category | ITE Code | Size (SF) | Total Daily Trips | Weekday AM Peak Hour |  |  | Weekday PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Total | In | Out | Total | In | Out |
| Industrial Park | 130 | 547,200 | 1,844 | 219 | 177 | 42 | 219 | 46 | 173 |

Per comments received from the City of Sherwood on the scoping memorandum, weekday peak hour driveway counts were conducted at a similar industrial development nearby, to confirm that the ITE land use code for Industrial Park would not underestimate trips for the planned development. Counts were collected during peak periods for three consecutive weekdays and analysis showed an average trip generation rate of approximately half that of ITE Industrial Park land use code. Therefore, for a conservative analysis, the ITE trip generation as presented in Table 3 was carried forward for the traffic analysis.

## Trip Distribution

Based on a review of general traffic patterns in the region, the proposed land use and external site access patterns, and prior history of our firm's involvement on other development projects in the City of Sherwood, the following site trip distributions are proposed:

- 10 percent to/from the west via SW Tualatin-Sherwood Road,
- 5 percent to/from the southwest via SW Oregon Street,
- 10 percent to/from the southwest via the future Blake Road,
- 20 percent to/from the southeast via the future Blake Road and SW $124^{\text {th }}$ Avenue extension,
- 5 percent to/from the north via Cipole Road,
- 15 percent to/from the north via SW $124^{\text {th }}$ Avenue,
- 10 percent to/from the east via SW $112^{\text {th }}$ Avenue - SW Avery Street, and
- 25 percent to/from the east via SW Tualatin-Sherwood Road.

For both site plan scenarios (SW Cipole Road termination or SW Cipole Road termination extension to Blake Road), the same trip distribution was used, though routing to and from the site varied. The trip distribution pattern for each site plan concept are displayed in Figures 4 and 5.

Site truck traffic percentage and distribution was estimated by review of the nearby industrial development driveway counts heavy vehicle percentage and turning movement counts collected at the NE $115^{\text {th }}$ Avenue / SW Tualatin Sherwood Road intersection. During the AM peak hour, it was estimated that 13 percent of the proposed development traffic would be heavy vehicles, and for the PM peak hour, it was estimated that 8 percent of the site traffic would be heavy vehicles. The east/west directional distribution of heavy vehicles at the NE $115^{\text {th }}$ Avenue / SW Tualatin Sherwood Road intersection generally even, therefore the heavy percentages listed above were applied evenly to each movement to and from the study site. More details on the percent heavy vehicles calculations and distribution is provided in Appendix D.



SW 120TH AVE/ SW TUALATIN-SHERWOOD RD

SW 115TH AVE/

$$
\begin{aligned}
& \text { SW 115TH AVE/ } \\
& \text { SW TUALATIN-SHERWOOD RD }
\end{aligned}
$$

$$
7
$$

SW 112TH AVE-SW AVERY ST/ SW TUALATIN-SHERWOOD RD


SW TONQUIN RD/
(9)


SW MURDOCK RD/ SW OREGON ST

SW 124THAVEI
BLAKE RD (FUTURE)


SW CIPOLE RD/
BLAKE RD (FUTURE)


## Year 2021 Total Traffic Conditions

The year 2021 total traffic conditions analysis identifies how the study area's transportation system will operate with the proposed development trips added to the background traffic volumes. Similar to the background year 2021 analysis, this analysis assumed that Blake Road would be in place from SW Oregon Street to SW $124^{\text {th }}$ Avenue, with limited re-distribution of trips from the SW Oregon Street / SW Tualatin-Sherwood Road and SW $124^{\text {th }}$ Avenue/SW Tualatin-Sherwood Road intersections.

## SW Cipole Road Cul-de-sac Termination Site Plan

Addition of the site trips shown in Figure 4 to the background 2021 volumes results in the operational results presented in Table 4. Appendix " $E$ " contains the year 2021 Total Traffic Cul-de-sac Site Plan level-of-service worksheets.

Table 4: Year 2021 Total Traffic Conditions - Cul-de-sac Site Plan Operational Analysis Results

| \# | Intersection | LOS $^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operating <br> Standard | Standard Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 1 | SW Langer Farms Parkway/SW Tualatin-Sherwood Road | C (25.3) | C (34.0) | 0.81 | 0.94 | Regional | V/C of 0.99 | Yes |
| 2 | SW Oregon Street/SW Tualatin Sherwood Road | B (19.3) | D (41.8) | 0.86 | 1.09 | Regional | V/C of 0.99 | No |
| 3 | SW Wildrose Place/SW TualatinSherwood Road | E (35.9) | F (132.9) | 0.05 (SB) | 0.42 (SB) | Regional | V/C of 0.99 | Yes |
| 4 | SW Cipole Road/SW TualatinSherwood Road | B (14.7) | C (34.8) | 0.81 | 0.94 | Regional | V/C of 0.99 | Yes |
| 5 | SW $124^{\text {th }}$ Avenue/SW TualatinSherwood Road | E (57.5) | D (37.9) | 0.99 | 0.83 | Regional | V/C of 0.99 | Yes |
| 6 | SW $120^{\text {th }}$ Avenue/SW TualatinSherwood Road | D (33.1) | C (23.6) | 0.12 (NB) | 0.13 (NB) | Regional | V/C of 0.99 | Yes |
| 7 | SW $115^{\text {th }}$ Avenue/SW TualatinSherwood Road | C (21.2) | B (18.1) | 0.83 | 0.77 | Regional | V/C of 0.99 | Yes |
| 8 | SW $112^{\text {th }}$ Avenue-SW Avery <br> Street/SW Tualatin-Sherwood Road | D (36.2) | C (29.0) | 0.83 | 0.79 | Regional | V/C of 0.99 | Yes |
| 9 | SW Oregon Street/ SW Tonquin Road | C (15.5) | F (87.2) | 0.31 (NB) | 1.03 (NB) | Regional | V/C of 0.99 | No |
| 10 | SW Oregon Street/ SW Murdock Road | A (9.3) | B (12.5) | 0.62 | 0.75 | Regional | V/C of 0.99 | Yes |
| 11 | Blake Road / SW 124 ${ }^{\text {th }}$ Avenue | B (12.4) | B (11.4) | 0.05(EB) | 0.02 (EB) | Regional | V/C of 0.99 | Yes |
| 12 | SW Cipole Road / Blake Road | Not Applicable to Cul-de-sac Site Plan Scenario |  |  |  |  |  |  |

[^12]
## SW Cipole Road Extension to Blake Road Site Plan

Addition of the site trips shown in Figure 5 to the background 2021 volumes results in the operational results presented in Table 5. Appendix "F" contains the year 2021 Total Traffic - Cipole Road Extension Site Plan level-of-service worksheets.

Table 5: Year 2021 Total Traffic Conditions - Cipole Road Extension Site Plan Operational Analysis Results

| \# | Intersection | LOS $^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operating <br> Standard | Standard Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 1 | SW Langer Farms Parkway/SW Tualatin-Sherwood Road | C (25.3) | C (34.0) | 0.81 | 0.94 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 2 | SW Oregon Street/SW Tualatin Sherwood Road | B (18.3) | D (40.0) | 0.85 | 1.08 | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | No |
| 3 | SW Wildrose Place/SW TualatinSherwood Road | D (34.6) | F (109.6) | 0.05 (SB) | 0.37 (SB) | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 4 | SW Cipole Road/SW TualatinSherwood Road | B (13.2) | C (30.0) | 0.78 | 0.91 | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 5 | SW $124^{\text {th }}$ Avenue/SW TualatinSherwood Road | E (58.3) | D (38.5) | 0.99 | 0.84 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 6 | SW $120^{\text {th }}$ Avenue/SW TualatinSherwood Road | D (33.2) | C (23.6) | 0.11 (NB) | 0.13 (NB) | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 7 | SW 115 ${ }^{\text {th }}$ Avenue/SW TualatinSherwood Road | C (21.1) | B (18.1) | 0.83 | 0.77 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 8 | SW 112 ${ }^{\text {th }}$ Avenue-SW Avery Street/SW Tualatin-Sherwood Road | D (36.3) | C (28.9) | 0.83 | 0.79 | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 9 | SW Oregon Street/ SW Tonquin Road | C (15.5) | F (87.2) | 0.31 (NB) | 1.03 (NB) | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | No |
| 10 | SW Oregon Street/ SW Murdock Road | A (9.3) | B (12.5) | 0.62 | 0.75 | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 11 | Blake Road / SW 124 ${ }^{\text {th }}$ Avenue | B (12.7) | B (11.5) | 0.06 (EB) | 0.05 (EB) | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 12 | SW Cipole Road / Blake Road | A (9.1) | A (9.2) | 0.02 (SB) | 0.07 (SB) | City of Sherwood | $\begin{gathered} \hline \text { LOS "E" or } \\ \text { V/C of } \\ 0.90 \end{gathered}$ | Yes |

${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or critical movement delay (TWSC), HCM 6 ${ }^{\text {th }}$ Ed. Level-of-Service and average delay per vehicle in seconds (roundabout);
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio (signalized) or HCM 6 ${ }^{\text {th }}$ Ed. Volume-to-Capacity ratio (roundabout). For TWSC intersections, the critical movement is shown;
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan.

As indicated in Tables 2, 4 and 5, the SW Oregon Street / SW Tualatin-Sherwood Road intersection v/c ratio is anticipated to exceed the regional operating standard during the PM peak hour, in year 2021 background conditions and with site development, considering both the SW Cipole Road cul-de-sac site plan and the SW Cipole Road extension site plan.

Additionally, the SW Oregon Street / SW Tonquin Road TWSC intersection v/c ratio is anticipated to exceed the regional operating standard during the PM peak hour with site development of either the SW Cipole Road cul-de-sac site plan or the SW Cipole Road extension site plan.

Year 2021 Total Traffic - Mitigation
As shown in Tables 4 and 5, the SW Oregon Street / SW Tonquin Road TWSC intersection v/c ratio is anticipated to exceed the regional operating standard during the PM peak hour with site development, independent of the site plan scenario pursued.

The City of Sherwood Transportation System Plan (TSP) identifies the reconstruction of the SW Oregon Street / SW Tonquin Road intersection as a roundabout as a "short-term" improvement. Additionally, Washington County's Transportation Development Tax (TDT) Road Project List identifies the reconstruction of the SW Oregon Street / SW Tonquin Road intersection as a roundabout in the 20142024 timeframe.

However, as the timeframe of the project and funding is unclear, mitigation of the SW Oregon Street / SW Tonquin Road intersection with either the installation of a traffic signal or roundabout was investigated. As summarized in Table 6, the SW Oregon Street / SW Tonquin Road intersection can meet the regional operating standards as a signalized or roundabout intersection. Appendix " $G$ " contains the Year 2021 Total Traffic - Tonquin/Oregon Mitigation level-of-service worksheets.

Table 6: Year 2021 Total Traffic Conditions - Mitigation Operational Analysis Results

|  | Intersection | LOS $^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operating <br> Standard | Standar <br> d Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 9 | SW Oregon Street/ SW Tonquin Road (signal) | A (7.9) | B (10.4) | 0.55 | 0.70 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 9 | SW Oregon Street/ SW Tonquin Road (roundabout) | B (11.3) | B (11.6) | 0.71 | 0.72 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |

${ }^{1} \mathrm{HCM} 2000$ Level-of-Service and average delay per vehicle in seconds (signalized) or $\mathrm{HCM} 6^{\text {th }}$ Ed. Level-of-Service and average delay per vehicle in seconds critical movement delay (roundabout);
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio (signalized) or HCM 6 th Ed. Volume-to-Capacity ratio (roundabout);
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan (RTFP).

## Year 2023 Background Traffic Conditions

The year 2023 background traffic conditions analysis identifies how the study area's transportation system will operate without the proposed development. Similar to the year 2021 background analysis, the year 2023 analysis includes trips from traffic attributed to general growth in the region (application of a 1.5 percent annual growth rate), trips from the in-process developments listed in the year 2021 background analysis section and some re-distribution of trips, assuming the connection of Blake Road from SW Oregon Street to SW $124^{\text {th }}$ Avenue.

Additionally, the 2023 background analysis includes the planned Widening of SW Tualatin-Sherwood Road to five lanes, project number 318 in the Washington County MSTIP 3e (Reference 6). Volumes on SW Tualatin-Sherwood Road and SW $124^{\text {th }}$ Avenue were increased an additional 5 percent on top of regional growth, to account for increased future demand.

Table 7 summarizes the operational analysis for the study intersections under the weekday AM and PM peak hour background 2023 traffic conditions and shows that all study intersections except for the SW Oregon Street/ SW Tonquin Road intersection are forecast to operate at levels which meet the mobility standards of the governing agency during both weekday AM and PM peak hours. Additionally, estimated queue lengths east-west along the SW Tualatin-Sherwood Road corridor are much lower than in the existing year 2019 and year 2021. Appendix " $H$ " contains the year 2023 background traffic level-of-service worksheets.

Table 7: Year 2023 Background Conditions Operational Analysis Results

| \# | Intersection | LOS ${ }^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operating <br> Standard | Standard Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 1 | SW Langer Farms Parkway/SW Tualatin-Sherwood Road | B (17.7) | C (24.0) | 0.63 | 0.77 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 2 | SW Oregon Street/SW Tualatin Sherwood Road | B (10.7) | B (17.4) | 0.73 | 0.83 | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 3 | SW Wildrose Place/SW TualatinSherwood Road | C (17.9) | C (14.7) | 0.02 (SB) | 0.07 (SB) | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 4 | SW Cipole Road/SW TualatinSherwood Road | A (6.2) | A (7.3) | 0.43 | 0.54 | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 5 | SW $124^{\text {th }}$ Avenue/SW TualatinSherwood Road | C (34.5) | C (26.0) | 0.73 | 0.62 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 6 | SW $120^{\text {th }}$ Avenue/SW TualatinSherwood Road | F (55.6) | C (22.2) | $\begin{aligned} & 0.20 \\ & (N B) \end{aligned}$ | 0.12 (NB) | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 7 | SW $115^{\text {th }}$ Avenue/SW TualatinSherwood Road | B (18.8) | B (13.2) | 0.58 | 0.49 | Regional | V/C of <br> 0.99 | Yes |
| 8 | SW 112 ${ }^{\text {th }}$ Avenue-SW Avery Street/SW Tualatin-Sherwood Road | D (36.9) | C (20.1) | 0.68 | 0.66 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 9 | SW Oregon Street/ SW Tonquin Road | C (15.6) | F (87.7) | $\begin{aligned} & 0.32 \\ & \text { (NB) } \end{aligned}$ | 1.03 (NB) | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | No |
| 10 | SW Oregon Street/ SW Murdock Road | A (9.4) | B (12.7) | 0.62 | 0.76 | Regional | V/C of <br> 0.99 | Yes |
| 11 | Blake Road / SW 124 ${ }^{\text {th }}$ Avenue | B (12.5) | B (11.5) | 0.05 (EB) | 0.02 (EB) | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 12 | SW Cipole Road / Blake Road | Future Access (Cipole Extension Scenario) |  |  |  |  |  |  |

${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or critical movement delay (TWSC), HCM 6 ${ }^{\text {th }}$ Ed. Level-of-Service and average delay per vehicle in seconds (roundabout);
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio (signalized) or HCM 6 ${ }^{\text {th }}$ Ed. Volume-to-Capacity ratio (roundabout). For TWSC intersections, the critical movement is shown;
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan.

## Year 2023 Total Traffic Conditions

The year 2023 total traffic conditions analysis identifies how the study area's transportation system will operate with the proposed development trips added to the 2023 background traffic volumes.

## SW Cipole Road Cul-de-sac Termination Site Plan

Addition of the site trips shows in Figure 4 to the year 2023 background volumes results in the operational results presented in Table 8. Appendix "I" contains the year 2023 Total Traffic Cul-de-sac Site Plan level-of-service worksheets.

Table 8: Year 2023 Total Traffic Conditions - Cul-de-sac Site Plan Operational Analysis Results

| \# | Intersection | LOS $^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operating <br> Standard | Standard Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 1 | SW Langer Farms Parkway/SW Tualatin-Sherwood Road | B (18.0) | C (24.7) | 0.65 | 0.81 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 2 | SW Oregon Street/SW Tualatin Sherwood Road | B (12.0) | B (19.0) | 0.78 | 0.85 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 3 | SW Wildrose Place/SW TualatinSherwood Road | C (18.4) | C (27.0) | 0.02 (SB) | 0.07 (SB) | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 4 | SW Cipole Road/SW TualatinSherwood Road | A (9.0) | B (13.8) | 0.48 | 0.58 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 5 | SW $124^{\text {th }}$ Avenue/SW TualatinSherwood Road | C (32.2) | C (28.0) | 0.74 | 0.63 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 6 | SW $120^{\text {th }}$ Avenue/SW TualatinSherwood Road | F (57.0) | C (23.3) | 0.21 (NB) | 0.13 (NB) | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 7 | SW $115^{\text {th }}$ Avenue/SW TualatinSherwood Road | B (18.7) | B (13.0) | 0.59 | 0.51 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 8 | SW $112^{\text {th }}$ Avenue-SW Avery Street/SW Tualatin-Sherwood Road | D (39.2) | C (20.7) | 0.70 | 0.68 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 9 | SW Oregon Street/ SW Tonquin Road | C (16.0) | F (105.7) | 0.33 (NB) | 1.09 (NB) | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | No |
| 10 | SW Oregon Street/ SW Murdock Road | B (9.8) | B (13.4) | 0.64 | 0.78 | Regional | V/C of 0.99 | Yes |
| 11 | Blake Road / SW 124 ${ }^{\text {th }}$ Avenue | B (12.7) | B (11.6) | 0.05 (EB) | 0.02 (EB) | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 12 | SW Cipole Road / Blake Road | Not Applicable to Cul-de-sac Site Plan Scenario |  |  |  |  |  |  |

${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or critical movement delay (TWSC), HCM $6^{\text {th }}$ Ed. Level-of-Service and average delay per vehicle in seconds (roundabout);
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio (signalized) or HCM 6 ${ }^{\text {th }}$ Ed. Volume-to-Capacity ratio (roundabout). For TWSC intersections, the critical movement is shown;
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan.

## SW Cipole Road Extension to Blake Road Site Plan

Addition of the site trips shows in Figure 5 to the year 2023 background volumes results in the operational results presented in Table 9. Appendix "J" contains the year 2023 Total Traffic Cipole Road Extension Site Plan level-of-service worksheets.

Table 9: Year 2023 Total Traffic Conditions - Cipole Road Extension Site Plan Operational Analysis Results

| \# | Intersection | LOS ${ }^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operating <br> Standard | Standard <br> Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 1 | SW Langer Farms Parkway/SW Tualatin-Sherwood Road | B (18.0) | C (24.7) | 0.65 | 0.81 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 2 | SW Oregon Street/SW Tualatin Sherwood Road | B (11.5) | B (17.5) | 0.76 | 0.83 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 3 | SW Wildrose Place/SW TualatinSherwood Road | C (18.2) | D (26.6) | 0.02 (SB) | 0.07 (SB) | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 4 | SW Cipole Road/SW TualatinSherwood Road | A (8.3) | B (11.0) | 0.47 | 0.56 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 5 | SW $124^{\text {th }}$ Avenue/SW TualatinSherwood Road | C (32.4) | C (26.9) | 0.74 | 0.64 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 6 | SW $120^{\text {th }}$ Avenue/SW TualatinSherwood Road | F (57.0) | C (23.3) | 0.21 (NB) | 0.13 (NB) | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 7 | SW $115^{\text {th }}$ Avenue/SW TualatinSherwood Road | B (18.7) | B (13.0) | 0.59 | 0.51 | Regional | $\begin{gathered} \mathrm{V} / \mathrm{C} \text { of } \\ 0.99 \end{gathered}$ | Yes |
| 8 | SW 112 ${ }^{\text {th }}$ Avenue-SW Avery Street/SW Tualatin-Sherwood Road | D (39.1) | C (20.6) | 0.70 | 0.68 | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 9 | SW Oregon Street/ SW Tonquin Road | C (16.0) | F (105.7) | 0.33 (NB) | 1.09 (NB) | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | No |
| 10 | SW Oregon Street/ SW Murdock Road | A (9.8) | B (13.4) | 0.64 | 0.78 | Regional | $\begin{gathered} \hline \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 11 | Blake Road / SW 124 ${ }^{\text {th }}$ Avenue | B (13.1) | B (11.8) | 0.06 (EB) | 0.05 (EB) | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 12 | SW Cipole Road / Blake Road | A (9.1) | A (9.2) | 0.02 (SB) | 0.07 (SB) | City of Sherwood | $\begin{gathered} \text { LOS "E" or } \\ \text { V/C of } \\ 0.90 \end{gathered}$ | Yes |

${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or critical movement delay (TWSC), HCM $6^{\text {th }}$ Ed. Level-of-Service and average delay per vehicle in seconds (roundabout);
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio (signalized) or HCM 6 ${ }^{\text {th }}$ Ed. Volume-to-Capacity ratio (roundabout). For TWSC intersections, the critical movement is shown;
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan.

## Year 2023 Total Traffic - Mitigation

As indicated in Tables 7, 8 and 9, the SW Oregon Street / SW Tonquin Road TWSC intersection v/c ratio is anticipated to exceed the regional operating standard during the PM peak hour in Year 2023 with or without site development.

As previously discussed, the timing and funding of the planned conversion of the SW Oregon Street / SW Tonquin Road intersection to a roundabout is unclear, therefore mitigation with either the installation of a traffic signal or roundabout was investigated. As summarized in Table 10, the SW Oregon Street / SW Tonquin Road intersection can meet the regional operating standards during the AM and PM peak hours as a signalized or roundabout intersection. Appendix " $K$ " contains the Year 2023 Total Traffic - Tonquin/Oregon Mitigation level-of-service worksheets.

Table 10: Year 2023 Total Traffic Conditions - Mitigation Operational Analysis Results

| \# | Intersection | LOS $^{1}$ |  | $\mathrm{V} / \mathrm{C}^{2}$ |  | Jurisdiction ${ }^{3}$ | Operating Standard | Standard Met? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |  |  |  |
| 9 | SW Oregon Street/ SW Tonquin Road (signal) | A (8.1) | B (10.6) | 0.57 | 0.72 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |
| 9 | SW Oregon Street/ SW Tonquin Road (roundabout) | B (11.9) | B (12.5) | 0.73 | 0.75 | Regional | $\begin{gathered} \text { V/C of } \\ 0.99 \end{gathered}$ | Yes |

${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or HCM 6 ${ }^{\text {th }}$ Ed. Level-of-Service and average delay per vehicle in seconds critical movement delay (roundabout);
${ }^{2}$ HCM 2000 Volume-to-Capacity ratio(signalized) or HCM 6 ${ }^{\text {th }}$ Ed. Volume-to-Capacity ratio (roundabout);
${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan (RTFP).

## Year 2023 Cipole Road Scenario Comparison

As shown in Tables 4, 5, 8 and 9, and summarized below in Table 11, the connection of SW Cipole Road to Blake Road has mixed impacts to the operations of the immediately adjacent intersections. Intersection operations that deteriorate with the extension of SW Cipole Road are highlighted in red, and operations that improve with the extension of SW Cipole Road are highlighted in green.

Table 11: Comparison of Cipole Road Extension Impacts on Total Traffic Operations (V/C)

| Scenario | SW Cipole Road / SW Tualatin-Sherwood Road |  | SW 124 ${ }^{\text {th }}$ Avenue / SW Tualatin-Sherwood Road |  | SW Cipole Road / <br> Blake Road |  | SW 124 ${ }^{\text {th }}$ Avenue / Blake Road |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | AM | PM | AM | PM | AM | PM | AM | PM |
| Year 2021 |  |  |  |  |  |  |  |  |
| Cipole Road Cul-de-sac | 0.81 | 0.94 | 0.99 | 0.83 | - | - | 0.01 | 0.04 |
| Cipole Road Extension | 0.78 | 0.91 | 0.99 | 0.84 | 0.02 | 0.07 | 0.06 | 0.04 |
| Year 2023* |  |  |  |  |  |  |  |  |
| Cipole Road Cul-de-sac | 0.48 | 0.58 | 0.74 | 0.63 | - | - | 0.05 | 0.05 |
| Cipole Road Extension | 0.47 | 0.56 | 0.74 | 0.64 | 0.02 | 0.07 | 0.06 | 0.05 |

* Accounts for planned 5-lane widening along Tualatin-Sherwood Road.

As shown above, regardless of whether or not SW Cipole Road is extended through the site, the adjacent study intersections are all anticipated to meet the regional mobility standard of $\mathrm{v} / \mathrm{c}$ of 0.99 or less. Nevertheless, while the extension of SW Cipole Road results in slightly improved operations at the SW Cipole Road / SW Tualatin-Sherwood Road intersection, operations remain the same or slightly deteriorate at the SW $124^{\text {th }}$ Avenue / SW Tualatin-Sherwood Road, SW Cipole Road/Blake Road and SW $124^{\text {th }}$ Avenue / Blake Road intersections. Therefore, there appears to be no significant system-wide benefit to extending SW Cipole Road through the site to connect with the future Blake Road.

In addition to the operational impacts of the SW Cipole Road extension, the impacts on traffic safety should also be considered. A connection to Blake Road would add an access point to the roadway network, introducing conflict. Were the connection to be made, vehicles (including large trucks) associated with the Sherwood Industrial Park would enter or leave the site by making unprotected left
turns across a collector street (Blake Road) and arterial roadway ( $124^{\text {th }}$ Avenue), whereas, without the connection to Blake Road, left-turning vehicles would have the added protection of traffic signal phasing at both the SW Cipole Road / SW Tualatin-Sherwood Road and SW $124^{\text {th }}$ Avenue/SW TualatinSherwood Road intersections. In our opinion, limiting Cipole Road to a cul-de-sac ending would result in fewer unprotected left-turn conflict points on the surrounding roadway network, especially those involving large trucks.

## SimTraffic Queuing Analysis

A Simtraffic queuing analysis was completed for four build scenarios (Year 2021, Year 2023, for both SW Cipoole Road Scenarios) during the PM peak hour, inclusive of the following study intersections:

- SW Cipole Road / SW Tualatin-Sherwood Road
- SW 124 ${ }^{\text {th }}$ Avenue / SW Tualatin-Sherwood Road
- SW $124^{\text {th }}$ Avenue / Blake Road
- SW Cipole Road / Blake Road

For the SimTraffic analysis, four 15 -minute periods were recorded, with the second period representative of the peak 15 -minute period, with the report results averaging five runs. Appendix " $L$ " contains the Year 2021 Total Traffic SimTraffic worksheets and Appendix " $M$ " contains the Year 2023 Total Traffic Simtraffic worksheets.

Estimated $95^{\text {th }}$ percentile queues improve significantly from year 2021 to 2023, as would be expected with the widening of the SW Tualatin-Sherwood Road corridor. As summarized in Table 12 below, by year 2023, all queues would be accommodated for both SW Cipole Road scenarios.

Table 12: Year 2023 Total Traffic Conditions PM Peak Hour - SimTraffic 95 ${ }^{\text {th }}$ Percentile Queue Summary

| Intersection | Scenario |  | Eastbound |  |  | Westbound |  |  | Northbound |  |  | Southbound |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right |
| SW Cipole <br> Road / <br> SW <br> Tualatin- <br> Sherwood <br> Road |  | Storage (feet) | 360 | 1100 | - | 250 | 790 | - | 200 | 500 | - | 300 | 725 | - |
|  | SW Cipole <br> Road Cul- <br> de-Sac | Queue (feet) | 50 | 175 | - | 50 | 200 | - | 150 | 100 | - | 125 | 100 | - |
|  | SW Cipole <br> Road <br> Extension | Queue (feet) | 75 | 150 | - | 50 | 175 | - | 125 | 100 | - | 125 | 125 | - |
| SW $124^{\text {th }}$ <br> Avenue / SW <br> Tualatin- <br> Sherwood <br> Road |  | Storage (feet) | $100^{1}$ | 790 | - | 375 | 1180 | - | 400 | 1000 | - | 300 | 730 | - |
|  | SW Cipole <br> Road Cul-de-Sac | Queue (feet) | 125 | 325 | - | 75 | 300 | - | 175 | 200 | - | 275 | 275 | - |
|  | SW Cipole <br> Road <br> Extension | Queue (feet) | 100 | 325 | - | 75 | 325 | - | 150 | 200 | - | 250 | 250 | - |
| SW 124th Avenue / Blake Road |  | Storage (feet) ${ }^{2}$ | 150 | 800 | - | 150 | - | - | - | 1000 | - | - | 1000 | - |
|  | SW Cipole <br> Road Cul-de-Sac | Queue (feet) | 50 | 50 | - | 50 | - | - | - | - | - | - | 25 | - |
|  | SW Cipole <br> Road Extension | Queue (feet) | 50 | 50 | - | 50 | - | - | - | 25 | - | - | 25 | - |
| SW Cipole Road / Blake Road |  | Storage (feet) ${ }^{3}$ | 150 | - | - | - | - | - | - | - | - | - | 400 | - |
|  | SW Cipole Road Cul-de-Sac | Queue (feet) | - | - | - | - | - | - | - | - | - | - | - | - |
|  | SW Cipole <br> Road <br> Extension | Queue (feet) | 25 | - | - | - | - | - | - | - | - | - | - | - |

Notes:
$95^{\text {th }}$ percentile queue lengths have been rounded up to the nearest car length, assuming one vehicle equals 25 feet;
Bold and highlighted cells indicate $95^{\text {th }}$ percentile queue lengths greater than the storage length;
${ }^{1}$ Storage measured as the length of white gore stripe for turn lane, additional queue storage available in TWLTL;
${ }^{2}$ 2Storage for future intersection left-turn lanes assumed to be 150 feet;
${ }^{3}$ Storage for future intersection eastbound left-turn lanes assumed to be 150.

## FINDINGS

In summary, the proposed Sherwood Industrial Park can be developed while maintaining acceptable levels of mobility at the study intersections, with and without the connection of SW Cipole Road to Blake Road through the study site, by future year 2023, assuming agency-planned improvements are in place. However, in the interim period between now and the site build-out year 2021, two intersections are forecast to exceed governing agency standards:

- The SW Oregon Street / SW Tualatin-Sherwood Road intersection is anticipated to exceed Regional operating standards by 2021, with or without the Sherwood Industrial Park development. However, when SW Tualatin-Sherwood Road is widening to five lanes by year 2023, the SW Oregon Street / SW Tualatin-Sherwood Road intersection will meet Regional operating standards.
- With the proposed development in place, the SW Oregon Street / SW Tonquin Road TWSC intersection is anticipated to exceed the Regional operating standard during the PM peak hour of year 2021. The SW Oregon Street / SW Tonquin Road intersection can meet the regional operating standards with either a signal or a roundabout installed.

Therefore, the following mitigation is recommended in conjunction with site development:

- Provision of a proportionate share fee-in-lieu for the provision of a temporary signal at the SW Oregon Street / SW Tonquin Road intersection.

As shown in the two total traffic year 2023 analyses, the connection of SW Cipole Road to the future Blake Road through the study site will not materially impact the ability of the study intersections to operate within the operational thresholds. A comparison of these two potential scenarios led to the following findings which support limiting SW Cipole Road to a cul-de-sac ending, rather than extending it through the site to Blake Road:

- Traffic Operations: Regardless of whether or not SW Cipole Road is extended through the site, the adjacent study intersections are all anticipated to meet the regional mobility standard. While the extension of SW Cipole Road results in slightly improved operations at the SW Cipole Road / SW Tualatin-Sherwood Road intersection, operations remain the same or slightly deteriorate at the SW $124^{\text {th }}$ Avenue / SW Tualatin-Sherwood Road, SW Cipole Road/Blake Road and SW $124^{\text {th }}$ Avenue / Blake Road intersections. Therefore, there appears to be no significant system-wide benefit to extending SW Cipole Road through the site to connect with the future Blake Road.
- Traffic Safety: A connection to Blake Road would add an access point to the roadway network, introducing conflict. Limiting SW Cipole Road to a cul-de-sac ending would result in fewer unprotected left-turn conflict points on the surrounding roadway network, especially those involving large trucks.

Following completion of this preliminary findings memorandum, Kittelson and Associates, Inc. (KAI) will prepare a full TIS per the requirements enumerated in Sherwood's Development Code Section 16.106.080, Washington County's Resolution \& Order 86-95, and scoping direction received from the City and County staff.

If you have any questions, please give us a call at (503) 535-7447.

## REFERENCES

1) Transportation Research Board. 2000 Highway Capacity Manual. 2000.
2) Transportation Research Board. Highway Capacity Manual, 6th Edition. 2016.
3) City of Sherwood. Transportation System Plan. 2014.
4) Metro. Regional Transportation Plan Update. 2014.
5) Institute of Transportation Engineers. Trip Generation Manual, $10^{\text {th }}$ Edition. 2017.
6) Washington County. MSTIP 3e Adopted Funding Program and Project List. 2016.
7) City of Sherwood. Capital Improvement Plan. 2018
8) Washington County. Transportation Development Tax Road Project List. 2018.

## ATTACHMENTS

Appendix A: Traffic Count Data
Appendix B: Year 2019 Existing Operations Worksheets
Appendix C: Year 2021 Background Operations Worksheets
Appendix D: Driveway Count Heavy Vehicle Data
Appendix E: Year 2021 Total Traffic - Cipole Road Cul-de-sac Operations Worksheets
Appendix F: Year 2021 Total Traffic - Cipole Road Extension Operations Worksheets
Appendix G: Year 2021 Total Traffic - Tonquin/Oregon Mitigation Worksheets
Appendix H: Year 2023 Background Operations Worksheets
Appendix I: Year 2023 Total Traffic - Cipole Road Cul-de-sac Operations Worksheets
Appendix J: Year 2023 Total Traffic - Cipole Road Extension Operations Worksheets
Appendix K: Year 2023 Total Traffic - Tonquin/Oregon Mitigation Worksheets
Appendix L: Year 2021 Total Traffic - SimTraffic Worksheets
Appendix M: Year 2023 Total Traffic - SimTraffic Worksheets

## Appendix B Year 2019 Existing Operations Worksheets



Analysis Period (min)
15
c Critical Lane Group


c Critical Lane Group

c Critical Lane Group

# Appendix C Year 2021 Background Operations Worksheets 



Analysis Period (min) 15
C Critical Lane Group

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
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| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\hat{F}$ |  | \% | $\uparrow$ |  | \% | ¢ |  | ${ }^{7}$ | $\uparrow$ |  |
| Traffic Volume (veh/h) | 23 | 10 | 20 | 4 | 4 | 2 | 0 | 373 | 24 | 16 | 193 | 12 |
| Future Volume (Veh/h) | 23 | 10 | 20 | 4 | 4 | 2 | 0 | 373 | 24 | 16 | 193 | 12 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 25 | 11 | 22 | 4 | 4 | 2 | 0 | 401 | 26 | 17 | 208 | 13 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (ft) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (fts) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | TWLTL |  |  | TWLTL |  |
| Median storage veh) |  |  |  |  |  |  |  | 2 |  |  | 2 |  |
| Upstream signal (ft) |  |  |  |  |  |  |  |  |  |  | 1007 |  |
| pX, platoon unblocked | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |  | 0.94 |  |  |  |  |  |
| vC , conflicting volume | 654 | 676 | 214 | 684 | 669 | 414 | 221 |  |  | 427 |  |  |
| VC1, stage 1 conf vol | 248 | 248 |  | 414 | 414 |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol | 405 | 427 |  | 270 | 255 |  |  |  |  |  |  |  |
| vCu , unblocked vol | 595 | 619 | 126 | 627 | 612 | 414 | 133 |  |  | 427 |  |  |
| tC, single (s) | 7.2 | 6.7 | 6.4 | 7.2 | 6.7 | 6.4 | 4.2 |  |  | 4.2 |  |  |
| $\mathrm{tC}, 2$ stage (s) | 6.2 | 5.7 |  | 6.2 | 5.7 |  |  |  |  |  |  |  |
| tF (s) | 3.6 | 4.1 | 3.4 | 3.6 | 4.1 | 3.4 | 2.3 |  |  | 2.3 |  |  |
| p0 queue free \% | 95 | 98 | 97 | 99 | 99 | 100 | 100 |  |  | 98 |  |  |
| cM capacity (veh/h) | 527 | 501 | 833 | 519 | 514 | 611 | 1287 |  |  | 1066 |  |  |
| Direction, Lane \# | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 | SB 1 | SB 2 |  |  |  |  |
| Volume Total | 25 | 33 | 4 | 6 | 0 | 427 | 17 | 221 |  |  |  |  |
| Volume Left | 25 | 0 | 4 | 0 | 0 | 0 | 17 | 0 |  |  |  |  |
| Volume Right | 0 | 22 | 0 | 2 | 0 | 26 | 0 | 13 |  |  |  |  |
| CSH | 527 | 682 | 519 | 543 | 1700 | 1700 | 1066 | 1700 |  |  |  |  |
| Volume to Capacity | 0.05 | 0.05 | 0.01 | 0.01 | 0.00 | 0.25 | 0.02 | 0.13 |  |  |  |  |
| Queue Length 95th (ft) | 4 | 4 | 1 | 1 | 0 | 0 | 1 | 0 |  |  |  |  |
| Control Delay (s) | 12.2 | 10.5 | 12.0 | 11.7 | 0.0 | 0.0 | 8.4 | 0.0 |  |  |  |  |
| Lane LOS | B | B | B | B |  |  | A |  |  |  |  |  |
| Approach Delay (s) | 11.2 |  | 11.8 |  | 0.0 |  | 0.6 |  |  |  |  |  |
| Approach LOS | B |  | B |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 1.2 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 35.7\% |  | CU Level | f Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |



C Critical Lane Group

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c Critical Lane Group


## Appendix D Year 2021 Total Traffic - Cipole Road Cul-desac Operations Worksheets




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| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\hat{F}$ |  | \% | $\uparrow$ |  | \% | ¢ |  | ${ }^{7}$ | $\uparrow$ |  |
| Traffic Volume (veh/h) | 23 | 10 | 20 | 4 | 4 | 2 | 0 | 391 | 24 | 16 | 197 | 12 |
| Future Volume (Veh/h) | 23 | 10 | 20 | 4 | 4 | 2 | 0 | 391 | 24 | 16 | 197 | 12 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 25 | 11 | 22 | 4 | 4 | 2 | 0 | 420 | 26 | 17 | 212 | 13 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (ft) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (fts) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | TWLTL |  |  | TWLTL |  |
| Median storage veh) |  |  |  |  |  |  |  | 2 |  |  | 2 |  |
| Upstream signal (ft) |  |  |  |  |  |  |  |  |  |  | 1007 |  |
| pX, platoon unblocked | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |  | 0.93 |  |  |  |  |  |
| vC , conflicting volume | 676 | 698 | 218 | 706 | 692 | 433 | 225 |  |  | 446 |  |  |
| VC1, stage 1 conf vol | 252 | 252 |  | 433 | 433 |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol | 424 | 446 |  | 274 | 259 |  |  |  |  |  |  |  |
| vCu , unblocked vol | 619 | 643 | 129 | 651 | 636 | 433 | 136 |  |  | 446 |  |  |
| tC, single (s) | 7.2 | 6.7 | 6.4 | 7.2 | 6.7 | 6.4 | 4.2 |  |  | 4.2 |  |  |
| $\mathrm{tC}, 2$ stage (s) | 6.2 | 5.7 |  | 6.2 | 5.7 |  |  |  |  |  |  |  |
| tF (s) | 3.6 | 4.1 | 3.4 | 3.6 | 4.1 | 3.4 | 2.3 |  |  | 2.3 |  |  |
| p0 queue free \% | 95 | 98 | 97 | 99 | 99 | 100 | 100 |  |  | 98 |  |  |
| cM capacity (veh/h) | 514 | 491 | 829 | 508 | 505 | 596 | 1282 |  |  | 1049 |  |  |
| Direction, Lane \# | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 | SB 1 | SB 2 |  |  |  |  |
| Volume Total | 25 | 33 | 4 | 6 | 0 | 446 | 17 | 225 |  |  |  |  |
| Volume Left | 25 | 0 | 4 | 0 | 0 | 0 | 17 | 0 |  |  |  |  |
| Volume Right | 0 | 22 | 0 | 2 | 0 | 26 | 0 | 13 |  |  |  |  |
| CSH | 514 | 674 | 508 | 532 | 1700 | 1700 | 1049 | 1700 |  |  |  |  |
| Volume to Capacity | 0.05 | 0.05 | 0.01 | 0.01 | 0.00 | 0.26 | 0.02 | 0.13 |  |  |  |  |
| Queue Length 95th (ft) | 4 | 4 | 1 | 1 | 0 | 0 | 1 | 0 |  |  |  |  |
| Control Delay (s) | 12.4 | 10.6 | 12.1 | 11.8 | 0.0 | 0.0 | 8.5 | 0.0 |  |  |  |  |
| Lane LOS | B | B | B | B |  |  | A |  |  |  |  |  |
| Approach Delay (s) | 11.4 |  | 12.0 |  | 0.0 |  | 0.6 |  |  |  |  |  |
| Approach LOS | B |  | B |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 1.2 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 36.6\% |  | CU Level | f Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


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C Critical Lane Group

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c Critical Lane Group

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| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | $\hat{1}$ |  | \% | $\uparrow$ |  | \% | $\uparrow$ |  | \% | $\uparrow$ |  |
| Traffic Volume (veh/h) | 11 | 5 | 10 | 22 | 14 | 16 | 0 | 225 | 3 | 2 | 327 | 44 |
| Future Volume (Veh/h) | 11 | 5 | 10 | 22 | 14 | 16 | 0 | 225 | 3 | 2 | 327 | 44 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Hourly flow rate (vph) | 11 | 5 | 10 | 23 | 15 | 17 | 0 | 234 | 3 | 2 | 341 | 46 |
| Pedestrians |  |  |  |  | 1 |  |  | 1 |  |  | 2 |  |
| Lane Width (ft) |  |  |  |  | 12.0 |  |  | 12.0 |  |  | 12.0 |  |
| Walking Speed (fts) |  |  |  |  | 3.5 |  |  | 3.5 |  |  | 3.5 |  |
| Percent Blockage |  |  |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | TWLTL |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  | 2 |  |
| Upstream signal (ft) |  |  |  |  |  |  |  |  |  |  | 978 |  |
| pX, platoon unblocked | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |  | 0.90 |  |  |  |  |  |
| vC , conflicting volume | 628 | 606 | 365 | 595 | 628 | 238 | 387 |  |  | 238 |  |  |
| $\mathrm{vC1}$, stage 1 conf vol | 368 | 368 |  | 236 | 236 |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol | 260 | 238 |  | 358 | 391 |  |  |  |  |  |  |  |
| vCu , unblocked vol | 528 | 503 | 234 | 490 | 527 | 238 | 258 |  |  | 238 |  |  |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| $\mathrm{tC}, 2$ stage (s) | 6.1 | 5.5 |  | 6.1 | 5.5 |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| po queue free \% | 98 | 99 | 99 | 96 | 97 | 98 | 100 |  |  | 100 |  |  |
| cM capacity (veh/h) | 577 | 564 | 715 | 598 | 553 | 791 | 1155 |  |  | 1310 |  |  |
| Direction, Lane \# | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 | SB 1 | SB 2 |  |  |  |  |
| Volume Total | 11 | 15 | 23 | 32 | 0 | 237 | 2 | 387 |  |  |  |  |
| Volume Left | 11 | 0 | 23 | 0 | 0 | 0 | 2 | 0 |  |  |  |  |
| Volume Right | 0 | 10 | 0 | 17 | 0 | 3 | 0 | 46 |  |  |  |  |
| CSH | 577 | 656 | 598 | 658 | 1700 | 1700 | 1310 | 1700 |  |  |  |  |
| Volume to Capacity | 0.02 | 0.02 | 0.04 | 0.05 | 0.00 | 0.14 | 0.00 | 0.23 |  |  |  |  |
| Queue Length 95th (ft) | 1 | 2 | 3 | 4 | 0 | 0 | 0 | 0 |  |  |  |  |
| Control Delay (s) | 11.4 | 10.6 | 11.3 | 10.7 | 0.0 | 0.0 | 7.8 | 0.0 |  |  |  |  |
| Lane LOS | B | B | B | B |  |  | A |  |  |  |  |  |
| Approach Delay (s) | 10.9 |  | 11.0 |  | 0.0 |  | 0.0 |  |  |  |  |  |
| Approach LOS | B |  | B |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 1.3 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 34.8\% | ICU Level of Service |  |  |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |

> Appendix E Year 2021 Total Traffic - Cipole Road Extension Operations Worksheets

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | $\uparrow$ | \% | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | 1 |  |
| Traffic Volume (vph) | 110 | 1005 | 68 | 62 | 655 | 75 | 15 | 2 | 11 | 47 | 9 | 30 |
| Future Volume (vph) | 110 | 1005 | 68 | 62 | 655 | 75 | 15 | 2 | 11 | 47 | 9 | 30 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 5.5 |  | 4.5 | 5.5 | 5.5 | 4.5 | 4.5 |  | 5.0 | 5.0 |  |
| Lane Util. Factor | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Frpb, ped/bikes | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Flpb, ped/bikes | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Frt | 1.00 | 0.99 |  | 1.00 | 1.00 | 0.85 | 1.00 | 0.87 |  | 1.00 | 0.88 |  |
| Flt Protected | 0.95 | 1.00 |  | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.95 | 1.00 |  |
| Satd. Flow (prot) | 1703 | 1721 |  | 1597 | 1639 | 1418 | 1597 | 1468 |  | 1289 | 1331 |  |
| Flt Permitted | 0.35 | 1.00 |  | 0.17 | 1.00 | 1.00 | 0.73 | 1.00 |  | 0.75 | 1.00 |  |
| Satd. Flow (perm) | 632 | 1721 |  | 279 | 1639 | 1418 | 1229 | 1468 |  | 1017 | 1331 |  |
| Peak-hour factor, PHF | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Adj. Flow (vph) | 115 | 1047 | 71 | 65 | 682 | 78 | 16 | 2 | 11 | 49 | 9 | 31 |
| RTOR Reduction (vph) | 0 | 1 | 0 | 0 | 0 | 14 | 0 | 10 | 0 | 0 | 29 | 0 |
| Lane Group Flow (vph) | 115 | 1117 | 0 | 65 | 682 | 64 | 16 | 3 | 0 | 49 | 11 | 0 |
| Confl. Bikes (\#/hr) |  |  | 5 |  |  |  |  |  |  |  |  |  |
| Heavy Vehicles (\%) | 6\% | 8\% | 13\% | 13\% | 15\% | 13\% | 13\% | 13\% | 13\% | 40\% | 13\% | 30\% |
| Bus Blockages (\#/hr) | 0 | 2 | 0 | 0 | 2 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| Turn Type | pm+pt | NA |  | pm+pt | NA | Prot | Perm | NA |  | Perm | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 | 6 |  | 8 |  |  | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |  |
| Actuated Green, G (s) | 125.3 | 118.5 |  | 122.6 | 117.4 | 117.4 | 11.8 | 11.8 |  | 11.3 | 11.3 |  |
| Effective Green, g (s) | 125.3 | 118.5 |  | 122.6 | 117.4 | 117.4 | 11.8 | 11.8 |  | 11.3 | 11.3 |  |
| Actuated g/C Ratio | 0.84 | 0.79 |  | 0.82 | 0.78 | 0.78 | 0.08 | 0.08 |  | 0.08 | 0.08 |  |
| Clearance Time (s) | 4.0 | 5.5 |  | 4.5 | 5.5 | 5.5 | 4.5 | 4.5 |  | 5.0 | 5.0 |  |
| Vehicle Extension (s) | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |  | 3.0 | 3.0 |  |
| Lane Grp Cap (vph) | 576 | 1359 |  | 273 | 1282 | 1109 | 96 | 115 |  | 76 | 100 |  |
| v/s Ratio Prot | c0.01 | c0.65 |  | 0.01 | 0.42 | 0.05 |  | 0.00 |  |  | 0.01 |  |
| v/s Ratio Perm | 0.16 |  |  | 0.19 |  |  | 0.01 |  |  | c0.05 |  |  |
| v/c Ratio | 0.20 | 0.82 |  | 0.24 | 0.53 | 0.06 | 0.17 | 0.02 |  | 0.64 | 0.11 |  |
| Uniform Delay, d1 | 3.1 | 9.4 |  | 10.5 | 6.1 | 3.7 | 64.5 | 63.8 |  | 67.4 | 64.7 |  |
| Progression Factor | 1.00 | 1.00 |  | 0.44 | 0.39 | 0.12 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Incremental Delay, d2 | 0.2 | 5.7 |  | 0.3 | 1.2 | 0.1 | 0.8 | 0.1 |  | 17.2 | 0.5 |  |
| Delay (s) | 3.3 | 15.1 |  | 4.9 | 3.6 | 0.5 | 65.3 | 63.9 |  | 84.6 | 65.2 |  |
| Level of Service | A | B |  | A | A | A | E | E |  | F | E |  |
| Approach Delay (s) |  | 14.0 |  |  | 3.4 |  |  | 64.7 |  |  | 75.9 |  |
| Approach LOS |  | B |  |  | A |  |  | E |  |  | E |  |


| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 2000 Control Delay | 13.2 | HCM 2000 Level of Service | B |
| HCM 2000 Volume to Capacity ratio | 0.78 |  | 15.0 |
| Actuated Cycle Length (s) | 150.0 | Sum of lost time (s) | E |
| Intersection Capacity Utilization | $83.0 \%$ | ICU Level of Service |  |
| Analysis Period (min) | 15 |  |  |
| c Critical Lane Group |  |  |  |





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c Critical Lane Group



## Appendix F Year 2025 Background Operations Worksheets



Analysis Period (min) 15
c Critical Lane Group

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \％${ }^{*}$ | 个 $\uparrow$ | 「 | \％${ }^{*}$ | 个 $\uparrow$ | 「 | 7＊ | 蚛 |  | ${ }^{7}$ | 个t |  |
| Traffic Volume（vph） | 75 | 1051 | 47 | 26 | 619 | 200 | 133 | 229 | 77 | 182 | 172 | 58 |
| Future Volume（vph） | 75 | 1051 | 47 | 26 | 619 | 200 | 133 | 229 | 77 | 182 | 172 | 58 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time（s） | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 |  | 4.0 | 5.5 |  |
| Lane Util．Factor | 0.97 | 0.95 | 1.00 | 0.97 | 0.95 | 1.00 | 0.97 | 0.95 |  | 1.00 | 0.95 |  |
| Frpb，ped／bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Flpb，ped／bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Frt | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 0.96 |  | 1.00 | 0.96 |  |
| Flt Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.95 | 1.00 |  |
| Satd．Flow（prot） | 3242 | 3299 | 1252 | 2334 | 3127 | 1381 | 3155 | 3013 |  | 1612 | 3007 |  |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.27 | 1.00 |  |
| Satd．Flow（perm） | 3242 | 3299 | 1252 | 2334 | 3127 | 1381 | 3155 | 3013 |  | 454 | 3007 |  |
| Peak－hour factor，PHF | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Adj．Flow（vph） | 81 | 1130 | 51 | 28 | 666 | 215 | 143 | 246 | 83 | 196 | 185 | 62 |
| RTOR Reduction（vph） | 0 | 0 | 15 | 0 | 0 | 59 | 0 | 26 | 0 | 0 | 24 | 0 |
| Lane Group Flow（vph） | 81 | 1130 | 36 | 28 | 666 | 156 | 143 | 303 | 0 | 196 | 223 | 0 |
| Confl．Bikes（\＃／hr） |  |  | 3 |  |  | 1 |  |  |  |  |  |  |
| Heavy Vehicles（\％） | 8\％ | 9\％ | 28\％ | 50\％ | 15\％ | 16\％ | 11\％ | 10\％ | 31\％ | 12\％ | 12\％ | 26\％ |
| Bus Blockages（\＃／hr） | 0 | 2 | 2 | 0 | 2 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| Turn Type | Prot | NA | $\mathrm{pt}+\mathrm{v}$ | Prot | NA | $\mathrm{pt}+0 \mathrm{v}$ | Prot | NA |  | pm＋pt | NA |  |
| Protected Phases | 5 | 2 | 23 | 1 | 6 | 67 | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases |  |  |  |  |  |  |  |  |  | 4 |  |  |
| Actuated Green，G（s） | 7.7 | 89.7 | 104.7 | 4.0 | 86.0 | 108.9 | 9.5 | 19.9 |  | 41.3 | 27.8 |  |
| Effective Green， g （s） | 7.7 | 89.7 | 104.7 | 4.0 | 86.0 | 108.9 | 9.5 | 19.9 |  | 41.3 | 27.8 |  |
| Actuated g／C Ratio | 0.05 | 0.60 | 0.70 | 0.03 | 0.57 | 0.73 | 0.06 | 0.13 |  | 0.28 | 0.19 |  |
| Clearance Time（s） | 4.0 | 5.5 |  | 4.0 | 5.5 |  | 4.0 | 5.5 |  | 4.0 | 5.5 |  |
| Vehicle Extension（s） | 1.5 | 4.5 |  | 1.5 | 4.5 |  | 0.2 | 2.0 |  | 0.2 | 2.0 |  |
| Lane Grp Cap（vph） | 166 | 1972 | 873 | 62 | 1792 | 1002 | 199 | 399 |  | 259 | 557 |  |
| v／s Ratio Prot | c0．02 | c0．34 | 0.03 | 0.01 | 0.21 | 0.11 | 0.05 | 0.10 |  | c0．09 | 0.07 |  |
| v／s Ratio Perm |  |  |  |  |  |  |  |  |  | c0．12 |  |  |
| v／c Ratio | 0.49 | 0.57 | 0.04 | 0.45 | 0.37 | 0.16 | 0.72 | 0.76 |  | 0.76 | 0.40 |  |
| Uniform Delay，d1 | 69.2 | 18.4 | 7.0 | 71.9 | 17.4 | 6.3 | 68.9 | 62.7 |  | 45.5 | 53.8 |  |
| Progression Factor | 0.95 | 1.03 | 2.74 | 1.35 | 0.49 | 1.55 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Incremental Delay，d2 | 0.8 | 1.1 | 0.0 | 1.8 | 0.6 | 0.0 | 9.9 | 7.2 |  | 10.6 | 0.2 |  |
| Delay（s） | 66.6 | 20.2 | 19.3 | 99.1 | 9.1 | 9.9 | 78.8 | 69.9 |  | 56.1 | 53.9 |  |
| Level of Service | E | C | B | F | A | A | E | E |  | E | D |  |
| Approach Delay（s） |  | 23.1 |  |  | 12.0 |  |  | 72.6 |  |  | 54.9 |  |
| Approach LOS |  | C |  |  | B |  |  | E |  |  | D |  |


| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 2000 Control Delay | 32.0 | HCM 2000 Level of Service | C |
| HCM 2000 Volume to Capacity ratio | 0.64 |  | 19.0 |
| Actuated Cycle Length（s） | 150.0 | Sum of lost time（s） | B |
| Intersection Capacity Utilization | $63.5 \%$ | ICU Level of Service |  |
| Analysis Period（min） | 15 |  |  |
| C Critical Lane Group |  |  |  |


|  | 4 |  |  | 7 |  |  | 4 | $\dagger$ | 7 |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\uparrow$ |  | \% | $\uparrow$ |  | \% | $\uparrow$ |  | ${ }^{*}$ | $\uparrow$ |  |
| Traffic Volume (veh/h) | 23 | 10 | 20 | 4 | 4 | 2 | 0 | 414 | 24 | 16 | 217 | 12 |
| Future Volume (Veh/h) | 23 | 10 | 20 | 4 | 4 | 2 | 0 | 414 | 24 | 16 | 217 | 12 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 25 | 11 | 22 | 4 | 4 | 2 | 0 | 445 | 26 | 17 | 233 | 13 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (ft) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (ft/s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | TWLTL |  |  | TWLTL |  |
| Median storage veh) |  |  |  |  |  |  |  | 2 |  |  | 2 |  |
| Upstream signal (ft) |  |  |  |  |  |  |  |  |  |  | 1007 |  |
| pX, platoon unblocked | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |  | 0.92 |  |  |  |  |  |
| vC , conflicting volume | 722 | 744 | 240 | 752 | 738 | 458 | 246 |  |  | 471 |  |  |
| VC1, stage 1 conf vol | 274 | 274 |  | 458 | 458 |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol | 449 | 471 |  | 294 | 280 |  |  |  |  |  |  |  |
| vCu , unblocked vol | 659 | 683 | 137 | 692 | 676 | 458 | 144 |  |  | 471 |  |  |
| tC, single (s) | 7.2 | 6.7 | 6.4 | 7.2 | 6.7 | 6.4 | 4.2 |  |  | 4.2 |  |  |
| $\mathrm{tC}, 2$ stage (s) | 6.2 | 5.7 |  | 6.2 | 5.7 |  |  |  |  |  |  |  |
| tF (s) | 3.6 | 4.1 | 3.4 | 3.6 | 4.1 | 3.4 | 2.3 |  |  | 2.3 |  |  |
| p0 queue free \% | 95 | 98 | 97 | 99 | 99 | 100 | 100 |  |  | 98 |  |  |
| cM capacity (veh/h) | 495 | 475 | 812 | 489 | 489 | 577 | 1260 |  |  | 1026 |  |  |
| Direction, Lane \# | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 | SB 1 | SB 2 |  |  |  |  |
| Volume Total | 25 | 33 | 4 | 6 | 0 | 471 | 17 | 246 |  |  |  |  |
| Volume Left | 25 | 0 | 4 | 0 | 0 | 0 | 17 | 0 |  |  |  |  |
| Volume Right | 0 | 22 | 0 | 2 | 0 | 26 | 0 | 13 |  |  |  |  |
| CSH | 495 | 657 | 489 | 515 | 1700 | 1700 | 1026 | 1700 |  |  |  |  |
| Volume to Capacity | 0.05 | 0.05 | 0.01 | 0.01 | 0.00 | 0.28 | 0.02 | 0.14 |  |  |  |  |
| Queue Length 95th (ft) | 4 | 4 | 1 | 1 | 0 | 0 | 1 | 0 |  |  |  |  |
| Control Delay (s) | 12.7 | 10.8 | 12.4 | 12.1 | 0.0 | 0.0 | 8.6 | 0.0 |  |  |  |  |
| Lane LOS | B | B | B | B |  |  | A |  |  |  |  |  |
| Approach Delay (s) | 11.6 |  | 12.2 |  | 0.0 |  | 0.6 |  |  |  |  |  |
| Approach LOS | B |  | B |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 1.2 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 37.9\% | ICU Level of Service |  |  |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


c Critical Lane Group

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

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## Appendix G Year 2025 Total Traffic - Cipole Road Cul-de-sac Operations Worksheets

## 4: Cipole Rd \& Tualatin-Sherwood Rd



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | 中 ${ }_{\text {d }}$ |  | \% | 中 ${ }^{\text {a }}$ |  | \% | $\uparrow$ |  | \% | $\uparrow$ |  |
| Traffic Volume (vph) | 110 | 1126 | 88 | 80 | 735 | 75 | 21 | 2 | 19 | 47 | , | 30 |
| Future Volume (vph) | 110 | 1126 | 88 | 80 | 735 | 75 | 21 | 2 | 19 | 47 | 9 | 30 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |


| Total Lost time (s) | 4.0 | 5.5 |  | 4.5 | 5.5 |  | 4.5 | 4.5 |  | 5.0 | 5.0 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Lane Util. Factor | 1.00 | 0.95 |  | 1.00 | 0.95 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |
| Frpb, ped/bikes | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |
| Flpb, ped/bikes | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Frt | 1.00 | 0.99 |  | 1.00 | 0.99 |  | 1.00 | 0.86 |  | 1.00 | 0.88 |
| Flt Protected | 0.95 | 1.00 |  | 0.95 | 1.00 |  | 0.95 | 1.00 | 0.95 | 1.00 |  |
| Satd. Flow (prot) | 1703 | 3277 |  | 1597 | 3088 |  | 1597 | 1452 |  | 1289 | 1331 |
| Flt Permitted | 0.32 | 1.00 |  | 0.19 | 1.00 |  | 0.73 | 1.00 |  | 0.74 | 1.00 |
| Satd. Flow (perm) | 577 | 3277 |  | 321 | 3088 |  | 1229 | 1452 |  | 1008 | 1331 |
| Peak-hour factor, PHF | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Adj. Flow (vph) | 115 | 1173 | 92 | 83 | 766 | 78 | 22 | 2 | 20 | 49 | 0 |
| RTOR Reduction (vph) | 0 | 3 | 0 | 0 | 4 | 0 | 0 | 18 | 0 | 0 | 29 |
| Lane Group Flow (vph) | 115 | 1262 | 0 | 83 | 840 | 0 | 22 | 4 | 0 | 49 | 11 |


| Confl. Bikes (\#hr) |  |  | 5 |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Heavy Vehicles (\%) | $6 \%$ | $8 \%$ | $13 \%$ | $13 \%$ | $15 \%$ | $13 \%$ | $13 \%$ | $13 \%$ | $13 \%$ | $40 \%$ | $13 \%$ | $30 \%$ |
| Bus Blockages (\#/hr) | 0 | 2 | 0 | 0 | 2 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |


| Turn Type | pm+pt | NA | pm+pt | NA | Perm | NA | Perm | NA |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Protected Phases | 5 | 2 | 1 | 6 |  | 8 |  | 4 |
| Permitted Phases | 2 |  | 6 |  | 8 |  | 4 |  |
| Actuated Green, G (s) | 123.2 | 116.1 | 124.5 | 117.0 | 11.9 | 11.9 | 11.4 | 11.4 |
| Effective Green, g (s) | 123.2 | 116.1 | 124.5 | 117.0 | 11.9 | 11.9 | 11.4 | 11.4 |
| Actuated g/C Ratio | 0.82 | 0.77 | 0.83 | 0.78 | 0.08 | 0.08 | 0.08 | 0.08 |
| Clearance Time (s) | 4.0 | 5.5 | 4.5 | 5.5 | 4.5 | 4.5 | 5.0 | 5.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 527 | 2536 | 330 | 2408 | 97 | 115 | 76 | 101 |
| v/s Ratio Prot | 0.01 | c0.39 | c0.01 | 0.27 |  | 0.00 |  | 0.01 |
| $\mathrm{v} / \mathrm{s}$ Ratio Perm | 0.17 |  | 0.20 |  | 0.02 |  | c0.05 |  |
| v/c Ratio | 0.22 | 0.50 | 0.25 | 0.35 | 0.23 | 0.03 | 0.64 | 0.11 |
| Uniform Delay, d1 | 2.6 | 6.2 | 3.3 | 5.0 | 64.7 | 63.7 | 67.3 | 64.6 |
| Progression Factor | 1.00 | 1.00 | 0.97 | 0.94 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | 0.2 | 0.7 | 0.4 | 0.4 | 1.2 | 0.1 | 17.2 | 0.5 |
| Delay (s) | 2.9 | 6.9 | 3.6 | 5.1 | 65.9 | 63.8 | 84.6 | 65.1 |
| Level of Service | A | A | A | A | E | E | F | E |
| Approach Delay (s) |  | 6.6 |  | 4.9 |  | 64.9 |  | 75.8 |
| Approach LOS |  | A |  | A |  | E |  | E |

Intersection Summary

| HCM 2000 Control Delay | 9.5 | HCM 2000 Level of Service | A |
| :--- | ---: | :--- | ---: |
| HCM 2000 Volume to Capacity ratio | 0.50 |  | 15.0 |
| Actuated Cycle Length (s) | 150.0 | Sum of lost time (s) | B |
| Intersection Capacity Utilization | $60.1 \%$ | ICU Level of Service |  |

Analysis Period (min)
15
c Critical Lane Group

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
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| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\hat{F}$ |  | \% | $\uparrow$ |  | \% | $\uparrow$ |  | ${ }^{*}$ | $\uparrow$ |  |
| Traffic Volume (veh/h) | 23 | 10 | 20 |  | 4 | 2 | 0 | 432 | 24 | 16 | 221 | 12 |
| Future Volume (Veh/h) | 23 | 10 | 20 | 4 | 4 | 2 | 0 | 432 | 24 | 16 | 221 | 12 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 25 | 11 | 22 | 4 | 4 | 2 | 0 | 465 | 26 | 17 | 238 | 13 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (ft) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (fts) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | TWLTL |  |  | TWLTL |  |
| Median storage veh) |  |  |  |  |  |  |  | 2 |  |  | 2 |  |
| Upstream signal (ft) |  |  |  |  |  |  |  |  |  |  | 1007 |  |
| pX, platoon unblocked | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |  | 0.92 |  |  |  |  |  |
| vC , conflicting volume | 748 | 770 | 244 | 778 | 763 | 478 | 251 |  |  | 491 |  |  |
| vC 1 , stage 1 conf vol | 278 | 278 |  | 478 | 478 |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol | 469 | 491 |  | 300 | 285 |  |  |  |  |  |  |  |
| vCu , unblocked vol | 683 | 707 | 137 | 716 | 700 | 478 | 144 |  |  | 491 |  |  |
| tC , single (s) | 7.2 | 6.7 | 6.4 | 7.2 | 6.7 | 6.4 | 4.2 |  |  | 4.2 |  |  |
| $\mathrm{tC}, 2$ stage (s) | 6.2 | 5.7 |  | 6.2 | 5.7 |  |  |  |  |  |  |  |
| tF (s) | 3.6 | 4.1 | 3.4 | 3.6 | 4.1 | 3.4 | 2.3 |  |  | 2.3 |  |  |
| p0 queue free \% | 95 | 98 | 97 | 99 | 99 | 100 | 100 |  |  | 98 |  |  |
| cM capacity (veh/h) | 483 | 465 | 809 | 478 | 479 | 562 | 1255 |  |  | 1008 |  |  |
| Direction, Lane \# | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 | SB1 | SB 2 |  |  |  |  |
| Volume Total | 25 | 33 | 4 | 6 | 0 | 491 | 17 | 251 |  |  |  |  |
| Volume Left | 25 | 0 | 4 | 0 | 0 | 0 | 17 | 0 |  |  |  |  |
| Volume Right | 0 | 22 | 0 | 2 | 0 | 26 | 0 | 13 |  |  |  |  |
| CSH | 483 | 649 | 478 | 504 | 1700 | 1700 | 1008 | 1700 |  |  |  |  |
| Volume to Capacity | 0.05 | 0.05 | 0.01 | 0.01 | 0.00 | 0.29 | 0.02 | 0.15 |  |  |  |  |
| Queue Length 95th (ft) | 4 | 4 | 1 | 1 | 0 | 0 | 1 | 0 |  |  |  |  |
| Control Delay (s) | 12.9 | 10.8 | 12.6 | 12.2 | 0.0 | 0.0 | 8.6 | 0.0 |  |  |  |  |
| Lane LOS | B | B | B | B |  |  | A |  |  |  |  |  |
| Approach Delay (s) | 11.7 |  | 12.4 |  | 0.0 |  | 0.5 |  |  |  |  |  |
| Approach LOS | B |  | B |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 1.1 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 38.8\% |  | CU Level | f Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

c Critical Lane Group

|  | 4 |  | $\checkmark$ | 7 |  | 4 | 4 | 4 | 7 |  | $\ddagger$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7} 1$ | 44 | 「7 | ${ }^{7}$ | 中4 | 「 | ${ }^{7}$ | 㻢 |  | ${ }^{*}$ | 㻢 |  |
| Traffic Volume（vph） | 70 | 958 | 130 | 61 | 994 | 127 | 129 | 137 | 12 | 207 | 218 | 226 |
| Future Volume（vph） | 70 | 958 | 130 | 61 | 994 | 127 | 129 | 137 | 12 | 207 | 218 | 226 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time（s） | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 |  | 4.0 | 5.5 |  |
| Lane Util．Factor | 0.97 | 0.95 | 1.00 | 0.97 | 0.95 | 1.00 | 0.97 | 0.95 |  | 1.00 | 0.95 |  |
| Frpb，ped／bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Flpb，ped／bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Frt | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 0.99 |  | 1.00 | 0.92 |  |
| Fit Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.95 | 1.00 |  |
| Satd．Flow（prot） | 3502 | 3457 | 1456 | 3502 | 3491 | 1571 | 3400 | 3497 |  | 1735 | 3238 |  |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.49 | 1.00 |  |
| Satd．Flow（perm） | 3502 | 3457 | 1456 | 3502 | 3491 | 1571 | 3400 | 3497 |  | 893 | 3238 |  |
| Peak－hour factor，PHF | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Adj．Flow（vph） | 73 | 998 | 135 | 64 | 1035 | 132 | 134 | 143 | 12 | 216 | 227 | 235 |
| RTOR Reduction（vph） | 0 | 0 | 54 | 0 | 0 | 43 | 0 | 4 | 0 | 0 | 97 | 0 |
| Lane Group Flow（vph） | 73 | 998 | 81 | 64 | 1035 | 89 | 134 | 152 | 0 | 216 | 365 | 0 |
| Confl．Peds．（\＃／hr） | 2 |  | 1 | 1 |  | 2 |  |  | 1 | 1 |  |  |
| Confl．Bikes（\＃／hr） |  |  | 1 |  |  | 2 |  |  |  |  |  |  |
| Heavy Vehicles（\％） | 0\％ | 4\％ | 10\％ | 0\％ | 3\％ | 2\％ | 3\％ | 2\％ | 0\％ | 4\％ | 4\％ | 2\％ |
| Bus Blockages（\＃／hr） | 0 | 2 | 2 | 0 | 2 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| Turn Type | Prot | NA | pt＋ov | Prot | NA | $\mathrm{pt}+\mathrm{ov}$ | Prot | NA |  | pm＋pt | NA |  |
| Protected Phases | 5 | 2 | 23 | 1 | 6 | 67 | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases |  |  |  |  |  |  |  |  |  | 4 |  |  |
| Actuated Green，G（s） | 5.2 | 44.1 | 57.2 | 4.9 | 43.8 | 64.2 | 7.6 | 11.9 |  | 30.8 | 19.2 |  |
| Effective Green，g（s） | 5.2 | 44.1 | 57.2 | 4.9 | 43.8 | 64.2 | 7.6 | 11.9 |  | 30.8 | 19.2 |  |
| Actuated g／C Ratio | 0.05 | 0.47 | 0.60 | 0.05 | 0.46 | 0.68 | 0.08 | 0.13 |  | 0.32 | 0.20 |  |
| Clearance Time（s） | 4.0 | 5.5 |  | 4.0 | 5.5 |  | 4.0 | 5.5 |  | 4.0 | 5.5 |  |
| Vehicle Extension（s） | 1.5 | 4.5 |  | 1.5 | 4.5 |  | 0.2 | 2.0 |  | 0.2 | 2.0 |  |
| Lane Grp Cap（vph） | 192 | 1608 | 878 | 181 | 1612 | 1063 | 272 | 438 |  | 422 | 655 |  |
| v／s Ratio Prot | c0．02 | 0.29 | 0.06 | 0.02 | c0．30 | 0.06 | 0.04 | 0.04 |  | c0．08 | c0．11 |  |
| v／s Ratio Perm |  |  |  |  |  |  |  |  |  | 0.09 |  |  |
| v／c Ratio | 0.38 | 0.62 | 0.09 | 0.35 | 0.64 | 0.08 | 0.49 | 0.35 |  | 0.51 | 0.56 |  |
| Uniform Delay，d1 | 43.2 | 19.1 | 7.9 | 43.4 | 19.5 | 5.2 | 41.8 | 37.9 |  | 24.8 | 34.0 |  |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Incremental Delay，d2 | 0.5 | 1.0 | 0.1 | 0.4 | 1.1 | 0.1 | 0.5 | 0.2 |  | 0.4 | 0.6 |  |
| Delay（s） | 43.7 | 20.0 | 8.0 | 43.9 | 20.6 | 5.3 | 42.3 | 38.1 |  | 25.2 | 34.6 |  |
| Level of Service | D | C | A | D | C | A | D | D |  | C | C |  |
| Approach Delay（s） |  | 20.1 |  |  | 20.2 |  |  | 40.0 |  |  | 31.6 |  |
| Approach LOS |  | C |  |  | C |  |  | D |  |  | C |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 24.1 |  | HCM 2000 | evel of | ervice |  | C |  |  |  |
| HCM 2000 Volume to Capacity ratio |  |  | 0.61 |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length（s） |  |  | 94.8 |  | Sum of lost | time（s） |  |  | 19.0 |  |  |  |
| Intersection Capacity Utilization |  |  | 61．1\％ |  | CU Level | Service |  |  | B |  |  |  |
| Analysis Period（min） |  |  | 15 |  |  |  |  |  |  |  |  |  |
| c Critical Lane Group |  |  |  |  |  |  |  |  |  |  |  |  |



## Appendix H Year 2025 Total Traffic - Cipole Road Extension Operations Worksheets

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \％${ }^{1}$ | 个4 | 「 | ＊＊ | 个4 | 「 | \％${ }^{1 / 1}$ | 性 |  | \％ | 个 ${ }_{\text {d }}$ |  |
| Traffic Volume（vph） | 78 | 1059 | 47 | 26 | 663 | 200 | 133 | 230 | 80 | 182 | 172 | 76 |
| Future Volume（vph） | 78 | 1059 | 47 | 26 | 663 | 200 | 133 | 230 | 80 | 182 | 172 | 76 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time（s） | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 | 5.5 | 4.0 | 5.5 |  | 4.0 | 5.5 |  |
| Lane Util．Factor | 0.97 | 0.95 | 1.00 | 0.97 | 0.95 | 1.00 | 0.97 | 0.95 |  | 1.00 | 0.95 |  |
| Frpb，ped／bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Flpb，ped／bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Fit | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 0.96 |  | 1.00 | 0.95 |  |
| Flt Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.95 | 1.00 |  |
| Satd．Flow（prot） | 3242 | 3299 | 1252 | 2334 | 3127 | 1381 | 3155 | 3006 |  | 1612 | 2961 |  |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |  | 0.26 | 1.00 |  |
| Satd．Flow（perm） | 3242 | 3299 | 1252 | 2334 | 3127 | 1381 | 3155 | 3006 |  | 449 | 2961 |  |
| Peak－hour factor，PHF | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Adj．Flow（vph） | 84 | 1139 | 51 | 28 | 713 | 215 | 143 | 247 | 86 | 196 | 185 | 82 |
| RTOR Reduction（vph） | 0 | 0 | 15 | 0 | 0 | 59 | 0 | 27 | 0 | 0 | 37 | 0 |
| Lane Group Flow（vph） | 84 | 1139 | 36 | 28 | 713 | 156 | 143 | 306 | 0 | 196 | 230 | 0 |
| Confl．Bikes（\＃／hr） |  |  | 3 |  |  | 1 |  |  |  |  |  |  |
| Heavy Vehicles（\％） | 8\％ | 9\％ | 28\％ | 50\％ | 15\％ | 16\％ | 11\％ | 10\％ | 31\％ | 12\％ | 12\％ | 26\％ |
| Bus Blockages（\＃／hr） | 0 | 2 | 2 | 0 | 2 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| Turn Type | Prot | NA | pt＋ov | Prot | NA | pt＋ov | Prot | NA |  | pm＋pt | NA |  |
| Protected Phases | 5 | 2 | 23 | 1 | 6 | 67 | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases |  |  |  |  |  |  |  |  |  | 4 |  |  |
| Actuated Green，G（s） | 7.8 | 89.6 | 104.6 | 4.0 | 85.8 | 108.6 | 9.5 | 20.1 |  | 41.4 | 27.9 |  |
| Effective Green，g（s） | 7.8 | 89.6 | 104.6 | 4.0 | 85.8 | 108.6 | 9.5 | 20.1 |  | 41.4 | 27.9 |  |
| Actuated g／C Ratio | 0.05 | 0.60 | 0.70 | 0.03 | 0.57 | 0.72 | 0.06 | 0.13 |  | 0.28 | 0.19 |  |
| Clearance Time（s） | 4.0 | 5.5 |  | 4.0 | 5.5 |  | 4.0 | 5.5 |  | 4.0 | 5.5 |  |
| Vehicle Extension（s） | 1.5 | 4.5 |  | 1.5 | 4.5 |  | 0.2 | 2.0 |  | 0.2 | 2.0 |  |
| Lane Grp Cap（vph） | 168 | 1970 | 873 | 62 | 1788 | 999 | 199 | 402 |  | 258 | 550 |  |
| v／s Ratio Prot | c0．03 | c0．35 | 0.03 | 0.01 | 0.23 | 0.11 | 0.05 | 0.10 |  | c0．09 | 0.08 |  |
| v／s Ratio Perm |  |  |  |  |  |  |  |  |  | c0．12 |  |  |
| v／c Ratio | 0.50 | 0.58 | 0.04 | 0.45 | 0.40 | 0.16 | 0.72 | 0.76 |  | 0.76 | 0.42 |  |
| Uniform Delay，d1 | 69.2 | 18.6 | 7.1 | 71.9 | 17.8 | 6.4 | 68.9 | 62.6 |  | 45.4 | 53.9 |  |
| Progression Factor | 1.14 | 0.78 | 0.47 | 1.38 | 0.35 | 0.93 | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Incremental Delay，d2 | 0.8 | 1.1 | 0.0 | 1.8 | 0.6 | 0.0 | 9.9 | 7.5 |  | 10.8 | 0.2 |  |
| Delay（s） | 80.0 | 15.7 | 3.3 | 101.1 | 6.9 | 6.0 | 78.8 | 70.1 |  | 56.2 | 54.1 |  |
| Level of Service | E | B | A | F | A | A | E | E |  | E | D |  |
| Approach Delay（s） |  | 19.4 |  |  | 9.4 |  |  | 72.7 |  |  | 55.0 |  |
| Approach LOS |  | B |  |  | A |  |  | E |  |  | D |  |


| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 2000 Control Delay | 29.6 | HCM 2000 Level of Service | C |
| HCM 2000 Volume to Capacity ratio | 0.65 |  | 19.0 |
| Actuated Cycle Length（s） | 150.0 | Sum of lost time（s） | C |
| Intersection Capacity Utilization | $64.9 \%$ | ICU Level of Service |  |
| Analysis Period（min） | 15 |  |  |
| C Critical Lane Group |  |  |  |


|  | $\downarrow$ |  |  | 7 |  |  |  | 4 |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\hat{\beta}$ |  | \% | $\uparrow$ |  | \% | $\uparrow$ |  | ${ }^{*}$ | $\uparrow$ |  |
| Traffic Volume (veh/h) | 27 | 10 | 24 |  | 4 | 2 | 18 | 414 | 24 | 16 | 217 | 12 |
| Future Volume (Veh/h) | 27 | 10 | 24 | 4 | 4 | 2 | 18 | 414 | 24 | 16 | 217 | 12 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 29 | 11 | 26 | 4 | 4 | 2 | 19 | 445 | 26 | 17 | 233 | 13 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (ft) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (fts) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | TWLTL |  |  | TWLTL |  |
| Median storage veh) |  |  |  |  |  |  |  | 2 |  |  | 2 |  |
| Upstream signal (ft) |  |  |  |  |  |  |  |  |  |  | 1007 |  |
| pX, platoon unblocked | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |  | 0.92 |  |  |  |  |  |
| vC , conflicting volume | 760 | 782 | 240 | 794 | 776 | 458 | 246 |  |  | 471 |  |  |
| vC 1 , stage 1 conf vol | 274 | 274 |  | 496 | 496 |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol | 487 | 509 |  | 298 | 280 |  |  |  |  |  |  |  |
| vCu , unblocked vol | 698 | 722 | 133 | 735 | 715 | 458 | 140 |  |  | 471 |  |  |
| tC , single (s) | 7.2 | 6.7 | 6.4 | 7.2 | 6.7 | 6.4 | 4.2 |  |  | 4.2 |  |  |
| $\mathrm{tC}, 2$ stage (s) | 6.2 | 5.7 |  | 6.2 | 5.7 |  |  |  |  |  |  |  |
| tF (s) | 3.6 | 4.1 | 3.4 | 3.6 | 4.1 | 3.4 | 2.3 |  |  | 2.3 |  |  |
| p0 queue free \% | 94 | 98 | 97 | 99 | 99 | 100 | 98 |  |  | 98 |  |  |
| cM capacity (veh/h) | 468 | 452 | 814 | 460 | 464 | 577 | 1260 |  |  | 1026 |  |  |
| Direction, Lane \# | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 | SB 1 | SB 2 |  |  |  |  |
| Volume Total | 29 | 37 | 4 | 6 | 19 | 471 | 17 | 246 |  |  |  |  |
| Volume Left | 29 | 0 | 4 | 0 | 19 | 0 | 17 | 0 |  |  |  |  |
| Volume Right | 0 | 26 | 0 | 2 | 0 | 26 | 0 | 13 |  |  |  |  |
| CSH | 468 | 657 | 460 | 496 | 1260 | 1700 | 1026 | 1700 |  |  |  |  |
| Volume to Capacity | 0.06 | 0.06 | 0.01 | 0.01 | 0.02 | 0.28 | 0.02 | 0.14 |  |  |  |  |
| Queue Length 95th (ft) | 5 | 4 | 1 | 1 | 1 | 0 | 1 | 0 |  |  |  |  |
| Control Delay (s) | 13.2 | 10.8 | 12.9 | 12.3 | 7.9 | 0.0 | 8.6 | 0.0 |  |  |  |  |
| Lane LOS | B | B | B | B | A |  | A |  |  |  |  |  |
| Approach Delay (s) | 11.9 |  | 12.6 |  | 0.3 |  | 0.6 |  |  |  |  |  |
| Approach LOS | B |  | B |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 1.5 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 38.1\% |  | CU Level | f Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |



c Critical Lane Group

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

c Critical Lane Group

|  | $\rangle$ |  |  | 7 |  |  | 4 | $\uparrow$ |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\hat{F}$ |  | \% | $\uparrow$ |  | \% | $\uparrow$ |  | \% | $\uparrow$ |  |
| Traffic Volume (veh/h) | 26 | 5 | 27 | 22 | 14 | 16 | 5 | 246 | 3 | 2 | 346 | 44 |
| Future Volume (Veh/h) | 26 | 5 | 27 | 22 | 14 | 16 | 5 | 246 | 3 | 2 | 346 | 44 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Hourly flow rate (vph) | 27 | 5 | 28 | 23 | 15 | 17 | 5 | 256 | 3 | 2 | 360 | 46 |
| Pedestrians |  |  |  |  | 1 |  |  | 1 |  |  | 2 |  |
| Lane Width (ft) |  |  |  |  | 12.0 |  |  | 12.0 |  |  | 12.0 |  |
| Walking Speed (fts) |  |  |  |  | 3.5 |  |  | 3.5 |  |  | 3.5 |  |
| Percent Blockage |  |  |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | TWLTL |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  | 2 |  |
| Upstream signal (ft) |  |  |  |  |  |  |  |  |  |  | 978 |  |
| pX, platoon unblocked | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 |  | 0.89 |  |  |  |  |  |
| vC , conflicting volume | 680 | 657 | 384 | 664 | 678 | 260 | 406 |  |  | 260 |  |  |
| VC 1 , stage 1 conf vol | 387 | 387 |  | 268 | 268 |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol | 292 | 270 |  | 396 | 410 |  |  |  |  |  |  |  |
| vCu , unblocked vol | 582 | 557 | 252 | 565 | 581 | 260 | 277 |  |  | 260 |  |  |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| $\mathrm{tC}, 2$ stage (s) | 6.1 | 5.5 |  | 6.1 | 5.5 |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| po queue free \% | 95 | 99 | 96 | 96 | 97 | 98 | 100 |  |  | 100 |  |  |
| cM capacity (veh/h) | 550 | 543 | 696 | 548 | 531 | 769 | 1135 |  |  | 1286 |  |  |
| Direction, Lane \# | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 | SB 1 | SB 2 |  |  |  |  |
| Volume Total | 27 | 33 | 23 | 32 | 5 | 259 | 2 | 406 |  |  |  |  |
| Volume Left | 27 | 0 | 23 | 0 | 5 | 0 | 2 | 0 |  |  |  |  |
| Volume Right | 0 | 28 | 0 | 17 | 0 | 3 | 0 | 46 |  |  |  |  |
| CSH | 550 | 668 | 548 | 635 | 1135 | 1700 | 1286 | 1700 |  |  |  |  |
| Volume to Capacity | 0.05 | 0.05 | 0.04 | 0.05 | 0.00 | 0.15 | 0.00 | 0.24 |  |  |  |  |
| Queue Length 95th (ft) | 4 | 4 | 3 | 4 | 0 | 0 | 0 | 0 |  |  |  |  |
| Control Delay (s) | 11.9 | 10.7 | 11.9 | 11.0 | 8.2 | 0.0 | 7.8 | 0.0 |  |  |  |  |
| Lane LOS | B | B | B | B | A |  | A |  |  |  |  |  |
| Approach Delay (s) | 11.2 |  | 11.3 |  | 0.2 |  | 0.0 |  |  |  |  |  |
| Approach LOS | B |  | B |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 1.7 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 35.8\% | ICU Level of Service |  |  |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |



## Appendix I Year 2021 Total Traffic SimTraffic Worksheets

## Summary of All Intervals

| Run Number | 21 | 22 | 23 | 24 | 25 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Start Time | $7: 10$ | $7: 10$ | $7: 10$ | $7: 10$ | $7: 10$ | $7: 10$ |
| End Time | $8: 20$ | $8: 20$ | $8: 20$ | $8: 20$ | $8: 20$ | $8: 20$ |
| Total Time (min) | 70 | 70 | 70 | 70 | 70 | 70 |
| Time Recorded (min) | 60 | 60 | 60 | 60 | 60 | 60 |
| \# of Intervals | 5 | 5 | 5 | 5 | 5 | 5 |
| \# of Recorded Intervals | 4 | 4 | 4 | 4 | 4 | 4 |
| Vehs Entered | 5193 | 5265 | 5137 | 5179 | 5199 | 5193 |
| Vehs Exited | 5166 | 5175 | 5092 | 5210 | 5161 | 5160 |
| Starting Vehs | 283 | 296 | 334 | 324 | 302 | 302 |
| Ending Vehs | 310 | 386 | 379 | 293 | 340 | 337 |
| Travel Distance (mi) | 7002 | 6994 | 6905 | 6840 | 6948 | 6938 |
| Travel Time (hr) | 343.3 | 340.4 | 349.2 | 307.4 | 363.4 | 340.7 |
| Total Delay (hr) | 148.1 | 145.8 | 157.8 | 116.9 | 170.5 | 147.8 |
| Total Stops | 8334 | 8479 | 8875 | 7095 | 9188 | 8391 |
| Fuel Used (gal) | 254.1 | 253.9 | 253.2 | 244.1 | 259.1 | 252.9 |

## Interval \#0 Information Seeding

| Start Time | $7: 10$ |
| :--- | ---: |
| End Time | $7: 20$ |
| Total Time (min) | 10 |
| Volumes adjusted by Growth Factors. |  |
| No data recorded this interval. |  |

## Interval \#1 Information Recording1

| Start Time | $7: 20$ |
| :--- | ---: | :--- |
| End Time | $7: 35$ |
| Total Time (min) | 15 |
| Volumes adjusted by Growth Factors, Anti PHF. |  |


| Run Number | 21 | 22 | 23 | 24 | 25 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1258 | 1228 | 1175 | 1224 | 1257 | 1226 |
| Vehs Exited | 1227 | 1272 | 1220 | 1288 | 1269 | 1258 |
| Starting Vehs | 283 | 296 | 334 | 324 | 302 | 302 |
| Ending Vehs | 314 | 252 | 289 | 260 | 290 | 276 |
| Travel Distance (mi) | 1734 | 1687 | 1638 | 1652 | 1690 | 1680 |
| Travel Time (hr) | 76.5 | 78.0 | 73.8 | 72.8 | 75.9 | 75.4 |
| Total Delay (hr) | 28.5 | 31.3 | 28.6 | 26.8 | 29.0 | 28.8 |
| Total Stops | 1714 | 1785 | 1630 | 1577 | 1742 | 1689 |
| Fuel Used (gal) | 61.3 | 60.9 | 58.1 | 58.9 | 60.9 | 60.0 |

SimTraffic Simulation Summary
Year 2021 Total Traffic - Cul-de-sac AM Peak Hour Conditions
Interval \#2 Information Recording1

| Start Time | $7: 35$ |
| :--- | ---: |
| End Time | $7: 50$ |
| Total Time (min) | 15 |
| Volumes adjusted by PHF, Growth Factors. |  |


| Run Number | 21 | 22 | 23 | 24 | 25 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1466 | 1446 | 1418 | 1437 | 1484 | 1445 |
| Vehs Exited | 1432 | 1350 | 1277 | 1329 | 1351 | 1348 |
| Starting Vehs | 314 | 252 | 289 | 260 | 290 | 276 |
| Ending Vehs | 348 | 348 | 430 | 368 | 423 | 381 |
| Travel Distance (mi) | 1857 | 1808 | 1820 | 1781 | 1812 | 1816 |
| Travel Time (hr) | 89.7 | 84.1 | 90.3 | 81.9 | 93.6 | 87.9 |
| Total Delay (hr) | 37.5 | 33.3 | 39.9 | 32.3 | 42.9 | 37.2 |
| Total Stops | 2219 | 2074 | 2258 | 2002 | 2436 | 2195 |
| Fuel Used (gal) | 67.2 | 64.6 | 65.7 | 63.6 | 67.4 | 65.7 |

Interval \#3 Information Recording1

| Start Time | $7: 50$ |
| :--- | ---: | :--- |
| End Time | $8: 05$ |
| Total Time (min) | 15 |
| Volumes adjusted by Growth Factors, Anti PHF. |  |


| Run Number | 21 | 22 | 23 | 24 | 25 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1288 | 1292 | 1274 | 1259 | 1241 | 1268 |
| Vehs Exited | 1275 | 1309 | 1340 | 1360 | 1246 | 1304 |
| Starting Vehs | 348 | 348 | 430 | 368 | 423 | 381 |
| Ending Vehs | 361 | 331 | 364 | 267 | 418 | 342 |
| Travel Distance (mi) | 1758 | 1768 | 1762 | 1761 | 1747 | 1759 |
| Travel Time (hr) | 90.0 | 87.6 | 97.1 | 79.7 | 97.2 | 90.3 |
| Total Delay (hr) | 40.9 | 38.4 | 48.1 | 30.5 | 48.9 | 41.4 |
| Total Stops | 2196 | 2215 | 2610 | 1863 | 2477 | 2272 |
| Fuel Used (gal) | 64.0 | 64.3 | 66.5 | 63.0 | 65.6 | 64.7 |

## Interval \#4 Information Recording1

| Start Time | $8: 05$ |
| :--- | ---: |
| End Time | $8: 20$ |
| Total Time (min) | 15 |
| Volumes adjusted by Growth Factors, Anti PHF. |  |


| Run Number | 21 | 22 | 23 | 24 | 25 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1181 | 1299 | 1270 | 1259 | 1217 | 1241 |
| Vehs Exited | 1232 | 1244 | 1255 | 1233 | 1295 | 1251 |
| Starting Vehs | 361 | 331 | 364 | 267 | 418 | 342 |
| Ending Vehs | 310 | 386 | 379 | 293 | 340 | 337 |
| Travel Distance (mi) | 1653 | 1731 | 1685 | 1646 | 1699 | 1683 |
| Travel Time (hr) | 87.1 | 90.7 | 88.1 | 73.0 | 96.6 | 87.1 |
| Total Delay (hr) | 41.2 | 42.8 | 41.1 | 27.4 | 49.8 | 40.5 |
| Total Stops | 2205 | 2405 | 2377 | 1653 | 2533 | 2233 |
| Fuel Used (gal) | 61.5 | 64.0 | 62.9 | 58.5 | 65.3 | 62.4 |

Intersection: 4: Cipole Rd \& Tualatin-Sherwood Rd

| Movement | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | T | R | L | TR | L | TR |
| Maximum Queue (ft) | 328 | 979 | 203 | 354 | 135 | 95 | 68 | 234 | 225 |
| Average Queue (ft) | 101 | 536 | 76 | 72 | 15 | 26 | 20 | 101 | 66 |
| 95th Queue (ft) | 316 | 1209 | 153 | 219 | 68 | 68 | 53 | 246 | 316 |
| Link Distance (ft) |  | 1104 |  | 819 |  |  | 419 | 805 |  |
| Upstream Blk Time (\%) |  | 1 |  |  |  |  |  |  | 0 |
| Queuing Penalty (veh) |  | 10 |  |  |  |  |  |  | 0 |
| Storage Bay Dist (ft) | 360 |  | 250 |  | 130 | 200 |  | 300 | 7 |
| Storage Blk Time (\%) |  | 14 | 0 | 2 | 0 |  |  | 0 | 0 |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | R | L | T | R | L | TR | L | T | R |
| Maximum Queue (ft) | 399 | 833 | 400 | 180 | 666 | 400 | 338 | 533 | 316 | 394 | 118 |
| Average Queue (ft) | 85 | 640 | 56 | 32 | 310 | 84 | 144 | 303 | 184 | 147 | 42 |
| 95th Queue (ft) | 289 | 975 | 261 | 110 | 581 | 293 | 319 | 595 | 318 | 313 | 92 |
| Link Distance (ft) |  | 819 |  |  | 1233 |  |  | 922 |  | 1894 | 1894 |
| Upstream Blk Time (\%) |  | 4 |  |  |  |  |  | 1 |  |  |  |
| Queuing Penalty (veh) |  | 47 |  |  |  |  |  | 3 |  |  |  |
| Storage Bay Dist (ft) | 375 |  | 375 | 375 |  | 375 | 400 |  | 300 |  |  |
| Storage Blk Time (\%) | 0 | 26 | 0 | 0 | 5 | 0 | 0 | 6 | 4 | 0 |  |
| Queuing Penalty (veh) | 0 | 31 | 1 | 0 | 11 | 0 | 0 | 8 | 6 | 0 |  |

Intersection: 11: 124th Ave \& Blake Road

| Movement | EB | EB | WB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | TR | L |
| Maximum Queue (ft) | 59 | 79 | 43 | 47 | 64 | 46 |
| Average Queue (ft) | 15 | 26 | 3 | 7 | 8 | 5 |
| 95th Queue (ft) | 43 | 63 | 21 | 31 | 70 | 23 |
| Link Distance (ft) |  | 807 |  | 1348 | 1018 |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  | 150 |  |  | 150 |
| Storage Bay Dist (ft) | 150 |  |  |  | 0 |  |

Intersection: 12: Blake Road \& Cipole Road

```
Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream BIk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (tt)
Storage BIk Time (%)
Queuing Penalty (veh)
```

Zone Summary
Zone wide Queuing Penalty: 140

## Summary of All Intervals

| Run Number | 31 | 32 | 33 | 34 | 35 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Start Time | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ |
| End Time | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ |
| Total Time (min) | 70 | 70 | 70 | 70 | 70 | 70 |
| Time Recorded (min) | 60 | 60 | 60 | 60 | 60 | 60 |
| \# of Intervals | 5 | 5 | 5 | 5 | 5 | 5 |
| \# of Recorded Intervals | 4 | 4 | 4 | 4 | 4 | 4 |
| Vehs Entered | 5722 | 5870 | 5837 | 5863 | 5735 | 5803 |
| Vehs Exited | 5690 | 5795 | 5777 | 5818 | 5668 | 5750 |
| Starting Vehs | 346 | 394 | 345 | 383 | 364 | 362 |
| Ending Vehs | 378 | 469 | 405 | 428 | 431 | 417 |
| Travel Distance (mi) | 7959 | 8121 | 8206 | 8155 | 7908 | 8070 |
| Travel Time (hr) | 399.8 | 445.2 | 435.2 | 433.0 | 392.4 | 421.1 |
| Total Delay (hr) | 177.0 | 217.4 | 205.5 | 204.5 | 171.2 | 195.1 |
| Total Stops | 9931 | 10707 | 10745 | 10277 | 9772 | 10283 |
| Fuel Used (gal) | 290.9 | 303.1 | 303.2 | 301.3 | 286.8 | 297.0 |

## Interval \#0 Information Seeding

| Start Time | $4: 45$ |
| :--- | ---: |
| End Time | $4: 55$ |
| Total Time (min) | 10 |
| Volumes adjusted by Growth Factors. |  |
| No data recorded this interval. |  |

Interval \#1 Information Recording1

| Start Time | $4: 55$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $5: 10$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number | 31 | 32 | 33 | 34 | 35 | Avg |
| Vehs Entered | 1447 | 1433 | 1452 | 1452 | 1417 | 1440 |
| Vehs Exited | 1377 | 1412 | 1430 | 1432 | 1431 | 1414 |
| Starting Vehs | 346 | 394 | 345 | 383 | 364 | 362 |
| Ending Vehs | 416 | 415 | 367 | 403 | 350 | 384 |
| Travel Distance (mi) | 1973 | 1956 | 2002 | 2038 | 1956 | 1985 |
| Travel Time (hr) | 97.1 | 93.3 | 94.9 | 101.3 | 89.1 | 95.1 |
| Total Delay (hr) | 41.8 | 38.4 | 38.7 | 44.1 | 34.1 | 39.4 |
| Total Stops | 2481 | 2332 | 2292 | 2656 | 2212 | 2395 |
| Fuel Used (gal) | 71.3 | 70.1 | 72.2 | 73.9 | 69.1 | 71.3 |

SimTraffic Simulation Summary
Year 2021 Total Traffic Culdesac PM Peak Hour Conditions
Interval \#2 Information Recording2

| Start Time | $5: 10$ |
| :--- | ---: | :--- |
| End Time | $5: 25$ |
| Total Time $(\min )$ | 15 |
| Volumes adjusted by PHF, Growth Factors. |  |


| Run Number | 31 | 32 | 33 | 34 | 35 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1504 | 1555 | 1580 | 1551 | 1487 | 1536 |
| Vehs Exited | 1481 | 1468 | 1441 | 1505 | 1438 | 1465 |
| Starting Vehs | 416 | 415 | 367 | 403 | 350 | 384 |
| Ending Vehs | 439 | 502 | 506 | 449 | 399 | 460 |
| Travel Distance (mi) | 2046 | 2127 | 2145 | 2096 | 2004 | 2084 |
| Travel Time (hr) | 110.5 | 118.0 | 113.7 | 113.2 | 97.1 | 110.5 |
| Total Delay (hr) | 53.5 | 58.5 | 54.1 | 54.7 | 41.0 | 52.4 |
| Total Stops | 2728 | 3040 | 2810 | 2872 | 2426 | 2776 |
| Fuel Used (gal) | 76.2 | 79.4 | 78.8 | 78.0 | 72.0 | 76.9 |

## Interval \#3 Information Recording2

| Start Time | $5: 25$ |
| :--- | ---: | :--- |
| End Time | $5: 40$ |
| Total Time (min) | 15 |
| Volumes adjusted by Growth Factors, Anti PHF. |  |


| Run Number | 31 | 32 | 33 | 34 | 35 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1354 | 1421 | 1390 | 1418 | 1416 | 1400 |
| Vehs Exited | 1462 | 1500 | 1471 | 1475 | 1366 | 1455 |
| Starting Vehs | 439 | 502 | 506 | 449 | 399 | 460 |
| Ending Vehs | 331 | 423 | 425 | 392 | 449 | 396 |
| Travel Distance (mi) | 1995 | 2026 | 2051 | 1983 | 1944 | 2000 |
| Travel Time (hr) | 97.1 | 117.3 | 117.5 | 106.2 | 96.4 | 106.9 |
| Total Delay (hr) | 41.1 | 60.4 | 60.0 | 50.5 | 42.1 | 50.8 |
| Total Stops | 2331 | 2610 | 2980 | 2309 | 2406 | 2527 |
| Fuel Used (gal) | 72.3 | 76.5 | 77.3 | 73.8 | 70.5 | 74.1 |

## Interval \#4 Information Recording2

| Start Time | 5:40 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| End Time | 5:55 |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number | 31 | 32 | 33 | 34 | 35 | Avg |
| Vehs Entered | 1417 | 1461 | 1415 | 1442 | 1415 | 1427 |
| Vehs Exited | 1370 | 1415 | 1435 | 1406 | 1433 | 1409 |
| Starting Vehs | 331 | 423 | 425 | 392 | 449 | 396 |
| Ending Vehs | 378 | 469 | 405 | 428 | 431 | 417 |
| Travel Distance (mi) | 1945 | 2012 | 2009 | 2038 | 2003 | 2001 |
| Travel Time (hr) | 95.0 | 116.6 | 109.1 | 112.3 | 109.8 | 108.6 |
| Total Delay (hr) | 40.6 | 60.1 | 52.7 | 55.3 | 54.0 | 52.5 |
| Total Stops | 2391 | 2725 | 2663 | 2440 | 2728 | 2591 |
| Fuel Used (gal) | 71.0 | 77.0 | 74.9 | 75.6 | 75.2 | 74.7 |

## Intersection: 4: Cipole Rd \& Tualatin-Sherwood Rd

| Movement | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | T | R | L | TR | L | TR |
| Maximum Queue (ft) | 220 | 727 | 189 | 787 | 130 | 170 | 138 | 136 | 174 |
| Average Queue (ft) | 33 | 265 | 29 | 402 | 7 | 77 | 57 | 51 | 76 |
| 95th Queue (ft) | 113 | 598 | 127 | 756 | 47 | 146 | 113 | 107 | 147 |
| Link Distance (ft) |  | 1103 |  | 819 |  |  | 440 | 805 |  |
| Upstream Blk Time (\%) |  |  |  | 0 |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  | 3 |  |  |  |  |  |
| Storage Bay Dist (ft) | 360 |  | 250 |  | 130 | 200 |  | 300 |  |
| Storage Blk Time (\%) |  | 5 | 0 | 20 | 0 | 0 |  |  |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| SB |  |  |  |  |  |  |  |  |  |  |
| irections Served | L | T | R | L | T | R | L | TR | L | T |
| Maximum Queue (ft) | 124 | 787 | 400 | 399 | 958 | 400 | 207 | 213 | 320 | 434 |
| Average Queue (ft) | 53 | 409 | 117 | 71 | 542 | 99 | 87 | 111 | 192 | 178 |
| 95th Queue (ft) | 112 | 738 | 391 | 262 | 1020 | 359 | 165 | 187 | 323 | 333 |
| Link Distance (ft) |  | 819 |  |  | 1233 |  |  | 892 |  | 1894 |
| Upstream Blk Time (\%) |  | 0 |  |  | 0 |  |  |  |  |  |
| Queuing Penalty (veh) |  | 2 |  |  | 3 |  |  |  |  |  |
| Storage Bay Dist (ft) | 100 |  | 375 | 375 |  | 375 | 400 |  | 300 |  |
| Storage BIk Time (\%) | 1 | 27 | 0 |  | 17 | 0 |  |  | 4 | 0 |
| Queuing Penalty (veh) | 14 | 50 | 1 |  | 30 | 0 |  |  | 8 | 1 |

Intersection: 11: 124th Ave \& Blake Road

| Movement | EB | EB | WB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | TR | TR |
| Maximum Queue (ft) | 50 | 59 | 36 | 58 | 14 | 7 |
| Average Queue (ft) | 11 | 15 | 14 | 22 | 1 | 0 |
| 95th Queue (ft) | 37 | 43 | 38 | 51 | 11 | 5 |
| Link Distance (ft) |  | 787 |  | 949 | 1716 | 892 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  | 150 |  |  |  |
| Storage Bay Dist (ft) | 150 |  |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |  |  |

## Intersection: 12: Blake Road \& Cipole Road

```
Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream BIk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (tt)
Storage BIk Time (%)
Queuing Penalty (veh)
```

Zone Summary
Zone wide Queuing Penalty: 120

## Summary of All Intervals

| Start Time | $7: 10$ |
| :--- | ---: |
| End Time | $8: 20$ |
| Total Time (min) | 70 |
| Time Recorded (min) | 60 |
| \# of Intervals | 5 |
| \# of Recorded Intervals | 4 |
| Vehs Entered | 5199 |
| Vehs Exited | 5163 |
| Starting Vehs | 281 |
| Ending Vehs | 317 |
| Travel Distance (mi) | 6824 |
| Travel Time (hr) | 325.0 |
| Total Delay (hr) | 135.2 |
| Total Stops | 8132 |
| Fuel Used (gal) | 245.7 |

## Interval \#0 Information Seeding

| Start Time | $7: 10$ |
| :--- | ---: | :--- |
| End Time | $7: 20$ |
| Total Time (min) | 10 |
| Volumes adjusted by Growth Factors. |  |
| No data recorded this interval. |  |

Interval \#1 Information Recording1

| Start Time | $7: 20$ |
| :--- | ---: |
| End Time | $7: 35$ |
| Total Time (min) |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |
| Vehs Entered | 1250 |
| Vehs Exited | 1270 |
| Starting Vehs | 281 |
| Ending Vehs | 261 |
| Travel Distance (mi) | 1653 |
| Travel Time (hr) | 72.4 |
| Total Delay (hr) | 26.4 |
| Total Stops | 1705 |
| Fuel Used (gal) | 58.8 |

SimTraffic Simulation Summary
Year 2021 Total Traffic - Cipole Extension AM Peak Hour Conditions
Interval \#2 Information Recording2

| Start Time | $7: 35$ |
| :--- | ---: |
| End Time | $7: 50$ |
| Total Time (min) |  |
| Volumes adjusted by PHF, Growth Factors. |  |
| Vehs Entered |  |
| Vehs Exited | 1457 |
| Starting Vehs | 1333 |
| Ending Vehs | 261 |
| Travel Distance (mi) | 385 |
| Travel Time (hr) | 1786 |
| Total Delay (hr) | 85.3 |
| Total Stops | 35.5 |
| Fuel Used (gal) | 2256 |

Interval \#3 Information Recording3

| Start Time | $7: 50$ |
| :--- | ---: |
| End Time | $8: 05$ |
| Total Time (min) | 15 |
| Volumes adjusted by Growth Factors, Anti PHF. |  |
| Vehs Entered | 1217 |
| Vehs Exited | 1288 |
| Starting Vehs | 385 |
| Ending Vehs | 314 |
| Travel Distance (mi) | 1667 |
| Travel Time (hr) | 84.7 |
| Total Delay (hr) | 38.4 |
| Total Stops | 2146 |
| Fuel Used (gal) | 61.1 |

Interval \#4 Information Recording4

| Start Time | $8: 05$ |
| :--- | ---: |
| End Time | $8: 20$ |
| Total Time (min) | 15 |
| Volumes adjusted by Growth Factors, Anti PHF. |  |
| Vehs Entered |  |
| Vehs Exited | 1275 |
| Starting Vehs | 1272 |
| Ending Vehs | 314 |
| Travel Distance (mi) | 317 |
| Travel Time (hr) | 1717 |
| Total Delay (hr) | 82.6 |
| Total Stops | 34.8 |
| Fuel Used (gal) | 2025 |

Intersection: 4: Cipole Rd \& Tualatin-Sherwood Rd

| Movement | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | T | R | L | TR | L | TR |
| Maximum Queue (ft) | 385 | 1104 | 173 | 318 | 155 | 51 | 51 | 215 | 65 |
| Average Queue (ft) | 129 | 512 | 47 | 58 | 18 | 21 | 17 | 74 | 21 |
| 95th Queue (ft) | 374 | 1225 | 113 | 171 | 82 | 54 | 45 | 174 | 51 |
| Link Distance (ft) |  | 1104 |  | 819 |  |  | 419 | 805 |  |
| Upstream Blk Time (\%) |  | 0 |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  | 2 |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 360 |  | 250 |  | 130 | 200 |  | 300 |  |
| Storage Blk Time (\%) | 0 | 14 |  | 1 | 0 |  |  |  |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | T | R | L | T | R | L | TR | L | T | R |
| Maximum Queue (ft) | 400 | 837 | 400 | 399 | 534 | 400 | 424 | 730 | 282 | 260 | 85 |
| Average Queue (ft) | 128 | 675 | 80 | 33 | 267 | 104 | 128 | 315 | 160 | 116 | 32 |
| 95th Queue (ft) | 388 | 970 | 327 | 154 | 459 | 333 | 340 | 525 | 267 | 200 | 73 |
| Link Distance (ft) |  | 819 |  |  | 1233 |  |  | 922 |  | 1894 | 1894 |
| Upstream Blk Time (\%) |  | 6 |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  | 62 |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 375 |  | 375 | 375 |  | 375 | 400 |  | 300 |  |  |
| Storage Blk Time (\%) | 0 | 29 | 0 | 0 | 3 | 0 |  | 7 | 0 |  |  |
| Queuing Penalty (veh) | 0 | 33 | 0 | 0 | 6 | 0 |  | 9 | 0 |  |  |

Intersection: 11: 124th Ave \& Blake Road

| Movement | EB | EB | WB | WB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L |
| Maximum Queue (ft) | 50 | 70 | 30 | 31 | 49 |
| Average Queue (ft) | 19 | 29 | 2 | 6 | 4 |
| 95th Queue (ft) | 45 | 61 | 15 | 25 | 19 |
| Link Distance (ft) |  | 807 |  | 1348 |  |
| Upstream Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  | 150 |  |  |
| Storage Bay Dist (ft) | 150 |  |  |  |  |

Intersection: 12: Blake Road \& Cipole Road

| Movement | SB |
| :--- | :---: |
| Directions Served | LR |
| Maximum Queue (ft) | 41 |
| Average Queue (ft) | 13 |
| 95th Queue (ft) | 35 |
| Link Distance (ft) | 546 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Zone Summary

[^13]
## Summary of All Intervals

| Run Number | 1 | 2 | 3 | 4 | 5 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Start Time | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ |
| End Time | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ |
| Total Time (min) | 70 | 70 | 70 | 70 | 70 | 70 |
| Time Recorded (min) | 60 | 60 | 60 | 60 | 60 | 60 |
| \# of Intervals | 5 | 5 | 5 | 5 | 5 | 5 |
| \# of Recorded Intervals | 4 | 4 | 4 | 4 | 4 | 4 |
| Vehs Entered | 5767 | 5907 | 5813 | 5697 | 5821 | 5797 |
| Vehs Exited | 5754 | 5827 | 5786 | 5572 | 5760 | 5742 |
| Starting Vehs | 335 | 385 | 336 | 339 | 352 | 348 |
| Ending Vehs | 348 | 465 | 363 | 464 | 413 | 411 |
| Travel Distance (mi) | 7900 | 8090 | 7988 | 7769 | 8069 | 7963 |
| Travel Time (hr) | 382.6 | 445.5 | 382.8 | 369.7 | 406.0 | 397.3 |
| Total Delay (hr) | 161.1 | 219.2 | 159.8 | 152.2 | 179.9 | 174.4 |
| Total Stops | 8972 | 11534 | 9568 | 8959 | 9404 | 9687 |
| Fuel Used (gal) | 284.0 | 302.5 | 286.3 | 276.8 | 293.5 | 288.6 |

## Interval \#0 Information Seeding

| Start Time | $4: 45$ |
| :--- | ---: |
| End Time | $4: 55$ |
| Total Time (min) | 10 |
| Volumes adjusted by Growth Factors. |  |
| No data recorded this interval. |  |

Interval \#1 Information Recording

| Start Time | 4:55 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| End Time | 5:10 |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number | 1 | 2 | 3 | 4 | 5 | Avg |
| Vehs Entered | 1392 | 1442 | 1451 | 1332 | 1406 | 1403 |
| Vehs Exited | 1387 | 1438 | 1407 | 1367 | 1396 | 1398 |
| Starting Vehs | 335 | 385 | 336 | 339 | 352 | 348 |
| Ending Vehs | 340 | 389 | 380 | 304 | 362 | 351 |
| Travel Distance (mi) | 1961 | 1985 | 1937 | 1850 | 1973 | 1941 |
| Travel Time (hr) | 87.2 | 96.0 | 90.3 | 82.5 | 89.0 | 89.0 |
| Total Delay (hr) | 32.1 | 40.4 | 36.2 | 30.7 | 34.0 | 34.7 |
| Total Stops | 2059 | 2497 | 2293 | 2002 | 2117 | 2190 |
| Fuel Used (gal) | 68.9 | 71.8 | 68.4 | 65.4 | 69.2 | 68.7 |

SimTraffic Simulation Summary
Year 2021 Total Traffic Cipole Extension PM Peak Hour Conditions
Interval \#2 Information Recording

| Start Time | $5: 10$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $5: 25$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by PHF, Growth Factors. |  |  |  |  |  |  |
| Run Number |  |  |  |  |  |  |
| Vehs Entered | 1641 | 1581 | 1543 | 1480 | 1604 | 1565 |
| Vehs Exited | 1516 | 1464 | 1477 | 1387 | 1495 | 1468 |
| Starting Vehs | 340 | 389 | 380 | 304 | 362 | 351 |
| Ending Vehs | 465 | 506 | 446 | 397 | 471 | 455 |
| Travel Distance (mi) | 2080 | 2048 | 2098 | 1947 | 2065 | 2048 |
| Travel Time (hr) | 104.1 | 107.1 | 102.8 | 88.2 | 104.9 | 101.4 |
| Total Delay (hr) | 45.9 | 49.9 | 44.4 | 33.8 | 46.8 | 44.1 |
| Total Stops | 2551 | 2797 | 2689 | 2216 | 2613 | 2569 |
| Fuel Used (gal) | 74.8 | 75.4 | 75.5 | 67.9 | 75.9 | 73.9 |

Interval \#3 Information Recording

| Start Time | 5:25 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| End Time | 5:40 |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number | 1 | 2 | 3 | 4 | 5 | Avg |
| Vehs Entered | 1361 | 1399 | 1378 | 1386 | 1407 | 1385 |
| Vehs Exited | 1470 | 1454 | 1480 | 1432 | 1502 | 1466 |
| Starting Vehs | 465 | 506 | 446 | 397 | 471 | 455 |
| Ending Vehs | 356 | 451 | 344 | 351 | 376 | 372 |
| Travel Distance (mi) | 1997 | 2024 | 2033 | 1954 | 2063 | 2014 |
| Travel Time (hr) | 103.4 | 119.5 | 98.9 | 91.5 | 109.6 | 104.6 |
| Total Delay (hr) | 47.5 | 62.8 | 42.2 | 36.8 | 52.0 | 48.3 |
| Total Stops | 2446 | 3169 | 2369 | 2148 | 2571 | 2536 |
| Fuel Used (gal) | 73.4 | 77.3 | 73.4 | 69.9 | 76.6 | 74.1 |

## Interval \#4 Information Recording

| Start Time | $5: 40$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $5: 55$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number | 1 | 2 | 3 | 4 | 5 | Avg |
| Vehs Entered | 1373 | 1485 | 1441 | 1499 | 1404 | 1437 |
| Vehs Exited | 1381 | 1471 | 1422 | 1386 | 1367 | 1405 |
| Starting Vehs | 356 | 451 | 344 | 351 | 376 | 372 |
| Ending Vehs | 348 | 465 | 363 | 464 | 413 | 411 |
| Travel Distance (mi) | 1863 | 2034 | 1920 | 2018 | 1968 | 1961 |
| Travel Time (hr) | 87.9 | 122.8 | 90.8 | 107.5 | 102.5 | 102.3 |
| Total Delay (hr) | 35.6 | 66.0 | 37.0 | 51.0 | 47.1 | 47.3 |
| Total Stops | 1916 | 3071 | 2217 | 2593 | 2103 | 2383 |
| Fuel Used (gal) | 66.9 | 77.9 | 69.1 | 73.7 | 71.8 | 71.9 |

Intersection: 4: Cipole Rd \& Tualatin-Sherwood Rd

| Movement | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | T | R | L | TR | L | TR |
| Maximum Queue (ft) | 201 | 501 | 86 | 748 | 122 | 135 | 100 | 132 | 157 |
| Average Queue (ft) | 33 | 170 | 13 | 320 | 11 | 52 | 38 | 47 | 65 |
| 95th Queue (ft) | 95 | 391 | 69 | 653 | 70 | 104 | 80 | 98 | 130 |
| Link Distance (ft) |  | 1103 |  | 819 |  |  | 440 | 805 |  |
| Upstream Blk Time (\%) |  |  |  | 0 |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  | 3 |  |  |  |  |  |
| Storage Bay Dist (ft) | 360 |  | 250 |  | 130 | 200 |  | 300 |  |
| Storage Blk Time (\%) |  | 1 |  | 16 | 0 |  |  |  |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | R | L | T | R | L | TR | L | T | R |
| Maximum Queue (ft) | 332 | 722 | 400 | 400 | 914 | 399 | 168 | 211 | 296 | 312 | 254 |
| Average Queue (ft) | 61 | 309 | 65 | 79 | 456 | 84 | 77 | 111 | 166 | 157 | 107 |
| 95th Queue (ft) | 203 | 558 | 271 | 279 | 976 | 327 | 136 | 196 | 270 | 264 | 206 |
| Link Distance (ft) |  | 819 |  |  | 1233 |  |  | 892 |  | 1894 | 1894 |
| Upstream Blk Time (\%) |  | 0 |  |  | 1 |  |  |  |  |  |  |
| Queuing Penalty (veh) |  | 0 |  |  | 13 |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 375 |  | 375 | 375 |  | 375 | 400 |  | 300 | 1 | 0 |
| Storage BIk Time (\%) |  | 5 | 0 |  | 13 | 0 |  |  | 1 | 1 |  |
| Queuing Penalty (veh) |  | 7 | 0 |  | 22 | 0 |  |  |  |  |  |

Intersection: 11: 124th Ave \& Blake Road

| Movement | EB | EB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | TR | L | TR |
| Maximum Queue (ft) | 43 | 65 | 50 | 67 | 10 | 11 | 6 | 6 |
| Average Queue (ft) | 17 | 22 | 17 | 22 | 1 | 1 | 0 | 0 |
| 95th Queue (ft) | 42 | 50 | 43 | 54 | 8 | 6 | 4 | 4 |
| Link Distance (ft) |  | 787 |  | 949 |  | 1716 |  | 892 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  | 150 |  | 150 |  | 150 |  |
| Storage Bay Dist (ft) | 150 |  | 150 |  |  |  |  |  |

Intersection: 12: Blake Road \& Cipole Road

| Movement | EB | SB |
| :--- | ---: | ---: |
| Directions Served | L | LR |
| Maximum Queue (ft) | 6 | 69 |
| Average Queue (ft) | 0 | 31 |
| 95th Queue (ft) | 5 | 57 |
| Link Distance (ft) |  | 300 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) | 150 |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

Zone Summary
Zone wide Queuing Penalty: 54

## Appendix J Year 2025 Total Traffic - SimTraffic Worksheets

SimTraffic Simulation Summary
Year 2025 Total Traffic Culdesac AM Peak Hour Conditions
Summary of All Intervals

| Run Number | 71 | 72 | 73 | 74 | 75 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Start Time | $7: 10$ | $7: 10$ | $7: 10$ | $7: 10$ | $7: 10$ | $7: 10$ |
| End Time | $8: 20$ | $8: 20$ | $8: 20$ | $8: 20$ | $8: 20$ | $8: 20$ |
| Total Time (min) | 70 | 70 | 70 | 70 | 70 | 70 |
| Time Recorded (min) | 60 | 60 | 60 | 60 | 60 | 60 |
| \# of Intervals | 5 | 5 | 5 | 5 | 5 | 5 |
| \# of Recorded Intervals | 4 | 4 | 4 | 4 | 4 | 4 |
| Vehs Entered | 5476 | 5452 | 5576 | 5598 | 5534 | 5526 |
| Vehs Exited | 5465 | 5477 | 5517 | 5597 | 5579 | 5525 |
| Starting Vehs | 294 | 302 | 286 | 310 | 317 | 301 |
| Ending Vehs | 305 | 277 | 345 | 311 | 272 | 298 |
| Travel Distance (mi) | 7498 | 7427 | 7652 | 7601 | 7594 | 7554 |
| Travel Time (hr) | 300.1 | 297.6 | 307.0 | 305.6 | 306.2 | 303.3 |
| Total Delay (hr) | 93.0 | 91.0 | 95.1 | 94.8 | 96.2 | 94.0 |
| Total Stops | 6825 | 6900 | 7085 | 6920 | 7074 | 6960 |
| Fuel Used (gal) | 267.4 | 263.8 | 273.4 | 272.0 | 272.5 | 269.8 |

Interval \#0 Information Seeding

| Start Time | $7: 10$ |
| :--- | ---: |
| End Time | $7: 20$ |
| Total Time (min) | 10 |
| Volumes adjusted by Growth Factors. |  |
| No data recorded this interval. |  |

Interval \#1 Information Recording1

| Start Time | $7: 20$ |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $7: 35$ |  |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |  |
| Run Number | 71 | 72 | 73 | 74 | 75 | Avg |  |
| Vehs Entered | 1293 | 1273 | 1267 | 1325 | 1287 | 1289 |  |
| Vehs Exited | 1329 | 1259 | 1258 | 1339 | 1314 | 1299 |  |
| Starting Vehs | 294 | 302 | 286 | 310 | 317 | 301 |  |
| Ending Vehs | 258 | 316 | 295 | 296 | 290 | 289 |  |
| Travel Distance (mi) | 1846 | 1739 | 1823 | 1855 | 1808 | 1814 |  |
| Travel Time (hr) | 72.4 | 68.8 | 72.9 | 71.9 | 71.5 | 71.5 |  |
| Total Delay (hr) | 21.9 | 20.4 | 22.8 | 20.9 | 21.6 | 21.5 |  |
| Total Stops | 1604 | 1596 | 1628 | 1494 | 1649 | 1589 |  |
| Fuel Used (gal) | 65.5 | 61.5 | 64.4 | 65.3 | 64.8 | 64.3 |  |

SimTraffic Simulation Summary
Year 2025 Total Traffic Culdesac AM Peak Hour Conditions
Interval \#2 Information Recording2

| Start Time | $7: 35$ |
| :--- | ---: |
| End Time | $7: 50$ |
| Total Time (min) | 15 |
| Volumes adjusted by PHF, Growth Factors. |  |


| Run Number | 71 | 72 | 73 | 74 | 75 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1545 | 1563 | 1614 | 1617 | 1602 | 1586 |
| Vehs Exited | 1473 | 1535 | 1591 | 1554 | 1511 | 1529 |
| Starting Vehs | 258 | 316 | 295 | 296 | 290 | 289 |
| Ending Vehs | 330 | 344 | 318 | 359 | 381 | 346 |
| Travel Distance (mi) | 2019 | 2043 | 2118 | 2129 | 2028 | 2067 |
| Travel Time (hr) | 84.6 | 83.4 | 88.0 | 88.8 | 83.6 | 85.7 |
| Total Delay (hr) | 28.4 | 26.4 | 29.0 | 29.7 | 27.2 | 28.2 |
| Total Stops | 1967 | 1987 | 2051 | 2012 | 1990 | 2003 |
| Fuel Used (gal) | 73.0 | 72.5 | 77.4 | 77.0 | 73.0 | 74.6 |

## Interval \#3 Information Recording3

| Start Time | $7: 50$ |
| :--- | ---: | :--- |
| End Time | $8: 05$ |
| Total Time (min) | 15 |
| Volumes adjusted by Growth Factors, Anti PHF. |  |


| Run Number | 71 | 72 | 73 | 74 | 75 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1317 | 1361 | 1324 | 1342 | 1332 | 1331 |
| Vehs Exited | 1356 | 1397 | 1378 | 1421 | 1461 | 1404 |
| Starting Vehs | 330 | 344 | 318 | 359 | 381 | 346 |
| Ending Vehs | 291 | 308 | 264 | 280 | 252 | 277 |
| Travel Distance (mi) | 1797 | 1907 | 1857 | 1823 | 1937 | 1864 |
| Travel Time (hr) | 72.1 | 77.7 | 71.7 | 74.5 | 78.6 | 74.9 |
| Total Delay (hr) | 22.5 | 24.7 | 20.2 | 23.7 | 25.0 | 23.2 |
| Total Stops | 1674 | 1830 | 1612 | 1738 | 1730 | 1715 |
| Fuel Used (gal) | 64.1 | 68.4 | 65.7 | 66.1 | 69.4 | 66.7 |

## Interval \#4 Information Recording4

| Start Time | $8: 05$ |
| :--- | ---: | :--- |
| End Time | $8: 20$ |
| Total Time (min) | 15 |
| Volumes adjusted by Growth Factors, Anti PHF. |  |


| Run Number | 71 | 72 | 73 | 74 | 75 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1321 | 1255 | 1371 | 1314 | 1313 | 1311 |
| Vehs Exited | 1307 | 1286 | 1290 | 1283 | 1293 | 1292 |
| Starting Vehs | 291 | 308 | 264 | 280 | 252 | 277 |
| Ending Vehs | 305 | 277 | 345 | 311 | 272 | 298 |
| Travel Distance (mi) | 1836 | 1738 | 1854 | 1794 | 1822 | 1809 |
| Travel Time (hr) | 71.0 | 67.7 | 74.5 | 70.4 | 72.5 | 71.2 |
| Total Delay (hr) | 20.3 | 19.5 | 23.0 | 20.5 | 22.4 | 21.1 |
| Total Stops | 1580 | 1487 | 1794 | 1676 | 1705 | 1649 |
| Fuel Used (gal) | 64.8 | 61.4 | 66.0 | 63.6 | 65.3 | 64.2 |

## Intersection: 4: Cipole Rd \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | T | TR | L | TR | L | TR |
| Maximum Queue (ft) | 120 | 198 | 280 | 139 | 215 | 222 | 73 | 55 | 165 | 84 |
| Average Queue (ft) | 43 | 68 | 95 | 46 | 56 | 70 | 18 | 17 | 55 | 28 |
| 95th Queue (ft) | 92 | 162 | 204 | 94 | 140 | 156 | 53 | 47 | 129 | 64 |
| Link Distance (ft) |  | 1102 | 1102 |  | 813 | 813 |  | 401 |  | 800 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  | 250 |  |  | 200 |  | 300 |  |
| Storage Bay Dist (ft) | 360 |  |  |  | 0 |  |  |  |  |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | EB | EB | WB | WB | WB | WB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | L | T | T | $R$ | R | L | T | T | $R$ | L | L |
| Maximum Queue (ft) | 91 | 206 | 384 | 440 | 337 | 67 | 110 | 319 | 315 | 216 | 151 | 196 |
| Average Queue (ft) | 30 | 59 | 171 | 192 | 26 | 7 | 34 | 147 | 147 | 61 | 75 | 96 |
| 95th Queue (ft) | 69 | 132 | 315 | 345 | 137 | 37 | 87 | 265 | 264 | 149 | 140 | 166 |
| Link Distance (ft) |  |  | 813 | 813 |  |  |  | 1225 | 1225 |  |  |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  | 375 | 375 | 375 |  |  | 375 | 300 | 300 |
| Storage Bay Dist (ft) | 250 | 250 |  |  | 1 | 0 |  |  | 0 | 0 | 0 |  |
| Storage Blk Time (\%) |  |  | 3 | 0 | 0 |  |  | 0 | 0 | 0 | 0 |  |
| Queuing Penalty (veh) |  |  | 2 | 0 |  |  |  |  |  |  | 0 |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | TR | L | T | TR |
| Maximum Queue (ft) | 256 | 279 | 318 | 376 | 324 |
| Average Queue (ft) | 120 | 136 | 185 | 121 | 113 |
| 95th Queue (ft) | 208 | 237 | 320 | 273 | 238 |
| Link Distance (ft) | 903 | 903 |  | 1875 | 1875 |
| Upstream Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  | 300 |  |  |
| Storage Bay Dist (ft) | 0 |  | 5 | 0 |  |
| Storage Blk Time (\%) | 0 | 5 | 0 |  |  |

Intersection: 11: 124th Ave \& Blake Road

| Movement | EB | EB | WB | WB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | TR | L | TR |
| Maximum Queue (ft) | 54 | 81 | 24 | 59 | 8 | 53 | 3 |
| Average Queue (ft) | 16 | 26 | 2 | 8 | 0 | 7 | 0 |
| 95th Queue (ft) | 44 | 65 | 14 | 37 | 4 | 31 | 3 |
| Link Distance (ft) |  | 802 |  | 1330 | 1017 |  | 903 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |
| Queuing Penalty (veh) | 150 |  | 150 |  |  |  |  |
| Storage Bay Dist (ft) | 150 |  |  |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |

Intersection: 12: Blake Road \& Cipole Road

| Movement |
| :--- |
| Directions Served |
| Maximum Queue (ft) |
| Average Queue (ft) |
| 95th Queue (ft) |
| Link Distance (ft) |
| Upstream Blk Time (\%) |
| Queuing Penalty (veh) |
| Storage Bay Dist (ft) |
| Storage Blk Time (\%) |
| Queuing Penalty (veh) |
| Zone Summary |

Zone wide Queuing Penalty: 8

## Summary of All Intervals

| Run Number | 81 | 82 | 83 | 84 | 85 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Start Time | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ |
| End Time | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ |
| Total Time (min) | 70 | 70 | 70 | 70 | 70 | 70 |
| Time Recorded (min) | 60 | 60 | 60 | 60 | 60 | 60 |
| \# of Intervals | 5 | 5 | 5 | 5 | 5 | 5 |
| \# of Recorded Intervals | 4 | 4 | 4 | 4 | 4 | 4 |
| Vehs Entered | 6156 | 6200 | 6024 | 6274 | 6158 | 6159 |
| Vehs Exited | 6164 | 6142 | 6001 | 6239 | 6136 | 6138 |
| Starting Vehs | 341 | 302 | 335 | 347 | 343 | 332 |
| Ending Vehs | 333 | 360 | 358 | 382 | 365 | 354 |
| Travel Distance (mi) | 8765 | 8848 | 8516 | 8750 | 8718 | 8719 |
| Travel Time (hr) | 432.6 | 372.7 | 348.0 | 386.4 | 408.7 | 389.7 |
| Total Delay (hr) | 188.5 | 126.1 | 110.5 | 141.7 | 165.1 | 146.4 |
| Total Stops | 8905 | 8758 | 8745 | 8799 | 8336 | 8710 |
| Fuel Used (gal) | 329.7 | 317.6 | 304.6 | 318.1 | 322.1 | 318.4 |

## Interval \#0 Information Seeding

| Start Time | $4: 45$ |
| :--- | ---: |
| End Time | $4: 55$ |
| Total Time (min) | 10 |
| Volumes adjusted by Growth Factors. |  |
| No data recorded this interval. |  |

Interval \#1 Information Recording1

| Start Time | $4: 55$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $5: 10$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number | 81 | 82 | 83 | 84 | 85 | Avg |
| Vehs Entered | 1514 | 1562 | 1460 | 1513 | 1531 | 1511 |
| Vehs Exited | 1504 | 1483 | 1465 | 1496 | 1474 | 1485 |
| Starting Vehs | 341 | 302 | 335 | 347 | 343 | 332 |
| Ending Vehs | 351 | 381 | 330 | 364 | 400 | 364 |
| Travel Distance (mi) | 2140 | 2175 | 2043 | 2145 | 2143 | 2129 |
| Travel Time (hr) | 88.2 | 87.8 | 83.6 | 90.0 | 92.4 | 88.4 |
| Total Delay (hr) | 28.7 | 27.0 | 26.4 | 29.8 | 32.5 | 28.9 |
| Total Stops | 2161 | 2241 | 2184 | 2164 | 2042 | 2161 |
| Fuel Used (gal) | 76.7 | 77.9 | 73.6 | 77.3 | 77.1 | 76.5 |

SimTraffic Simulation Summary
Year 2025 Total Traffic Culdesac PM Peak Hour Conditions
Interval \#2 Information Recording2

| Start Time | $5: 10$ |
| :--- | ---: |
| End Time | $5: 25$ |
| Total Time $(\min )$ | 15 |
| Volumes adjusted by PHF, Growth Factors. |  |


| Run Number | 81 | 82 | 83 | 84 | 85 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1698 | 1612 | 1590 | 1689 | 1649 | 1645 |
| Vehs Exited | 1607 | 1624 | 1523 | 1657 | 1606 | 1606 |
| Starting Vehs | 351 | 381 | 330 | 364 | 400 | 364 |
| Ending Vehs | 442 | 369 | 397 | 396 | 443 | 405 |
| Travel Distance (mi) | 2327 | 2269 | 2187 | 2227 | 2253 | 2252 |
| Travel Time (hr) | 110.0 | 95.3 | 91.3 | 99.2 | 105.0 | 100.2 |
| Total Delay (hr) | 45.2 | 32.5 | 30.3 | 36.6 | 42.1 | 37.3 |
| Total Stops | 2452 | 2270 | 2301 | 2359 | 2175 | 2308 |
| Fuel Used (gal) | 86.2 | 81.7 | 78.6 | 80.8 | 83.4 | 82.1 |

Interval \#3 Information Recording3

| Start Time | $5: 25$ |
| :--- | ---: | :--- |
| End Time | $5: 40$ |
| Total Time (min) | 15 |
| Volumes adjusted by Growth Factors, Anti PHF. |  |


| Run Number | 81 | 82 | 83 | 84 | 85 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1473 | 1565 | 1432 | 1545 | 1493 | 1499 |
| Vehs Exited | 1537 | 1533 | 1507 | 1535 | 1543 | 1530 |
| Starting Vehs | 442 | 369 | 397 | 396 | 443 | 405 |
| Ending Vehs | 378 | 401 | 322 | 406 | 393 | 376 |
| Travel Distance (mi) | 2180 | 2220 | 2144 | 2179 | 2204 | 2186 |
| Travel Time (hr) | 115.8 | 95.1 | 87.9 | 100.4 | 109.5 | 101.7 |
| Total Delay (hr) | 54.9 | 33.3 | 28.0 | 39.6 | 48.0 | 40.8 |
| Total Stops | 2203 | 2182 | 2118 | 2059 | 2109 | 2131 |
| Fuel Used (gal) | 84.3 | 79.4 | 76.6 | 80.3 | 83.0 | 80.7 |

## Interval \#4 Information Recording4

| Start Time | $5: 40$ |
| :--- | ---: | :--- |
| End Time | $5: 55$ |
| Total Time (min) | 15 |
| Volumes adjusted by Growth Factors, Anti PHF. |  |


| Run Number | 81 | 82 | 83 | 84 | 85 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1471 | 1461 | 1542 | 1527 | 1485 | 1493 |
| Vehs Exited | 1516 | 1502 | 1506 | 1551 | 1513 | 1517 |
| Starting Vehs | 378 | 401 | 322 | 406 | 393 | 376 |
| Ending Vehs | 333 | 360 | 358 | 382 | 365 | 354 |
| Travel Distance (mi) | 2117 | 2185 | 2141 | 2199 | 2118 | 2152 |
| Travel Time (hr) | 118.6 | 94.4 | 85.2 | 96.9 | 101.8 | 99.4 |
| Total Delay (hr) | 59.6 | 33.4 | 25.8 | 35.7 | 42.5 | 39.4 |
| Total Stops | 2089 | 2065 | 2142 | 2217 | 2010 | 2108 |
| Fuel Used (gal) | 82.5 | 78.6 | 75.9 | 79.7 | 78.7 | 79.1 |

## Intersection: 4: Cipole Rd \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | T | TR | L | TR | L | TR |
| Maximum Queue (ft) | 86 | 211 | 208 | 63 | 262 | 282 | 146 | 89 | 112 | 98 |
| Average Queue (ft) | 22 | 68 | 91 | 14 | 97 | 101 | 56 | 39 | 43 | 47 |
| 95th Queue (ft) | 56 | 150 | 179 | 42 | 201 | 207 | 109 | 74 | 88 | 84 |
| Link Distance (ft) |  | 1100 | 1100 |  | 813 | 813 |  | 422 |  | 800 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  | 250 |  |  | 200 |  | 300 |  |
| Storage Bay Dist (ft) | 360 |  |  |  | 0 |  | 0 |  |  |  |
| Storage Blk Time (\%) |  |  |  |  | 0 |  | 0 |  |  |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | EB | EB | WB | WB | WB | WB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | L | T | T | R | L | L | T | T | R | L | L |
| Maximum Queue (ft) | 70 | 157 | 339 | 337 | 169 | 46 | 73 | 303 | 304 | 92 | 103 | 103 |
| Average Queue (ft) | 19 | 42 | 161 | 176 | 41 | 12 | 35 | 162 | 158 | 30 | 42 | 50 |
| 95th Queue (ft) | 50 | 99 | 292 | 297 | 112 | 39 | 66 | 263 | 256 | 75 | 80 | 89 |
| Link Distance (ft) |  |  | 813 | 813 |  |  |  | 1225 | 1225 |  |  |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  | 375 | 375 | 375 |  |  | 375 | 300 | 300 |
| Storage Bay Dist (ft) | 250 | 250 |  |  | 0 |  |  |  | 0 |  |  |  |
| Storage Blk Time (\%) |  |  | 2 | 0 |  |  |  | 0 |  |  |  |  |
| Queuing Penalty (veh) |  |  | 1 | 0 |  |  |  |  |  |  |  |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | TR | L | T | TR |
| Maximum Queue (ft) | 99 | 95 | 247 | 182 | 264 |
| Average Queue (ft) | 51 | 45 | 116 | 96 | 134 |
| 95th Queue (ft) | 91 | 90 | 211 | 163 | 233 |
| Link Distance (ft) | 873 | 873 |  | 1875 | 1875 |
| Upstream Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |
| Storage Bay Dist (ft) |  |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |

Intersection: 11: 124th Ave \& Blake Road

| Movement | EB | EB | WB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | TR | TR |
| Maximum Queue (ft) | 38 | 52 | 38 | 55 | 4 | 10 |
| Average Queue (ft) | 8 | 13 | 13 | 19 | 0 | 0 |
| 95th Queue (ft) | 30 | 43 | 34 | 47 | 3 | 5 |
| Link Distance (ft) |  | 781 |  | 930 | 1716 | 873 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |
| Queuing Penalty (veh) | 150 |  | 150 |  |  |  |
| Storage Bay Dist (ft) | 150 |  |  |  |  |  |

Intersection: 12: Blake Road \& Cipole Road

| Movement |
| :--- |
| Directions Served |
| Maximum Queue (ft) |
| Average Queue (ft) |
| 95th Queue (ft) |
| Link Distance (ft) |
| Upstream Blk Time (\%) |
| Queuing Penalty (veh) |
| Storage Bay Dist (ft) |
| Storage Blk Time (\%) |
| Queuing Penalty (veh) |
| Zone Summary |

Zone wide Queuing Penalty: 2

## Summary of All Intervals

| Run Number | 51 | 52 | 53 | 54 | 55 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Start Time | $7: 10$ | $7: 10$ | $7: 10$ | $7: 10$ | $7: 10$ | $7: 10$ |
| End Time | $8: 20$ | $8: 20$ | $8: 20$ | $8: 20$ | $8: 20$ | $8: 20$ |
| Total Time (min) | 70 | 70 | 70 | 70 | 70 | 70 |
| Time Recorded (min) | 60 | 60 | 60 | 60 | 60 | 60 |
| \# of Intervals | 5 | 5 | 5 | 5 | 5 | 5 |
| \# of Recorded Intervals | 4 | 4 | 4 | 4 | 4 | 4 |
| Vehs Entered | 5615 | 5645 | 5678 | 5604 | 5548 | 5615 |
| Vehs Exited | 5622 | 5679 | 5673 | 5606 | 5570 | 5631 |
| Starting Vehs | 307 | 302 | 282 | 280 | 312 | 290 |
| Ending Vehs | 300 | 268 | 287 | 278 | 290 | 282 |
| Travel Distance (mi) | 7631 | 7662 | 7719 | 7588 | 7572 | 7634 |
| Travel Time (hr) | 306.3 | 308.3 | 307.9 | 310.7 | 299.3 | 306.5 |
| Total Delay (hr) | 95.1 | 95.6 | 94.4 | 101.0 | 89.7 | 95.2 |
| Total Stops | 6829 | 7200 | 7043 | 7332 | 6776 | 7036 |
| Fuel Used (gal) | 271.8 | 274.4 | 274.7 | 272.3 | 267.9 | 272.2 |

Interval \#0 Information Seeding

| Start Time | $7: 10$ |
| :--- | ---: |
| End Time | $7: 20$ |
| Total Time (min) | 10 |
| Volumes adjusted by Growth Factors. |  |
| No data recorded this interval. |  |

Interval \#1 Information Recording1

| Start Time | $7: 20$ |
| :--- | :---: |
| End Time | $7: 35$ |
| Total Time $(\min )$ | 15 |
| Volumes adjusted by Growth Factors, Anti PHF. |  |


| Run Number | 51 | 52 | 53 | 54 | 55 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1297 | 1350 | 1330 | 1377 | 1281 | 1329 |
| Vehs Exited | 1337 | 1348 | 1331 | 1376 | 1343 | 1348 |
| Starting Vehs | 307 | 302 | 282 | 280 | 312 | 290 |
| Ending Vehs | 267 | 304 | 281 | 281 | 250 | 272 |
| Travel Distance (mi) | 1844 | 1843 | 1819 | 1858 | 1841 | 1841 |
| Travel Time (hr) | 73.4 | 72.1 | 70.5 | 74.8 | 72.0 | 72.6 |
| Total Delay (hr) | 22.6 | 20.8 | 20.3 | 23.4 | 21.0 | 21.6 |
| Total Stops | 1641 | 1660 | 1625 | 1778 | 1582 | 1657 |
| Fuel Used (gal) | 65.6 | 65.1 | 64.3 | 66.5 | 64.5 | 65.2 |

SimTraffic Simulation Summary
Year 2025 Total Traffic Cipole Extension AM Peak Hour Conditions
Interval \#2 Information Recording2

| Start Time | $7: 35$ |
| :--- | ---: |
| End Time | $7: 50$ |
| Total Time (min) | 15 |
| Volumes adjusted by PHF, Growth Factors. |  |


| Run Number | 51 | 52 | 53 | 54 | 55 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1581 | 1634 | 1631 | 1641 | 1580 | 1612 |
| Vehs Exited | 1491 | 1604 | 1567 | 1579 | 1484 | 1548 |
| Starting Vehs | 267 | 304 | 281 | 281 | 250 | 272 |
| Ending Vehs | 357 | 334 | 345 | 343 | 346 | 344 |
| Travel Distance (mi) | 1954 | 2150 | 2124 | 2057 | 1992 | 2055 |
| Travel Time (hr) | 80.4 | 89.7 | 87.8 | 89.4 | 80.7 | 85.6 |
| Total Delay (hr) | 26.2 | 29.7 | 28.7 | 32.0 | 25.5 | 28.4 |
| Total Stops | 1883 | 2116 | 2032 | 2206 | 1916 | 2028 |
| Fuel Used (gal) | 70.0 | 77.3 | 76.2 | 75.2 | 71.3 | 74.0 |

Interval \#3 Information Recording3

| Start Time | $7: 50$ |
| :--- | ---: | :--- |
| End Time | $8: 05$ |
| Total Time (min) | 15 |
| Volumes adjusted by Growth Factors, Anti PHF. |  |


| Run Number | 51 | 52 | 53 | 54 | 55 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1407 | 1337 | 1361 | 1295 | 1325 | 1341 |
| Vehs Exited | 1472 | 1360 | 1419 | 1323 | 1384 | 1393 |
| Starting Vehs | 357 | 334 | 345 | 343 | 346 | 344 |
| Ending Vehs | 292 | 311 | 287 | 315 | 287 | 295 |
| Travel Distance (mi) | 2002 | 1834 | 1881 | 1852 | 1879 | 1890 |
| Travel Time (hr) | 79.9 | 74.4 | 75.1 | 74.0 | 74.7 | 75.6 |
| Total Delay (hr) | 24.6 | 23.6 | 23.0 | 23.2 | 22.9 | 23.5 |
| Total Stops | 1704 | 1790 | 1687 | 1678 | 1646 | 1698 |
| Fuel Used (gal) | 71.1 | 66.3 | 67.2 | 66.0 | 66.7 | 67.5 |

Interval \#4 Information Recording4

| Start Time | $8: 05$ |
| :--- | ---: | :--- |
| End Time | $8: 20$ |
| Total Time (min) | 15 |
| Volumes adjusted by Growth Factors, Anti PHF. |  |


| Run Number | 51 | 52 | 53 | 54 | 55 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1330 | 1324 | 1356 | 1291 | 1362 | 1332 |
| Vehs Exited | 1322 | 1367 | 1356 | 1328 | 1359 | 1343 |
| Starting Vehs | 292 | 311 | 287 | 315 | 287 | 295 |
| Ending Vehs | 300 | 268 | 287 | 278 | 290 | 282 |
| Travel Distance (mi) | 1831 | 1836 | 1895 | 1821 | 1860 | 1848 |
| Travel Time (hr) | 72.7 | 72.2 | 74.5 | 72.5 | 71.9 | 72.7 |
| Total Delay (hr) | 21.8 | 21.5 | 22.4 | 22.4 | 20.2 | 21.6 |
| Total Stops | 1601 | 1634 | 1699 | 1670 | 1632 | 1648 |
| Fuel Used (gal) | 65.1 | 65.7 | 67.1 | 64.6 | 65.3 | 65.6 |

Intersection: 4: Cipole Rd \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | T | TR | L | TR | L | TR |
| Maximum Queue (ft) | 116 | 212 | 299 | 112 | 182 | 213 | 68 | 46 | 118 | 118 |
| Average Queue (ft) | 46 | 64 | 92 | 40 | 55 | 70 | 17 | 12 | 50 | 33 |
| 95th Queue (ft) | 86 | 152 | 198 | 85 | 142 | 159 | 49 | 38 | 107 | 82 |
| Link Distance (ft) |  | 1102 | 1102 |  | 813 | 813 |  | 401 |  | 800 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  | 250 |  |  | 200 |  | 300 |  |
| Storage Bay Dist (ft) | 360 |  |  |  | 0 |  |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  | 0 |  |  |  |  |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | EB | EB | WB | WB | WB | WB | WB | NB | NB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | L | T | T | R | L | L | T | T | R | L | L |
| Maximum Queue (ft) | 90 | 242 | 413 | 492 | 154 | 80 | 115 | 304 | 326 | 219 | 140 | 183 |
| Average Queue (ft) | 31 | 64 | 186 | 216 | 21 | 9 | 31 | 138 | 139 | 75 | 64 | 80 |
| 95th Queue (ft) | 72 | 154 | 334 | 378 | 90 | 45 | 80 | 245 | 251 | 169 | 130 | 147 |
| Link Distance (ft) |  |  | 813 | 813 |  |  |  | 1225 | 1225 |  |  |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 250 | 250 |  |  | 375 | 375 | 375 |  |  | 375 | 300 | 300 |
| Storage Blk Time (\%) |  | 0 | 3 | 1 |  |  |  | 0 | 0 |  |  |  |
| Queuing Penalty (veh) |  | 0 | 2 | 0 |  |  |  | 0 | 0 |  |  |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | TR | L | T | TR |
| Maximum Queue (ft) | 237 | 279 | 269 | 234 | 246 |
| Average Queue (ft) | 126 | 142 | 151 | 100 | 109 |
| 95th Queue (ft) | 210 | 240 | 260 | 184 | 209 |
| Link Distance (ft) | 903 | 903 |  | 1875 | 1875 |
| Upstream Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  | 300 |  |  |
| Storage Bay Dist (ft) |  |  | 0 |  |  |
| Storage Blk Time (\%) |  |  | 0 |  |  |

Intersection: 11: 124th Ave \& Blake Road

| Movement | EB | EB | WB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | L |
| Maximum Queue (ft) | 61 | 70 | 30 | 56 | 33 | 38 |
| Average Queue (ft) | 17 | 22 | 3 | 8 | 4 | 5 |
| 95th Queue (ft) | 44 | 56 | 17 | 34 | 19 | 24 |
| Link Distance (ft) |  | 802 |  | 1330 |  |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  | 150 |  | 150 | 150 |
| Storage Bay Dist (ft) | 150 |  | 150 |  |  |  |

Intersection: 12: Blake Road \& Cipole Road

| Movement | EB | SB |
| :--- | ---: | ---: |
| Directions Served | L | LR |
| Maximum Queue (ft) | 14 | 46 |
| Average Queue (ft) | 1 | 11 |
| 95th Queue (ft) | 12 | 36 |
| Link Distance (ft) |  | 546 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) | 150 |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Zone Summary |  |  |

Zone wide Queuing Penalty: 4

SimTraffic Simulation Summary
Year 2025 Total Traffic Cipole Extension PM Peak Hour Conditions
Summary of All Intervals

| Run Number | 61 | 62 | 63 | 64 | 65 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Start Time | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ | $4: 45$ |
| End Time | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ | $5: 55$ |
| Total Time (min) | 70 | 70 | 70 | 70 | 70 | 70 |
| Time Recorded (min) | 60 | 60 | 60 | 60 | 60 | 60 |
| \# of Intervals | 5 | 5 | 5 | 5 | 5 | 5 |
| \# of Recorded Intervals | 4 | 4 | 4 | 4 | 4 | 4 |
| Vehs Entered | 6410 | 6121 | 6307 | 6203 | 6166 | 6240 |
| Vehs Exited | 6335 | 6114 | 6241 | 6217 | 6189 | 6218 |
| Starting Vehs | 341 | 337 | 321 | 340 | 339 | 332 |
| Ending Vehs | 416 | 344 | 387 | 326 | 316 | 353 |
| Travel Distance (mi) | 8863 | 8647 | 8988 | 8831 | 8684 | 8803 |
| Travel Time (hr) | 399.3 | 366.6 | 393.1 | 387.1 | 372.1 | 383.6 |
| Total Delay (hr) | 151.1 | 125.8 | 143.9 | 141.2 | 129.4 | 138.3 |
| Total Stops | 8865 | 8662 | 9094 | 9088 | 8964 | 8938 |
| Fuel Used (gal) | 324.1 | 311.7 | 329.1 | 322.6 | 315.0 | 320.5 |

Interval \#0 Information Seeding

| Start Time | $4: 45$ |
| :--- | ---: |
| End Time | $4: 55$ |
| Total Time (min) | 10 |
| Volumes adjusted by Growth Factors. |  |
| No data recorded this interval. |  |

Interval \#1 Information Recording1

| Start Time | $4: 55$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $5: 10$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number |  |  |  |  |  |  |
| Vehs Entered | 1605 | 1470 | 1530 | 1560 | 1498 | 1523 |
| Vehs Exited | 1586 | 1516 | 1480 | 1526 | 1482 | 1516 |
| Starting Vehs | 341 | 337 | 321 | 340 | 339 | 332 |
| Ending Vehs | 360 | 291 | 371 | 374 | 355 | 347 |
| Travel Distance (mi) | 2209 | 2125 | 2195 | 2188 | 2140 | 2171 |
| Travel Time (hr) | 89.2 | 85.4 | 88.3 | 90.2 | 87.1 | 88.0 |
| Total Delay (hr) | 27.5 | 26.4 | 27.5 | 29.0 | 27.5 | 27.6 |
| Total Stops | 2153 | 2064 | 2136 | 2225 | 2109 | 2137 |
| Fuel Used (gal) | 78.7 | 75.7 | 78.2 | 78.5 | 75.9 | 77.4 |

SimTraffic Simulation Summary
Year 2025 Total Traffic Cipole Extension PM Peak Hour Conditions
Interval \#2 Information Recording2

| Start Time | $5: 10$ |
| :--- | ---: |
| End Time | $5: 25$ |
| Total Time $(\min )$ | 15 |
| Volumes adjusted by PHF, Growth Factors. |  |


| Run Number | 61 | 62 | 63 | 64 | 65 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1723 | 1670 | 1624 | 1658 | 1658 | 1667 |
| Vehs Exited | 1626 | 1571 | 1569 | 1610 | 1571 | 1591 |
| Starting Vehs | 360 | 291 | 371 | 374 | 355 | 347 |
| Ending Vehs | 457 | 390 | 426 | 422 | 442 | 425 |
| Travel Distance (mi) | 2334 | 2231 | 2301 | 2341 | 2190 | 2280 |
| Travel Time (hr) | 106.9 | 95.7 | 103.2 | 102.2 | 94.2 | 100.4 |
| Total Delay (hr) | 41.9 | 33.3 | 39.6 | 37.4 | 33.0 | 37.1 |
| Total Stops | 2429 | 2326 | 2525 | 2406 | 2443 | 2426 |
| Fuel Used (gal) | 85.9 | 80.2 | 85.0 | 85.4 | 80.0 | 83.3 |

## Interval \#3 Information Recording3

| Start Time | $5: 25$ |
| :--- | ---: |
| End Time | $5: 40$ |
| Total Time (min) | 15 |
| Volumes adjusted by Growth Factors, Anti PHF. |  |


| Run Number | 61 | 62 | 63 | 64 | 65 | Avg |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehs Entered | 1538 | 1513 | 1558 | 1549 | 1547 | 1538 |
| Vehs Exited | 1619 | 1542 | 1603 | 1594 | 1620 | 1595 |
| Starting Vehs | 457 | 390 | 426 | 422 | 442 | 425 |
| Ending Vehs | 376 | 361 | 381 | 377 | 369 | 366 |
| Travel Distance (mi) | 2224 | 2181 | 2257 | 2224 | 2228 | 2223 |
| Travel Time (hr) | 105.0 | 95.5 | 100.4 | 100.8 | 101.0 | 100.5 |
| Total Delay (hr) | 42.3 | 34.8 | 37.6 | 38.8 | 38.5 | 38.4 |
| Total Stops | 2178 | 2262 | 2123 | 2398 | 2253 | 2241 |
| Fuel Used (gal) | 81.9 | 80.1 | 82.6 | 82.6 | 82.4 | 81.9 |

## Interval \#4 Information Recording4

| Start Time | $5: 40$ |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| End Time | $5: 55$ |  |  |  |  |  |
| Total Time (min) | 15 |  |  |  |  |  |
| Volumes adjusted by Growth Factors, Anti PHF. |  |  |  |  |  |  |
| Run Number |  |  |  |  |  |  |
| Vehs Entered | 1544 | 1468 | 1595 | 1436 | 1463 | 1500 |
| Vehs Exited | 1504 | 1485 | 1589 | 1487 | 1516 | 1517 |
| Starting Vehs | 376 | 361 | 381 | 377 | 369 | 366 |
| Ending Vehs | 416 | 344 | 387 | 326 | 316 | 353 |
| Travel Distance (mi) | 2095 | 2110 | 2235 | 2078 | 2125 | 2129 |
| Travel Time (hr) | 98.2 | 90.0 | 101.2 | 93.8 | 89.8 | 94.6 |
| Total Delay (hr) | 39.4 | 31.3 | 39.1 | 35.9 | 30.4 | 35.2 |
| Total Stops | 2105 | 2010 | 2310 | 2059 | 2159 | 2133 |
| Fuel Used (gal) | 77.7 | 75.7 | 83.3 | 76.0 | 76.7 | 77.9 |

Intersection: 4: Cipole Rd \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | WB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | T | TR | L | TR | L | TR |
| Maximum Queue (ft) | 59 | 162 | 206 | 96 | 285 | 28 | 100 | 83 | 98 | 98 |
| Average Queue (ft) | 20 | 60 | 81 | 13 | 101 | 101 | 39 | 30 | 42 | 48 |
| 95th Queue (ft) | 48 | 137 | 173 | 57 | 219 | 215 | 83 | 64 | 84 | 85 |
| Link Distance (ft) |  | 1100 | 1100 |  | 813 | 813 |  | 422 |  | 800 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  | 250 |  |  | 200 |  | 300 |  |
| Storage Bay Dist (ft) | 360 |  |  |  | 1 |  |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  | 0 |  |  |  |  |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | EB | EB | EB | EB | EB | WB | WB | WB | WB | WB | NB | NB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | L | T | T | R | L | L | T | T | R | L | L |
| Maximum Queue ( t ) | 66 | 169 | 315 | 388 | 243 | 46 | 72 | 318 | 335 | 124 | 107 | 102 |
| Average Queue (ft) | 18 | 47 | 167 | 195 | 47 | 15 | 38 | 174 | 172 | 30 | 45 | 52 |
| 95th Queue (ft) | 50 | 112 | 284 | 312 | 138 | 40 | 65 | 283 | 281 | 83 | 89 | 92 |
| Link Distance (ft) |  |  | 813 | 813 |  |  |  | 1225 | 1225 |  |  |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (tt) | 250 | 250 |  |  | 375 | 375 | 375 |  |  | 375 | 300 | 300 |
| Storage BIk Time (\%) |  |  | 1 | 0 | 0 |  |  |  | 0 |  |  |  |
| Queuing Penalty (veh) |  |  | 1 | 0 | 0 |  |  |  | 0 |  |  |  |

Intersection: 5: 124th Ave \& Tualatin-Sherwood Rd

| Movement | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | TR | L | T | TR |
| Maximum Queue (ft) | 97 | 115 | 247 | 260 | 286 |
| Average Queue (ft) | 46 | 44 | 121 | 95 | 136 |
| 95th Queue (ft) | 87 | 90 | 205 | 183 | 235 |
| Link Distance (ft) | 873 | 873 |  | 1875 | 1875 |
| Upstream Blk Time (\%) |  |  |  |  |  |
| Queuing Penalty (veh) |  |  | 300 |  |  |
| Storage Bay Dist (ft) |  |  | 0 |  |  |
| Storage Blk Time (\%) |  | 0 |  |  |  |

Intersection: 11: 124th Ave \& Blake Road

| Movement | EB | EB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | TR | L | TR |
| Maximum Queue (ft) | 43 | 50 | 52 | 53 | 26 | 10 | 5 | 15 |
| Average Queue (ft) | 15 | 20 | 14 | 20 | 1 | 0 | 0 | 1 |
| 95th Queue (ft) | 40 | 45 | 40 | 47 | 10 | 7 | 4 | 11 |
| Link Distance (ft) |  | 781 |  | 930 |  | 1716 |  | 873 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  | 150 |  | 150 |  | 150 |  |
| Storage Bay Dist (ft) | 150 |  | 150 |  |  |  |  |  |

Intersection: 12: Blake Road \& Cipole Road

| Movement | EB | SB |
| :--- | ---: | ---: |
| Directions Served | L | LR |
| Maximum Queue (ft) | 12 | 63 |
| Average Queue (ft) | 0 | 31 |
| 95th Queue (ft) | 6 | 54 |
| Link Distance (ft) |  | 300 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) | 150 |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Zone Summary |  |  |

Zone wide Queuing Penalty: 1




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# Wetland Delineation T-S Corporate Park in Sherwood, Oregon 

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## I. INTRODUCTION

Pacific Habitat Services, Inc. (PHS) conducted a wetland delineation for a potential development parcel on SW Tualatin-Sherwood Road in Sherwood, Washington County, Oregon (Township 2 South, Range 1 West, Section 28D, portion of tax lot 1100). Delineation fieldwork began in 2017 when lot 1100 was the northern portion of a larger lot (lot 100). Lot 100 has since been partitioned into lots 1100, the northern extent, and 1200, the southern extent.

David Evans and Associates (DEA) delineated the southern portion of tax lot 1100 in 2017 in conjunction with the Willamette Water Supply Program pipeline and water treatment facility. The Department of State Lands (DSL) approved the DEA delineation in 2017 (DSL WD\# 2017-0008). That delineation encompassed the current boundary of tax lot 1200 to the south as well as the southern portion of tax lot 1100 .

This report presents the results of PHS' wetland delineation of the study area. Figures, including a map depicting the location of wetlands within the study area, are located in Appendix A. Data sheets in Appendix B document onsite conditions. Ground-level photos of the site are located in Appendix C. A discussion of the wetland delineation methodology is provided for the client in Appendix D.

## II. RESULTS AND DISCUSSION

## A. Landscape Setting and Land Use

The study area borders SW Tualatin-Sherwood Road to the north; on the west by a partially developed site including a municipal water storage facility; on the east by SW $124^{\text {th }}$ Avenue; and on the south by a partially developed industrial storage site. Nearby land uses include partially landscaped rural residential lots, aggregate mining operations, and small-scale commercial/ industrial activities. Until the summer of 2019, the site included a former farmhouse and other structures in the west-central portion of the site.

Vegetation communities upslope of the formerly farmed area near Tualatin-Sherwood Road have formed in the relatively rocky, hilly terrain of the 'Tonquin Scablands, an area between Sherwood and Tualatin that was scoured by the enormous Bretz flood events during the Pleistocene era. More recent human disturbance (both logging and farming) has also helped shape the current vegetation cover.

The mostly forested to shrubby upland areas upslope of the agricultural fields are comprised of a relatively young to mature overstory of Douglas fir (Pseudotsuga menziesii, FACU), with bigleaf maple (Acer macrophyllum, FACU), Oregon white oak (Quercus garryana, FACU), and madrone (Arbutus menziesii, UPL) also present. The shrub understory is dense and commonly dominated by poison oak (Toxicodendron diversilobum, FAC), tall Oregon grape (Mahonia aquifolium, FACU), oceanspray (Holodiscus discolor, FACU), snowberry (Symphoricarpos albus, FACU), and Saskatoon serviceberry (Amelanchier alnifolia, FACU). Sword fern (Polystichum munitum, FACU) is a common groundcover species. Invasive shrubs such as Himalayan blackberry (Rubus armeniacus, FAC) and Scotch broom (Cytisus scoparius, UPL) are common in more recently disturbed edge habitats.

Scoured depressions within the more hilly terrain above the open fields can be poorly drained and seasonally ponded, often supporting wetland plant assemblages. The overstory consists of Oregon ash (Fraxinus latifolia, FACW), with shrubs such as hardhack spirea (Spiraea douglasii, FACW), rose (Rosa spp., FAC), willows (Salix spp., FAC to FACW) and snowberry often present. Common emergent species include slough sedge (Carex obnupta, OBL), soft rush (Juncus effusus, FACW), spreading rush (Juncus patens, FACW), reed canarygrass (Phalaris arundinacea, FACW), and fringed willow-herb (Epilobium ciliatum, FACW).

A broad seasonal swale extends northward from a hillside seep zone dividing the formerly farmed area into two large fields. The hillside seep zone was only accessible after an excavator cleared trails through dense Himalayan blackberry and poison oak thickets. The seepage may result from slow subsurface drainage of a seasonally ponded depression (Wetland C) further upslope near the southern edge of the study area.

The seasonal swale supports a stand of mature and sapling Oregon ash, willows, hardhack spirea, ninebark (Physocarpus capitatus, FAC), blackberries, reed canarygrass, soft rush, tall fescue (Schedonorus arundinaceus, FAC), and creeping buttercup (Ranunculus repens, FACW). The seasonally charged surface flows are culverted beneath Tualatin-Sherwood Road, ultimately feeding to Hedges Creek.

## B. Site Alterations

Despite a long history of agricultural activities within most of the northern portion of the lot, wetland features within the parcel have been mostly undisturbed in recent years. This is likely due to the excessive seasonal wetness along the seasonal swale in particular, with clearing and cultivation activities mostly avoiding the lower wet areas. At the same time, the relatively steep, rocky slopes in the southwest portion of the lot provide poor soils and appear to have been left alone, except as a timber source. A former farmhouse and other agricultural structures were sited along the transition from tillable soil to rocky conditions. These structures are still visible on maps and recent air photos, though all were removed in the summer of 2019. All that remains of prior development is the gravel driveway from Tualatin-Sherwood Road and the various remains of building foundations.

The areas adjoining a seasonally ponded depression (Wetland C) have been forested since at least the early 1950s; however, much of that same area was logged in the late 1980s or early 1990s. The logging did not appear to change land use patterns as the boundary between farmed land and the forested area seems little changed since 1952. Ground disturbance north-northeast of Wetland C is evident from aerial photographs dating to the early 2000s (available on GoogleEarth). The movement of soil and/or aggregate appears to have occurred only between 2002 and 2005, but there is no evidence that any wetlands were filled during that period. The slopes east and west of Wetland C are naturally steep (based upon the extent of naturally occurring rock at and near the surface). At Wetland C's northern extent, the slopes are less steep and there was no evidence of fill at or near the wetland boundary.

## C. Precipitation Data and Analysis

Precipitation histories reviewed for both the original delineation and subsequent field work. Tables 1 and 2 compare the monthly precipitation amounts recorded at the Portland KGW TV station to the average monthly precipitation records, as well as to the normal precipitation range as identified in the Natural Resource Conservation Service's (NRCS) WETS climate table. This data reveals that conditions were observed on this site during periods of excessive rainfall (in the spring of 2017) and during an unusually dry fall (in 2019).

For the period tabulated in Table 1, observed precipitation in February, March and April were not within the normal range of variability; instead, each month was much higher than the average.

Table 1: Comparison of average and observed monthly precipitation at Portland's KGW TV, prior to the March through April 2017 delineation fieldwork.

| Month | Average <br> Precipitation | 30\% Chance Will Have |  | Observed <br> Average Than $^{\mathbf{1}}$ | More Than $^{\text {Percent of }}$ <br> Average $^{\mathbf{1}}$ |
| :--- | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |
| February | 4.93 | 3.03 | 5.97 | 12.18 | $247 \%$ |
| March | 5.30 | 4.08 | 6.15 | 8.40 | $158 \%$ |
| April | 3.61 | 2.74 | 4.20 | 4.63 | $128 \%$ |
| May | 2.51 | 1.46 | 3.05 | 2.25 | $90 \%$ |
| June | 1.51 | 0.80 | 1.84 | 1.12 | $74 \%$ |

1. Source: NRCS WETS Table (period from 1995 to 2018) and Climatological Data for KGW-TV in Portland, OR (http://
http://agacis.rcc-acis.org/?fips=41051)
The 2.6 inches of precipitation observed over the two weeks prior to the late April fieldwork was also higher than normal for the time of year, however, the weeks preceding a final visit to Wetland C in late June 2017 were lower than normal yet still within the normal range of variability. As such, PHS personnel believe that relatively "normal circumstances" in terms of site hydrology have prevailed for PHS's delineation fieldwork, despite the wide variation from normal rainfall amounts during previous months. Site gradients provided reasonable drainage through the northern reaches of the site. Wetland C, however, likely ponded to a greater depth and for a longer period than normal, delaying effective data collection until early summer.

Table 2: Comparison of average and observed monthly precipitation at Portland's KGW TV, prior to the October through December 2019 delineation fieldwork.

| Month | Average <br> Precipitation $^{1}$ | 30\% Chance Will Have <br> Less Than <br> Average $^{\mathbf{1}}$ |  | More Than <br> Average $^{\mathbf{1}}$ | Observed <br> Precipitation |
| :--- | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |
| July | 0.42 | 0.32 | 0.53 | 0.26 | $62 \%$ |
| August | 0.57 | 0.13 | 0.56 | 0.59 | $104 \%$ |
| September | 1.71 | 0.76 | 2.09 | 4.40 | $257 \%$ |
| October | 4.12 | 2.49 | 5.00 | 1.81 | $44 \%$ |
| November | 6.78 | 4.74 | 8.06 | 1.58 | $23 \%$ |

[^14]Rainfall totals for the two-week period prior to each of the 2019 field dates where data was collected were 0.19 inches for October 15, and 1.96 inches for December 13. These totals are both lower than normal for any two week period at that time of year; however, as 2019 field work occurred in the fall of the year, significantly drier conditions were anticipated as a result of seasonal variability. As the 2019 field work was focused on confirming the prior delineation and collecting supporting, updated sample points, the drier than normal conditions in October through early December were of less importance as these wetlands would not normally have been recharged hydrologically by the late fall, even under normal precipitation patterns.

## D. Methods

PHS determined the location of wetlands within the study area based on the presence of wetland hydrology, hydric soils, and hydrophytic vegetation. This approach is in accordance with the Routine On-site Determination, as described in the Corps of Engineers Wetland Delineation Manual, Wetlands Research Program Technical Report Y-87-1 ("The 1987 Manual") and the Regional Supplement to the Corps of Engineers Wetland Delineation Manual: Western Mountains, Valleys, and Coast Region, May 2010). The study area was originally delineated on March 2 and March 26, 2017, with additional site data collected on April 28 and June 28, 2017. In order to submit the results of the delineation to the agencies for concurrence, the wetland boundaries were confirmed and/or additional data collected on October 15, November 8 and December 13, 2019.

The entire study area was investigated for the presence of wetlands or other waters. The northern portion of the seasonal swale (Wetland A) was flagged on March 2, 2017; its upper wetland edge was determined by the relatively sharp topographic break at the spring/seep edge, vegetation changes, and the presence of flowing surface water, near surface free water, and saturation. Hydric soil indicators typically included redoximorphic features in this area.

Data for both Wetland A and Wetland B was collected during early April 2017, both to allow water levels to recede and to again see each site under more typical weather conditions (since both February and March 2017 were exceedingly wet months).

The southern ponded depression (Wetland C) was flagged on March 26, 2017, in an attempt to allow water levels to recede somewhat from the first visit. At that time, much of the boundary (based on vegetation transitions) was obscured by high water, which was inundating areas dominated by mostly upland plants and could not be confirmed for hydric soils. As such, a rough boundary slightly within the inundated area was flagged to include some FACU vegetation.

Data for Wetland C was collected at a later date in April 2017 to allow full drawdown of this seasonally ponded area.

The 2019 field visits largely confirmed the prior delineation, with a portion of the southern boundary of Wetland A moved about 25 feet to the north where the area lacked indicators for both hydric soils and hydrology.

## E. Description of all Wetlands and Other Non-Wetland Waters

PHS delineated three wetlands in the study area.

## Wetland A

Wetland A is a broad seasonal swale (2.34 acres) that extends northward to Tualatin-Sherwood Road. The swale originates from a band of hillside seeps or springs. The seepage band was not initially accessible due to dense poison oak and blackberry cover; however, access trails were cut using a tracked excavator. LIDAR imagery also indicated a short, confined area where the surface water originates; this feature was verified once the site could be accessed. Its origin is likely percolation through porous soils from the large depression further upslope to the south (Wetland C), which is discussed below.

Several shallow incised channels extend northward from the seeps through the broad swale, ultimately to form a single larger channel near Tualatin Sherwood Road. Wetland conditions extend for some distance to either side of the channels, supporting a mature stand of Oregon ash. The seasonally charged surface flows are culverted beneath Tualatin-Sherwood Road, ultimately flowing to Hedges Creek (a Tualatin River tributary).

The swale's Cowardin class ranges from palustrine emergent through scrub-shrub and forested, saturated/semipermanent/seasonal (PEMY/PSSY/PFOY) wetland, while the Hydrogeomorphic (HGM) class is Slope, largely due to its moderate to shallow gradient and upslope seepage/groundwater spring origins.

Vegetation within Wetland A is dominated by a mature Oregon ash stand, as mentioned above. Also present are a variety of shrubs and herbaceous species that include willows, hardhack spirea, Pacific ninebark, Himalayan blackberry, reed canarygrass, soft rush, Cooley's hedgenettle (Stachys chamissonis, FACW), tall fescue and creeping velvetgrass (Holcus mollis, FACW).

Species encountered in uplands adjacent to Wetland A include Himalayan blackberry, St. John's wort (Hypericum perforatum, FACU), bedstraw (Galium aparine, FACU), shiny geranium (Geranium lucidum, UPL), clovers (Trifolium spp., FAC-FACU), perennial ryegrass (Lolium perenne, FAC), and Queen Anne's lace (Daucus carota, FACU).

Soils within Wetland A were typically silt loams and silty clay loams, and generally met the redox dark surface hydric soil indicator. The swale's seasonally charged hydrology is largely driven by upslope groundwater seepage and to some extent direct rainfall onto the site. Scattered shallow inundation, flowing surface water (in small channels), near surface water tables and saturation, and oxidized rhizospheres were all in evidence at the time of sampling.

## Wetland B

Wetland $B$ is a small, arguably isolated concave wetland ( 0.03 acre ) on a gentle slope in the now fallow field west of Wetland A. This location was actively farmed prior to 2017, but appears to have been left fallow since at least Fall 2017. The wetland appears to be fed by seasonally charged upslope groundwater seepage and overland sheet flow. Its Cowardin class is palustrine emergent, saturated/ semipermanent/ seasonal (PEMY) wetland, while the HGM class is Slope-Flats.

Vegetation within Wetland B is dominated by weakly emergent, mostly non-native species that includes a hybrid clover (Trifolium sp., FAC), perennial ryegrass (Lolium perenne, FAC), and lesser hawkbit (Leontodon saxatilis, FACU).

Less common species in the wetland, as well as within the adjacent uplands included the hybrid clover, red clover (Trifolium pratense, FACU), sheep sorrel (Rumex acetosella, FACU), common velvetgrass (Holcus lanatus, FAC) and hairy cats-ear (Hypochaeris radicata, FACU).

Soils within Wetland B were silt to silt loam that met the redox dark surface hydric soil indicator. This subtly shallow, depressional area exhibited seasonally charged hydrology driven by upslope groundwater seepage and overland sheetflow. Shallow inundation, near surface water tables and saturation, and oxidized rhizospheres were in evidence at the time of sampling.

## Wetland C

Wetland C is a 0.54 acre depressional feature at the south end of the site that extends outside property boundaries. The depression is likely an old scour feature from the Bretz flood events, with relatively steep sideslopes on the west and east sides. The north edge is comparatively low in elevation, but topography rises several feet just to the north, sufficient to contain seasonal rainfall accumulations and act as an impoundment. Its Cowardin class is primarily forested, seasonally flooded/saturated (PFOE) wetland, while the HGM class is Depressional Closed Non-Permanent (DCNP).

Vegetation within Wetland C is dominated by a mostly mature Oregon ash stand, with relatively sparse understory in many places due to prolonged seasonal ponding. Scattered Pacific willow (Salix lasiandra, FACW) trees are also present within the depression. Shrubs include willows, hardhack spirea, and clustered rose (Rosa pisocarpa, FAC). Emergent cover is sparser at the north end of the wetland, due to increased duration and depth of inundation, in addition to a dense ash overstory. Observed vegetation is limited to small percentages of shiny geranium, annual bluegrass (Poo annua, FAC) and bedstraw. The south end includes spreading rush (Juncus patens, FACW), taperfruit shortscale sedge (Carex leptopoda, FAC), largeleaf avens (Geum macrophyllum, FAC), shiny geranium, and slough sedge (Carex obnupta, OBL).

Woody species encountered along the upland edge included Oregon white oak, madrone, Saskatoon serviceberry, beaked hazelnut (Corylus cornuta, FACU), snowberry, salal (Gaultheria shallon, FACU), California dewberry (Rubus ursinus, FACU), and poison oak. Herbaceous species included shiny geranium and sword fern.

The soils within Wetland C were silt loams that generally met the redox dark surface and/or depleted matrix hydric soil indicators. The large depressional area was deeply ponded (over 3 feet deep in some places) for most of the winter and spring months of 2017; access to soils along the wetland edge (as indicated by hydrophytic vegetation) was not feasible until the month of June. Indicators of the extensive seasonal ponding (observed after waters had receded) included algal mats, sparsely vegetated concave surface, water stained leaves, and oxidized rhizospheres. Fieldwork in the fall of 2019 confirmed the prior delineation and a sample pit near the north end, where seasonal ponding would be the deepest revealed the presence of hydric soils beginning just below the organic horizon and extending to a depth of at least the documented depth of 16 inches.

## Roadside Ditch

An excavated roadside ditch extends for much of the northern boundary of the study area west of Wetland A along Tualatin-Sherwood Road. The ditch conveys stormwater runoff from the road as well as groundwater inputs from the study area. It also receives seasonal runoff from Wetland A, though it is not possible to determine the extent of backwater flooding from Wetland A from inflow down the ditch from the west. A 36 -inch diameter culvert beneath Tualatin-Sherwood Road conveys these combined flows northward beneath the roadway and toward Hedges Creek. The ditch has been excavated from uplands, with the exception of the portion of ditch immediately adjacent to Wetland A. A 12-inch culvert allows stormwater flows within the ditch to pass beneath the site's driveway. This culvert, however, is well upslope from the section of ditch that meets all three wetland indicators. A total of 0.03 acre of the roadside ditch, the section closest to Wetland A, meets wetland criteria.

## F. Deviation from Local Wetland Inventory or Nationa Wetland Inventory

A Local Wetland Inventory (LWI) has been prepared for both the cities of Sherwood and Tualatin; however, neither inventory's scope included the study area. The National Wetland Inventory (NWI) mapping depicts a single, narrow emergent wetland within Tax Lot 1100. This feature extends unbroken from south of the property through both Wetland C and Wetland A. PHS' findings, by contrast, show both features with significant widths and with no surface connection between them.

## G. Mapping Method

PHS flagged the limits of the wetlands within the study area with blue tape flagging, while sample points were flagged using lime-green tape. The accuracy of the surveyed wetland boundaries is subcentimeter (survey provided by Northwest Survey, Inc.); accuracy of sample points is $+/-3$ feet.

## H. Additional Information

Roadside ditches are regulated by the Department of State Lands (DSL) according to specific criteria outlined in OAR 141-085-515 (10). Ditches are exempt from regulation if they are:
a) Ten feet wide or less at the ordinary high water line;
b) Artificially created from upland or from wetlands;
c) Not adjacent and connected or contiguous with other wetlands; and
d) Do not contain food or game fish.

All lengths of roadside ditch within the study area are less than 10 feet wide and do not contain food or game fish. A portion of ditch immediately west of Wetland A also satisfies all three wetland criteria. This section was delineated for the benefit of the Corps of Engineers (Corps) but will not be jurisdictional by DSL per the aforementioned criteria.

Ditches are evaluated for Corps regulatory jurisdiction based upon connection and flow, as well as the presence of wetland characteristics. The Corps is likely to assume jurisdiction over a roadside ditch as a water of the US if it displays evidence of an Ordinary High Water Mark (OHWM), ultimately discharges to other waters of the US, and is a Relatively Permanent Water (RPW), (i.e., is estimated to carry water for more than three months of the year). Lacking these, the Corps may also assume jurisdiction over sections of ditch that otherwise satisfy the three wetland criteria.

No portion of onsite ditch has relatively permanent flow and likewise, none provides a hydrologic connection between jurisdictional features. A portion of the roadside ditch does, however, meet all wetland criteria and has been delineated accordingly.

The identification of jurisdictional wetland within the ditch west of Wetland $A$ is counter to a prior delineation. A delineation completed for Washington County associated with the extension of SW $124^{\text {th }}$ Avenue included a portion of the right-of-way along Tualatin Sherwood Road (DSL WD\# 2014-0448; Corps number is unknown). That delineation identified a much more limited connection between roadside ditches and Wetland A (the wetland was also designated as Wetland A for the County project). The delineation also identified just a few feet of roadside ditch west of the wetland, but about 340 feet of potentially jurisdictional ditch east of the wetland. The section of roadside ditch east of Wetland A was eliminated in late 2017 or early 2018 as part of road widening activities along Tualatin Sherwood Road approaching its intersection with SW $124^{\text {th }}$ Avenue. Both sections of ditch were exempt from DSL jurisdiction per OAR 141-085-0515 (10).

## I. Results and Conclusions

As described in Section D above, PHS delineated three potentially jurisdictional wetlands within the study area, plus a roadside ditch. The total area of wetlands is 2.94 acres, as summarized in the following table.

Table 3. Total wetland within T-S Corporate Park in Sherwood, Oregon

| Water Feature | Area <br> (square feet / acre) | Cowardin Class | HGM Class |
| :--- | :---: | :---: | :---: |
| Wetland A | $102,074 / 2.34$ | PFOY/PSSY/PEMY | Slope |
| Wetland B | $1,318 / 0.03$ | PEMY | Slope/Flat |
| Wetland C | $23,442 / 0.54$ | PFOY | DCNP |
| Roadside Ditch | $1,213 / 0.03$ | PEMY | Slope |
| Total Wetlands | $\mathbf{1 2 8 , 0 4 7} / \mathbf{2 . 9 4}$ |  |  |

Of these features, it is likely that both Wetlands A and C will be regulated by DSL and the Corps, while Wetland B is arguably isolated and may fall outside of Corps jurisdiction. In addition, the roadside ditch is likely to be regulated throughout is length by the Corps; however, DSL may only take jurisdiction of those portions of the ditch immediately adjacent to Wetland A.

## J. Required Disclaimer

This report documents the investigation, best professional judgment and conclusions of the investigators. It is correct and complete to the best of our knowledge. It should be considered a Preliminary Jurisdictional Determination of wetlands and other waters and used at your own risk unless it has been reviewed and approved in writing by the Oregon Department of State Lands in accordance with OAR 141-090-0005 through 141-090-0055.

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US Geologic Survey, 2014. 7.5-minute topographic map, Sherwood, OR quadrangle.
(http://viewer/nationalmap.gov/basic)

## Appendix A

Figures







x:Project Directories\6100\6163 ORR Property\AutoCAD\Plot Dwgs)Fig6 WetDel ov.dwg, 1/3/2020 3:01:13 PM, AutoCAD PDF (High Quality Print).pc3


X:IProject Directories|6100|6163 ORR Property)AutoCAD\Plot Dwgs)Fig6A WetDel.dwg, 1/7/2020 12:48:19 PM, AutoCAD PDF (High Quality Print).pc3


X:|Project Directories|6100|6163 ORR Property)AutoCAD\Plot Dwgs)|Fig6B WetDel.dwg, 1/7/2020 12:48:36 PM, AutoCAD PDF (High Quality Print).pc3

## Appendix B

## Wetland Determination Data Sheets



## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region



SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  | Is Sampled Area within a Wetland? | Yes |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes |  | No | X |  |  |  | X |
| Wetland Hydrology Present? | Yes |  | No | X |  |  |  |  |

## Remarks:

Sample pit in roadside ditch

## VEGETATION - Use scientific names of plants.



[^15]Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:



Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region



SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.


## VEGETATION - Use scientific names of plants.



## Remarks:

Rubus armeniacus is primarily rooted above the bottom of the ditch. Salix lasiandra and Juncus effusus are actually in the ditch.

Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:



Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region



SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes |  | No | X | Is Sampled Area within a Wetland? | Yes |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes | X | No |  |  |  |  | X |
| Wetland Hydrology Present? | Yes |  | No | X |  |  |  |  |

## Remarks:

Marginally hydric soils, lacking other indicators

## VEGETATION - Use scientific names of plants.



Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:



Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## Remarks:

Marginal hydrology, probably high due to heavy rains

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region



SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes | X | No | Is Sampled Area within a Wetland? | Yes | X | No |
| Wetland Hydrology Present? | Yes | X | No |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:



Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region

| Project/Site: | Orr Property, Tualatin |  | City/County: | Tualatin/Washington |  | Sampling Date: | 12/1 | 2019 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Applicant/Owner: | Trammel |  |  |  | State: | OR | Sampling Point: | 5 |
| Investigator(s): |  | S/DG | Section, Township, Range: |  | Section 28D, Township T2S, Range 1W |  |  |  |
| Landform (hillslope | race, etc.:) |  |  | Local relief (concave, convex, none): |  | none | Slope (\%): | 1 |
| Subregion (LRR): |  | LRR A | Lat: | 45.3690 | Long: | -122.8078 | Datum: | WSG85 |
| Soil Map Unit Nam | Huberly silt loam |  |  | NWI Classification: |  |  | PFO1C |  |

Are climatic/hydrologic conditions on the site typical for this time of year? Yes No_ (if no, explain in Remarks)

| Are vegetation | Soil | or Hydrology | significantly disturbed? | Are "Normal Circumstances" present? (Y/N) | Y |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Are vegetation | Soil | or Hydrology | naturally problematic? | explain any answers in Remarks.) |  |

SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes | X | No | is Sampled Area within a Wetland? | Yes | X | No |
| Wetland Hydrology Present? | Yes | X | No |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:



Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region



SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes | X | No | Is Sampled Area within a Wetland? | Yes | X | No |
| Wetland Hydrology Present? | Yes | X | No |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:



## Field Observations:

| Surface Water Present? | Yes |  | No | X | Depth (inches): |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Water Table Present? | Yes | X | No |  | Depth (inches): | 11 |
| Saturation Present? <br> (includes capillary fringe) | Yes | X | No |  | Depth (inches): | 6 |

Wetland Hydrology Present?
Yes $\quad \mathrm{X}$ No

Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region

| Project/Site: | Orr Property, Tualatin |  | City/County: | Tualatin/Washington |  |  | Sampling Date: OR | 12/13/2019 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Applicant/Owner: | Trammel Crow |  |  |  |  | State: |  | pling Point: | 7 |
| Investigator(s): | FS/DG |  | Section, Township, Range: |  | Section 28D, Township T2S, Range 1W |  |  |  |  |
| Landform (hillslope, terra | race, etc.:) |  |  | Local relief (con | ave, |  | none | Slope (\%): | 1 |
| Subregion (LRR): |  | LRR A | Lat: | 45.369 |  | Long: | -122.8076 | Datum: | WSG85 |
| Soil Map Unit Name: | Huberly silt loam |  |  |  |  | NWI Classification: |  | None |  |
| Are climatic/hydrologic | conditions on | site typical for | of year? | Yes | X | N | (if no, exp | Remarks) |  |
| Are vegetation | Soil | or Hydrology | significantly disturbed? |  | Are "Normal Circumstances" present? (Y/N) |  |  | Y |  |
| Are vegetation |  | or Hydrology | naturally problematic? If needed, explain any answers in Remarks.) |  |  |  |  |  |  |

SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  | Is Sampled Area within a Wetland? | Yes | No |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes |  | No | X |  |  |  | X |
| Wetland Hydrology Present? | Yes |  | No | X |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


[^16]Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:



Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region

| Project/Site: | Orr Property, Tualatin |  | City/County: | Tualatin/Washington |  |  | Sampling Date: OR | 10/15/2019 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Applicant/Owner: | Trammel Crow |  |  |  |  | State: |  | pling Point: | 8 |
| Investigator(s): | MS/SE |  | Section, Township, Range: |  | Section 28D, Township T2S, Range 1W |  |  |  |  |
| Landform (hillslope, terra | race, etc.:) |  |  | Local relief (con | ave, |  | none | Slope (\%): | 1 |
| Subregion (LRR): |  | RR A | Lat: | 45.368 |  | Long: | -122.8082 | Datum: | WSG85 |
| Soil Map Unit Name: | Huberly silt loam |  |  |  |  | NWI Classification: |  | None |  |
| Are climatic/hydrologic | conditions on | site typical for | of year? | Yes | X | N | (if no, exp | Remarks) |  |
| Are vegetation | Soil | or Hydrology | significantly disturbed? |  | Are "Normal Circumstances" present? (Y/N) |  |  | Y |  |
| Are vegetation |  | or Hydrology | naturally problematic? If needed, explain any answers in Remarks.) |  |  |  |  |  |  |

SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  | Is Sampled Area within a Wetland? | Yes | No |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes |  | No | X |  |  |  | X |
| Wetland Hydrology Present? | Yes |  | No | X |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


## Remarks:

Sample right at the edge of the thicket; represents shrub and herb areas.

Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:

| Primary Indicators (minimum of one required; check all that apply) |  |  |  |  |  |  | Secondary Indicators (2 or more required) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Surface Water (A1) |  |  |  | Water stained Leaves (B9) (Except MLRA |  |  | Water stained Leaves (B9) |  |  |
| High Water Table (A2) |  |  |  | 1, 2, 4A, and 4B) |  |  |  | and |  |
| Saturation (A3) |  |  |  | Salt Crust (B11) |  |  | Drai | (B10) |  |
| Water Marks (B1) |  |  |  | Aquatic Invertebrates (B13) |  |  | Dry | Ta |  |
| Sediment Deposits (B2) |  |  |  | Hydrogen Sulfide Odor (C1) |  |  | Satu | on A | mag |
| Drift Deposits (B3) |  |  |  | Oxidized Rhizospheres along Living Roots (C3) |  |  | Ge | tion |  |
| Algal Mat or Crust (B4) |  |  |  | Presence of Reduced Iron (C4) |  |  | Shall | (D3) |  |
| Iron Deposits (B5) |  |  |  | Recent Iron Reduction in Plowed Soils (C6) |  |  | Fac | (D5) |  |
| Surface Soil Cracks (B6) |  |  |  | Stunted or Stressed Plants (D1) (LRR A) |  |  | Rai | ds (D |  |
| Inundation Visible on Aerial Imagery (B7) |  |  |  | Other (Explain in Remarks) |  |  | Fros | mock |  |
| Sparsely Vegetated Concave Surface (B8) |  |  |  |  |  |  |  |  |  |
| Field Observations: |  |  |  | Depth (inches): |  | Wetland Hydrology Present? |  |  |  |
| Surface Water Present? |  |  | X |  |  |  |  |  |  |
| Water Table Present? | Yes |  | X | Depth (inches): | $>15$ |  |  |  |  |
| Saturation Present? (includes capillary fringe) | Yes |  | X | Depth (inches): | >15 |  | Yes | No | X |

Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region



SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes | X | No | Is Sampled Area within a Wetland? | Yes | X | No |
| Wetland Hydrology Present? | Yes | X | No |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


[^17]Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


## HYDROLOGY

## Wetland Hydrology Indicators:



Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## Remarks:

This pit is within a few feet of a narrow ditch about two feet deep located near the western wetland boundary. When full this area would be saturated, with a shallow water table. When not full of water the ditch likely lowers local water tables and results in drier conditions immediately along it.

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region

| Project/Site: | Orr Property, Tualatin |  | City/County: | Tualatin/Washington |  |  | Sampling Date: OR | 10/15/2019 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Applicant/Owner: | Trammel Crow |  |  |  |  | State: |  | pling Point: | 10 |
| Investigator(s): | MS/SE |  | Section, Township, Range: |  | Section 28D, Township T2S, Range 1W |  |  |  |  |
| Landform (hillslope, te | race, etc.:) |  |  | Local relief (con | ave, |  | none | Slope (\%): | 1 |
| Subregion (LRR): |  | LRR A | Lat: | 45.368 |  | Long: | -122.8080 | Datum: | WSG85 |
| Soil Map Unit Name: | Aloha silt loam |  |  |  |  | NWI Classification: |  | None |  |
| Are climatic/hydrologic | conditions on | site typical for | of year? | Yes | X | , | (if no, exp | Remarks) |  |
| Are vegetation |  | or Hydrology | significantly disturbed? |  | Are "Normal Circumstances" present? (Y/N) |  |  | Y |  |
| Are vegetation |  | or Hydrology | naturally problematic? If needed, explain any answers in Remarks.) |  |  |  |  |  |  |

SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes | X | No | is Sampled Area within a Wetland? | Yes | X | No |
| Wetland Hydrology Present? | Yes | X | No |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:



Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region

| Project/Site: | Orr Property, Tualatin |  | City/County: | Tualatin/Washington |  |  | Sampling Date: OR | 10/15/2019 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Applicant/Owner: | Trammel Crow |  |  |  |  | State: |  | pling Point: | 11 |
| Investigator(s): | MS/SE |  | Section, Township, Range: |  | Section 28D, Township T2S, Range 1W |  |  |  |  |
| Landform (hillslope, te | race, etc.:) |  |  | Local relief (con | ave, |  | none | Slope (\%): | 2 |
| Subregion (LRR): |  | LRR A | Lat: | 45.368 |  | Long: | -122.8080 | Datum: | WSG85 |
| Soil Map Unit Name: | Aloha silt loam |  |  |  |  | NWI Classification: |  | None |  |
| Are climatic/hydrologic | conditions on | site typical for | of year? | Yes | X | , | (if no, exp | Remarks) |  |
| Are vegetation | Soil | or Hydrology | significantly disturbed? |  | Are "Normal Circumstances" present? (Y/N) |  |  | Y |  |
| Are vegetation |  | or Hydrology | naturally problematic? If needed, explain any answers in Remarks.) |  |  |  |  |  |  |

SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  | Is Sampled Area within a Wetland? | Yes | No |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes |  | No | X |  |  |  | X |
| Wetland Hydrology Present? | Yes |  | No | X |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


[^18]Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:

| Primary Indicators (minimum of one required; check all that apply) |  |  |  |  |  |  | Secondary Indicators (2 or more required) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Surface Water (A1) |  |  |  | Water stained Leaves (B9) (Except MLRA |  |  | Water stained Leaves (B9) |  |  |
| High Water Table (A2) |  |  |  | 1, 2, 4A, and 4B) |  |  |  | and |  |
| Saturation (A3) |  |  |  | Salt Crust (B11) |  |  | Drain | (B10) |  |
| Water Marks (B1) |  |  |  | Aquatic Invertebrates (B13) |  |  | Dry-S | T T |  |
| Sediment Deposits (B2) |  |  |  | Hydrogen Sulfide Odor (C1) |  |  | Satur | on A | mag |
| Drift Deposits (B3) |  |  |  | Oxidized Rhizospheres along Living Roots (C3) |  |  | Geo | ion |  |
| Algal Mat or Crust (B4) |  |  |  | Presence of Reduced Iron (C4) |  |  | Shall | (D3) |  |
| Iron Deposits (B5) |  |  |  | Recent Iron Reduction in Plowed Soils (C6) |  |  | Fac-N | (D5) |  |
| Surface Soil Cracks (B6) |  |  |  | Stunted or Stressed Plants (D1) (LRR A) |  |  | Rais | ds (D |  |
| Inundation Visible on Aerial Imagery (B7) |  |  |  | Other (Explain in Remarks) |  |  | Frost-Heave Hummocks (D7) |  |  |
| Sparsely Vegetated Concave Surface (B8) |  |  |  |  |  |  |  |  |  |
| Field Observations |  |  |  |  |  |  |  |  |  |
| Surface Water Present? |  |  | X | Depth (inches): |  |  |  |  |  |
| Water Table Present? | Yes |  | X | Depth (inches): | $>14$ | Wetland Hyd | logy Present? |  |  |
| Saturation Present? <br> (includes capillary fringe) | Yes |  | X | Depth (inches): | >14 |  | Yes | No | X |

Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region



SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  | Is Sampled Area within a Wetland? | Yes | No |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes |  | No | X |  |  |  | X |
| Wetland Hydrology Present? | Yes |  | No | X |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


## Remarks:

Weakly hydrophytic; doesn't meet $\mathrm{Pl}<3.0$

Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:



Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region



SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes | X | No | Is Sampled Area within a Wetland? | Yes | X | No |
| Wetland Hydrology Present? | Yes | X | No |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


## Remarks:

At edge of Fraxinus latifolia seedling/sapling stand; dense growth of 1-3 inch diameter ash, nothing over 12 feet tall

Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:



Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region



SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  | Is Sampled Area within a Wetland? | Yes | No |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes |  | No | X |  |  |  | X |
| Wetland Hydrology Present? | Yes |  | No | X |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


Marginally hydrophytic based on dominants; does not meet $\mathrm{Pl}<3.0$

Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:

| Primary Indicators (minimum of one required; check all that apply) |  |  |  |  |  |  | Secondary Indicators (2 or more required) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Surface Water (A1) |  |  |  | Water stained Leaves (B9) (Except MLRA |  |  | Water stained Leaves (B9) |  |  |
| High Water Table (A2) |  |  |  | 1, 2, 4A, and 4B) |  |  |  | and |  |
| Saturation (A3) |  |  |  | Salt Crust (B11) |  |  | Dra | (B10) |  |
| Water Marks (B1) |  |  |  | Aquatic Invertebrates (B13) |  |  | Dry | T T |  |
| Sediment Deposits (B2) |  |  |  | Hydrogen Sulfide Odor (C1) |  |  | Satu | on A | mag |
| Drift Deposits (B3) |  |  |  | Oxidized Rhizospheres along Living Roots (C3) |  |  | Ge | ion |  |
| Algal Mat or Crust (B4) |  |  |  | Presence of Reduced Iron (C4) |  |  | Shall | (D3) |  |
| Iron Deposits (B5) |  |  |  | Recent Iron Reduction in Plowed Soils (C6) |  |  | Fac | (D5) |  |
| Surface Soil Cracks (B6) |  |  |  | Stunted or Stressed Plants (D1) (LRR A) |  |  |  | ds (D |  |
| Inundation Visible on Aerial Imagery (B7) |  |  |  | Other (Explain in Remarks) |  |  | Frost-Heave Hummocks (D7) |  |  |
| Sparsely Vegetated Concave Surface (B8) |  |  |  |  |  |  |  |  |  |
| Field Observations |  |  |  |  |  |  |  |  |  |
| Surface Water Present? |  |  | X | Depth (inches): |  |  |  |  |  |
| Water Table Present? | Yes |  | X | Depth (inches): | $>16$ | Wetland Hyd | logy Present? |  |  |
| Saturation Present? <br> (includes capillary fringe) | Yes |  | X | Depth (inches): | >16 |  | Yes | No | X |

Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region



SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes | X | No | Is Sampled Area within a Wetland? | Yes | X | No |
| Wetland Hydrology Present? | Yes | X | No |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


[^19]Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:

| Primary Indicators (minimum of one required; che |  | Secondary Indicators (2 or more required) |  |
| :---: | :---: | :---: | :---: |
| Surface Water (A1) | Water stained Leaves (B9) (Except MLRA |  | Water stained Leaves (B9) |
| High Water Table (A2) | 1,2, 4A, and 4B) |  | (MLRA1, 2, 4A, and 4B) |
| Saturation (A3) | Salt Crust (B11) |  | Drainage Patterns (B10) |
| Water Marks (B1) | Aquatic Invertebrates (B13) |  | Dry-Season Water Table (C2) |
| Sediment Deposits (B2) | Hydrogen Sulfide Odor (C1) |  | Saturation Visible on Aerial Imagery (C9) |
| Drift Deposits (B3) | Oxidized Rhizospheres along Living Roots (C3) | X | Geomorphic Position (D2) |
| Algal Mat or Crust (B4) | Presence of Reduced Iron (C4) |  | Shallow Aquitard (D3) |
| Iron Deposits (B5) | Recent Iron Reduction in Plowed Soils (C6) |  | Fac-Neutral Test (D5) |
| Surface Soil Cracks (B6) | Stunted or Stressed Plants (D1) (LRR A) | X | Raised Ant Mounds (D6) (LRR A) |
| Inundation Visible on Aerial Imagery (B7) | Other (Explain in Remarks) |  | Frost-Heave Hummocks (D7) |

## Field Observations:

| Surface Water Present? | Yes | No | X | Depth (inches): |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Water Table Present? | Yes | No | X | Depth (inches): | >15 |
| Saturation Present? <br> (includes capillary fringe) | Yes | No | X | Depth (inches): | >15 |

Wetland Hydrology Present?
Yes $\quad \mathrm{X}$ No

Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region



SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  | Is Sampled Area within a Wetland? | Yes | No |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes |  | No | X |  |  |  | X |
| Wetland Hydrology Present? | Yes |  | No | X |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


[^20]Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:
Depleted Matrix starts below 11 inches therefore does not meet depth requirement for F3.

## HYDROLOGY

## Wetland Hydrology Indicators:



Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region



SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  | Is Sampled Area within a Wetland? | Yes | No |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes |  | No | X |  |  |  | X |
| Wetland Hydrology Present? | Yes | X | No |  |  |  |  |  |

VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


## HYDROLOGY

## Wetland Hydrology Indicators:



Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

Remarks:
Active hydrology indicators appear to be in response to recent heavy precipitation, not wetland condition

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region



SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes | X | No | Is Sampled Area within a Wetland? | Yes | X | No |
| Wetland Hydrology Present? | Yes | X | No |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:



Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region

| Project/Site: | ORR Property |  | City/County: | Tualatin/Washington |  |  | Sampling Date: OR | 4/28/2017 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Applicant/Owner: | Trammel Crow Co. |  |  |  |  | State: |  | pling Point: | 19 |
| Investigator(s): | FS/DG |  | Section, Township, Range: |  | Section 28D, Township T2S, Range 1W |  |  |  |  |
| Landform (hillslope, te | rrace, etc.:) |  |  | Local relief (con | ave, |  | none | Slope (\%): | 2 |
| Subregion (LRR): |  | LRR A | Lat: | 45.368 |  | Long: | -122.80821 | Datum: | WGS84 |
| Soil Map Unit Name: | Aloha silt loam |  |  |  |  | NWI Classification: |  | None |  |
| Are climatic/hydrologic | conditions on | site typical for | of year? | Yes | X | , | (if no, exp | Remarks) |  |
| Are vegetation | Soil | or Hydrology | significantly disturbed? |  | Are "Normal Circumstances" present? (Y/N) |  |  | Y |  |
| Are vegetation |  | or Hydrology | naturally problematic? If needed, explain any answers in Remarks.) |  |  |  |  |  |  |

SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes | X | No | Is Sampled Area within a Wetland? | Yes | X | No |
| Wetland Hydrology Present? | Yes | X | No |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


[^21]Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:



Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region



SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes |  | No | X | Is Sampled Area within a Wetland? | Yes | No |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes |  | No | X |  |  |  | X |
| Wetland Hydrology Present? | Yes | X | No |  |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


[^22]Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:



Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

Remarks:
Hydrology present due to recent rains; possibly perched on tight soil layer

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region

| Project/Site: | Orr Property, Tualatin |  | City/County: | Tualatin/Washington |  |  | Sampling Date: OR | 10/15/2019 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Applicant/Owner: | Trammel Crow |  |  |  |  | State: |  | pling Point: | 21 |
| Investigator(s): | MS/SE |  | Section, Township, Range: |  | Section 28D, Township 2S, Range 1W |  |  |  |  |
| Landform (hillslope, te | race, etc.:) |  |  | Local relief (con | ave, |  | none | Slope (\%): | 10\% |
| Subregion (LRR): |  | LRR A | Lat: | 45.367 |  | Long: | -122.8085 | Datum: | WSG85 |
| Soil Map Unit Name: | Aloha silt loam |  |  |  |  | NWI Classification: |  | None |  |
| Are climatic/hydrologic | conditions on | site typical for | of year? | Yes | X | , | (if no, exp | Remarks) |  |
| Are vegetation | Soil | or Hydrology | significantly disturbed? |  | Are "Normal Circumstances" present? (Y/N) |  |  | Y |  |
| Are vegetation |  | or Hydrology | naturally problematic? If needed, explain any answers in Remarks.) |  |  |  |  |  |  |

SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  | Is Sampled Area within a Wetland? | Yes | No |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes |  | No | X |  |  |  | X |
| Wetland Hydrology Present? | Yes |  | No | X |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


[^23]Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:

| Primary Indicators (minimum of one required; check all that apply) |  |  |  |  |  |  | Secondary Indicators (2 or more required) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Surface Water (A1) |  |  |  | Water stained Leaves (B9) (Except MLRA |  |  | Water stained Leaves (B9) |  |  |
| High Water Table (A2) |  |  |  | 1, 2, 4A, and 4B) |  |  |  | and |  |
| Saturation (A3) |  |  |  | Salt Crust (B11) |  |  | Drai | (B10) |  |
| Water Marks (B1) |  |  |  | Aquatic Invertebrates (B13) |  |  | Dry | Ta |  |
| Sediment Deposits (B2) |  |  |  | Hydrogen Sulfide Odor (C1) |  |  | Satu | on A | mag |
| Drift Deposits (B3) |  |  |  | Oxidized Rhizospheres along Living Roots (C3) |  |  | Geo | tion ( |  |
| Algal Mat or Crust (B4) |  |  |  | Presence of Reduced Iron (C4) |  |  | Shall | (D3) |  |
| Iron Deposits (B5) |  |  |  | Recent Iron Reduction in Plowed Soils (C6) |  |  | Fac | (D5) |  |
| Surface Soil Cracks (B6) |  |  |  | Stunted or Stressed Plants (D1) (LRR A) |  |  | Rai | ds (D | R A) |
| Inundation Visible on Aerial Imagery (B7) |  |  |  | Other (Explain in Remarks) |  |  | Fros | mock |  |
| Sparsely Vegetated Concave Surface (B8) |  |  |  |  |  |  |  |  |  |
| Field Observations: |  |  |  | Depth (inches): |  | Wetland Hydrology Present? |  |  |  |
| Surface Water Present? |  |  | X |  |  |  |  |  |  |
| Water Table Present? | Yes |  | X | Depth (inches): | $>16$ |  |  |  |  |
| Saturation Present? <br> (includes capillary fringe) | Yes |  | X | Depth (inches): | >16 |  | Yes | No | X |

Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region

| Project/Site: | Orr Property, Tualatin |  | City/County: | Tualatin/Washington |  |  | Sampling Date OR | 10/15/2019 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Applicant/Owner: | Trammel Crow |  |  |  |  | State: |  | pling Point: | 22 |
| Investigator(s): | MS/SE |  | Section, Township, Range: |  | Section 28D, Township 2S, Range 1W |  |  |  |  |
| Landform (hillslope, te | rrace, etc.:) |  |  | Local relief (co | ave, |  | none | Slope (\%): | 2 |
| Subregion (LRR): |  | LRR A | Lat: | 45.36 |  | Long: | -122.8086 | Datum: | WSG85 |
| Soil Map Unit Name: | Aloha silt loam |  |  |  |  | NWI Classification: |  | None |  |
| Are climatic/hydrologic | conditions | site typical for | of year? | Yes | X |  | _ ${ }^{\text {if no, exp }}$ | Remarks) |  |
| Are vegetation | Soil | or Hydrology | significantly disturbed? |  | Are "Normal Circumstances" present? (Y/N) |  |  | Y |  |
| Are vegetation |  | or Hydrology | naturally problematic? If needed, explain any answers in Remarks.) |  |  |  |  |  |  |

SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes | X | No | Is Sampled Area within a Wetland? | Yes | X | No |
| Wetland Hydrology Present? | Yes | X | No |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:



Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:
Observations in the spring of 2017 confirmed the presence of seasonally high water table in this vicinity.

## Remarks:

This sample point is located at the base of a slope break. It is also at the former edge of acricultural activities and has been disturbed for decades. Vegetation has therefore been disturbed and might otherwise satisfy the FAC-Neutral test.

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region

| Project/Site: | Orr Property, Tualatin |  | City/County: | Tualatin/Washington |  |  | Sampling Date: <br> OR | 10/15/2019 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Applicant/Owner: | Trammel Crow |  |  |  |  | State: |  | pling Point: | 23 |
| Investigator(s): | MS/SE |  | Section, Township, Range: |  | Section 28D, Township 2S, Range 1W |  |  |  |  |
| Landform (hillslope, te | race, etc.:) |  |  | Local relief (con | ave, |  | none | Slope (\%): | 3 |
| Subregion (LRR): |  | LRR A | Lat: | 45.367 |  | Long: | -122.8086 | Datum: | WSG85 |
| Soil Map Unit Name: | Aloha silt loam |  |  |  |  | NWI Classification: |  | None |  |
| Are climatic/hydrologic | conditions on | site typical for | of year? | Yes | X | , | (if no, exp | Remarks) |  |
| Are vegetation | Soil | or Hydrology | significantly disturbed? |  | Are "Normal Circumstances" present? (Y/N) |  |  | Y |  |
| Are vegetation |  | or Hydrology | naturally problematic? If needed, explain any answers in Remarks.) |  |  |  |  |  |  |

SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  | Is Sampled Area within a Wetland? | Yes | No |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes |  | No | X |  |  |  | X |
| Wetland Hydrology Present? | Yes |  | No | X |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:

| Primary Indicators (minimum of one required; check all that apply) |  |  |  |  |  |  | Secondary Indicators (2 or more required) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Surface Water (A1) |  |  |  | Water stained Leaves (B9) (Except MLRA |  |  | Water stained Leaves (B9) |  |  |
| High Water Table (A2) |  |  |  | 1, 2, 4A, and 4B) |  |  |  | and |  |
| Saturation (A3) |  |  |  | Salt Crust (B11) |  |  | Drai | (B10) |  |
| Water Marks (B1) |  |  |  | Aquatic Invertebrates (B13) |  |  | Dry | Ta |  |
| Sediment Deposits (B2) |  |  |  | Hydrogen Sulfide Odor (C1) |  |  | Satu | on A | mag |
| Drift Deposits (B3) |  |  |  | Oxidized Rhizospheres along Living Roots (C3) |  |  | Geo | tion ( |  |
| Algal Mat or Crust (B4) |  |  |  | Presence of Reduced Iron (C4) |  |  | Shall | (D3) |  |
| Iron Deposits (B5) |  |  |  | Recent Iron Reduction in Plowed Soils (C6) |  |  | Fac | (D5) |  |
| Surface Soil Cracks (B6) |  |  |  | Stunted or Stressed Plants (D1) (LRR A) |  |  | Rai | ds (D | R A) |
| Inundation Visible on Aerial Imagery (B7) |  |  |  | Other (Explain in Remarks) |  |  | Fros | mock |  |
| Sparsely Vegetated Concave Surface (B8) |  |  |  |  |  |  |  |  |  |
| Field Observations: |  |  |  | Depth (inches): |  | Wetland Hydrology Present? |  |  |  |
| Surface Water Present? |  |  | X |  |  |  |  |  |  |
| Water Table Present? | Yes |  | X | Depth (inches): | $>16$ |  |  |  |  |
| Saturation Present? <br> (includes capillary fringe) | Yes |  | X | Depth (inches): | >16 |  | Yes | No | X |

Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region

| Project/Site: | Orr Property, Tualatin |  | City/County: | Tualatin/Washington |  |  | Sampling Date: OR | 10/15/2019 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Applicant/Owner: | Trammel Crow |  |  |  |  | State: |  | pling Point: | 24 |
| Investigator(s): | MS/SE |  | Section, Township, Range: |  | Section 28D, Township 2S, Range 1W |  |  |  |  |
| Landform (hillslope, te | race, etc.:) | gently slope |  | Local relief (concave, convex, none): |  |  | none | Slope (\%): | 2 |
| Subregion (LRR): |  | LRR A | Lat: | 45.367 |  | Long: | -122.8088 | Datum: | WSG85 |
| Soil Map Unit Name: | Aloha silt loam |  |  |  |  | NWI Cla | ication: | None |  |
| Are climatic/hydrologic | conditions on | site typical for | of year? | Yes | X | , | (if no, exp | Remarks) |  |
| Are vegetation | Soil | or Hydrology | significantly dis | bed? | Are | umstanc | present? (Y/N) | Y |  |
| Are vegetation |  | or Hydrology | naturally proble | atic? If needed, | explain | rs in Rem |  |  |  |

SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | No | X | Is Sampled Area within a Wetland? | Yes | No |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes | No | X |  |  |  | X |
| Wetland Hydrology Present? | Yes | No | X |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


## Remarks:

Herbs continued: Epilobium ciliatum - FACW = 1\%, Plantago lanceolata - FACU $=1 \%$

Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:

| Primary Indicators (minimum of one required; check all that apply) |  |  |  |  |  |  | Secondary Indicators (2 or more required) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Surface Water (A1) |  |  |  | Water stained Leaves (B9) (Except MLRA |  |  | Water stained Leaves (B9) |  |  |
| High Water Table (A2) |  |  |  | 1, 2, 4A, and 4B) |  |  |  | and |  |
| Saturation (A3) |  |  |  | Salt Crust (B11) |  |  | Drai | (B10) |  |
| Water Marks (B1) |  |  |  | Aquatic Invertebrates (B13) |  |  | Dry | Ta |  |
| Sediment Deposits (B2) |  |  |  | Hydrogen Sulfide Odor (C1) |  |  | Satu | on A | mag |
| Drift Deposits (B3) |  |  |  | Oxidized Rhizospheres along Living Roots (C3) |  |  | Geo | tion ( |  |
| Algal Mat or Crust (B4) |  |  |  | Presence of Reduced Iron (C4) |  |  | Shall | (D3) |  |
| Iron Deposits (B5) |  |  |  | Recent Iron Reduction in Plowed Soils (C6) |  |  | Fac | (D5) |  |
| Surface Soil Cracks (B6) |  |  |  | Stunted or Stressed Plants (D1) (LRR A) |  |  | Rai | ds (D | R A) |
| Inundation Visible on Aerial Imagery (B7) |  |  |  | Other (Explain in Remarks) |  |  | Fros | mock |  |
| Sparsely Vegetated Concave Surface (B8) |  |  |  |  |  |  |  |  |  |
| Field Observations: |  |  |  | Depth (inches): |  | Wetland Hydrology Present? |  |  |  |
| Surface Water Present? |  |  | X |  |  |  |  |  |  |
| Water Table Present? | Yes |  | X | Depth (inches): | $>16$ |  |  |  |  |
| Saturation Present? <br> (includes capillary fringe) | Yes |  | X | Depth (inches): | >16 |  | Yes | No | X |

Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region



SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes | X | No | Is Sampled Area within a Wetland? | Yes | X | No |
| Wetland Hydrology Present? | Yes | X | No |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:

| Primary Indicators (minimum of one required; check all that apply) |  |  |  |  |  |  | Secondary Indicators (2 or more required) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Surface Water (A1) |  |  |  | Water stained Leaves (B9) (Except MLRA |  |  | Water stained Leaves (B9) |  |  |
| High Water Table (A2) |  |  |  | 1, 2, 4A, and 4B) |  |  |  | (MLRA1 | and |
| Saturation (A3) |  |  |  | Salt Crust (B11) |  |  |  | Drainage | (B1 |
| Water Marks (B1) |  |  |  | Aquatic Invertebrates (B13) |  |  |  | Dry-Sea | r Ta |
| Sediment Deposits (B2) |  |  |  | Hydrogen Sulfide Odor (C1) |  |  |  | Saturatio | on |
| Drift Deposits (B3) |  |  |  | X Oxidized Rhizospheres along Living Roots (C3) |  |  |  | Geomorp | tion ( |
| Algal Mat or Crust (B4) |  |  |  | Presence of Reduced Iron (C4) |  |  |  | Shallow | (D3) |
| Iron Deposits (B5) |  |  |  | Recent Iron Reduction in Plowed Soils (C6) |  |  |  | Fac-Neut | (D5) |
| Surface Soil Cracks (B6) |  |  |  | Stunted or Stressed Plants (D1) (LRR A) |  |  |  | Raised A |  |
| Inundation Visible on Aerial Imagery (B7) |  |  |  | Other (Explain in Remarks) |  |  |  | Frost-He | mock |
| Sparsely Vegetated Concave Surface (B8) |  |  |  |  |  |  |  |  |  |
| Field Observations: |  |  |  | Depth (inches): |  | Wetland Hydrology Present? |  |  |  |
| Surface Water Present? |  |  | X |  |  |  |  |  |  |
| Water Table Present? | Yes |  | X | Depth (inches): | >14 |  |  |  |  |
| Saturation Present? <br> (includes capillary fringe) | Yes |  | X | Depth (inches): | $>14$ |  | Yes | X |  |

Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region



SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes | X | No | is Sampled Area within a Wetland? | Yes | X | No |
| Wetland Hydrology Present? | Yes | X | No |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


[^24]Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:



Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region



SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | No | X | Is Sampled Area within a Wetland? | Yes |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes | No | X |  |  |  | X |
| Wetland Hydrology Present? | Yes | No | X |  |  |  |  |

Remarks:

VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:

| Primary Indicators (minimum of one required; check all that apply) |  |  |  |  |  |  | Secondary Indicators (2 or more required) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Surface Water (A1) |  |  |  | Water stained Leaves (B9) (Except MLRA |  |  | Water stained Leaves (B9) |  |  |
| High Water Table (A2) |  |  |  | 1, 2, 4A, and 4B) |  |  |  | and |  |
| Saturation (A3) |  |  |  | Salt Crust (B11) |  |  | Dra | (B10) |  |
| Water Marks (B1) |  |  |  | Aquatic Invertebrates (B13) |  |  | Dry | T T |  |
| Sediment Deposits (B2) |  |  |  | Hydrogen Sulfide Odor (C1) |  |  | Satu | on A | mag |
| Drift Deposits (B3) |  |  |  | Oxidized Rhizospheres along Living Roots (C3) |  |  | Ge | ion |  |
| Algal Mat or Crust (B4) |  |  |  | Presence of Reduced Iron (C4) |  |  | Shall | (D3) |  |
| Iron Deposits (B5) |  |  |  | Recent Iron Reduction in Plowed Soils (C6) |  |  | Fac | (D5) |  |
| Surface Soil Cracks (B6) |  |  |  | Stunted or Stressed Plants (D1) (LRR A) |  |  |  | ds (D |  |
| Inundation Visible on Aerial Imagery (B7) |  |  |  | Other (Explain in Remarks) |  |  | Frost-Heave Hummocks (D7) |  |  |
| Sparsely Vegetated Concave Surface (B8) |  |  |  |  |  |  |  |  |  |
| Field Observations |  |  |  |  |  |  |  |  |  |
| Surface Water Present? |  |  | X | Depth (inches): |  |  |  |  |  |
| Water Table Present? | Yes |  | X | Depth (inches): | $>16$ | Wetland Hyd | logy Present? |  |  |
| Saturation Present? <br> (includes capillary fringe) | Yes |  | X | Depth (inches): | >16 |  | Yes | No | X |

Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region



SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes | X | No | Is Sampled Area within a Wetland? | Yes | X | No |
| Wetland Hydrology Present? | Yes | X | No |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:



Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

Remarks:
Ponding observed for extended period during winter and spring months

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region



SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes | X | No | is Sampled Area within a Wetland? | Yes | X | No |
| Wetland Hydrology Present? | Yes | X | No |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


## Remarks:

Meets $\mathrm{Pl}<3.0$ (not dominance test); sparse groundcover due to extended seasonal ponding

Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:



Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

Remarks:
Ponding observed for extended period during winter and spring months

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region

| Project/Site: | ORR Property |  | City/County: | Tualatin/Washington |  |  | Sampling Date: | 6/28/2017 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Applicant/Owner: | Trammel Crow Co. |  |  |  |  | State: | OR | pling Point: | 30 |
| Investigator(s): | DG |  | Section, Township, Range: |  | Section 28D, Township 2S, Range 1W |  |  |  |  |
| Landform (hillslope, te | race, etc.:) | hillslope |  | Local relief (concave, convex, none): |  |  | none | Slope (\%): | 9 |
| Subregion (LRR): |  | LRR A | Lat: | 45.368 |  | Long: | -122.80821 | Datum: | WGS84 |
| Soil Map Unit Name: | Quatama loam |  |  |  |  | NWI Cl | fication: | None |  |
| Are climatic/hydrologic | conditions on | he site typical for | of year? | Yes | X |  | (if no, ex | Remarks) |  |
| Are vegetation | Soil | or Hydrology | significantly dis | bed? | Are | umstan | present? (Y/N) | Y |  |
| Are vegetation |  | or Hydrology | naturally proble | atic? If needed | explain | rs in Re |  |  |  |

SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | No | X | Is Sampled Area within a Wetland? | Yes |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes | No | X |  |  |  | X |
| Wetland Hydrology Present? | Yes | No | X |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


## HYDROLOGY

## Wetland Hydrology Indicators:



Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

Remarks:
Based on landscape position and lack of hydrophytic vegetation, presumed to lack hydrologic indicators despite limited depth of sampling.

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region



SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | X | No |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes | X | No | is Sampled Area within a Wetland? | Yes | X | No |
| Wetland Hydrology Present? | Yes | X | No |  |  |  |  |

## Remarks:

VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:



Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## WETLAND DETERMINATION DATA FORM - Western Mountains, Valleys, and Coast Region

| Project/Site: | Orr Property, Tualatin |  | City/County: | Tualatin/Washington |  |  | Sampling Date: <br> OR | 10/15/2019 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Applicant/Owner: | Trammel Crow |  |  |  |  | State: |  | pling Point: | 32 |
| Investigator(s): | MS/SE |  | Section, Township, Range: |  | Section 28D, Township 2S, Range 1W |  |  |  |  |
| Landform (hillslope, terra | race, etc.:) |  |  | Local relief (con | ave, |  | none | Slope (\%): | 10\% |
| Subregion (LRR): |  | LRR A | Lat: | 45.366 |  | Long: | -122.8110 | Datum: | WSG85 |
| Soil Map Unit Name: | Xerochrepts rock outcrop complex |  |  |  |  | NWI Cla | cation: None |  |  |
| Are climatic/hydrologic | conditions on | site typical for | of year? | Yes | X | , | (if no, exp | Remarks) |  |
| Are vegetation |  | or Hydrology | significantly disturbed? |  | Are "Normal Circumstances" present? (Y/N) |  |  | Y |  |
| Are vegetation |  | or Hydrology | naturally problematic? If needed, explain any answers in Remarks.) |  |  |  |  |  |  |

SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? | Yes | No | X | Is Sampled Area within a Wetland? | Yes |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hydric Soil Present? | Yes | No | X |  |  |  | X |
| Wetland Hydrology Present? | Yes | No | X |  |  |  |  |

Remarks:

VEGETATION - Use scientific names of plants.


## Remarks:

Shrubs continued: Holodiscus discolor - FACU = 2\%, Polystichum munitum - FACU - 2\%, Mahonia nervosa - (FACU) - 1\%.

Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY

## Wetland Hydrology Indicators:

| Primary Indicators (minimum of one required; check all that apply) |  |  |  |  |  |  | Secondary Indicators (2 or more required) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Surface Water (A1) |  |  |  | Water stained Leaves (B9) (Except MLRA |  |  | Water stained Leaves (B9) |  |  |
| High Water Table (A2) |  |  |  | 1, 2, 4A, and 4B) |  |  |  | and |  |
| Saturation (A3) |  |  |  | Salt Crust (B11) |  |  | Drai | (B10) |  |
| Water Marks (B1) |  |  |  | Aquatic Invertebrates (B13) |  |  | Dry | Ta |  |
| Sediment Deposits (B2) |  |  |  | Hydrogen Sulfide Odor (C1) |  |  | Satu | on A | mag |
| Drift Deposits (B3) |  |  |  | Oxidized Rhizospheres along Living Roots (C3) |  |  | Geo | tion ( |  |
| Algal Mat or Crust (B4) |  |  |  | Presence of Reduced Iron (C4) |  |  | Shall | (D3) |  |
| Iron Deposits (B5) |  |  |  | Recent Iron Reduction in Plowed Soils (C6) |  |  | Fac | (D5) |  |
| Surface Soil Cracks (B6) |  |  |  | Stunted or Stressed Plants (D1) (LRR A) |  |  | Rai | ds (D | R A) |
| Inundation Visible on Aerial Imagery (B7) |  |  |  | Other (Explain in Remarks) |  |  | Fros | mock |  |
| Sparsely Vegetated Concave Surface (B8) |  |  |  |  |  |  |  |  |  |
| Field Observations: |  |  |  | Depth (inches): |  | Wetland Hydrology Present? |  |  |  |
| Surface Water Present? |  |  | X |  |  |  |  |  |  |
| Water Table Present? | Yes |  | X | Depth (inches): | $>14$ |  |  |  |  |
| Saturation Present? <br> (includes capillary fringe) | Yes |  | X | Depth (inches): | >14 |  | Yes | No | X |

Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

## Appendix C

 Site Photos


## Photo A:

Looking south along western edge of Wetland $A$, from near TualatinSherwood Road.

Photo taken: April 28, 2017

## Photo B:

Looking northeast into Wetland A, near Sample point \#4.

Photo taken: April 28, 2017


Project \#6163 12/19/2019


Photo documentation
T-S Corporate Park—Sherwood, Oregon


## Photo C:

Looking southwest into Wetland A, with young ash saplings (left) and mature ash stand (right).

Photo taken: April 28, 2017

## Photo D:

Looking west at lower (north) end of Wetland A and culvert feeding under Tualatin-Sherwood Road.

Photo taken: December 13, 2019


Photo documentation
T-S Corporate Park—Sherwood, Oregon


## Photo E:

Looking northwest across Wetland A toward isolated upland area.

Photo taken: April 28, 2017

Photo F:
Looking east across Wetland B in agricultural field, toward Wetland A . Sample points visible in this photo reflect the location of data collected in 2017 and are not in the same locations as data collected in 2019.

Photo taken: April 28, 2017



Photo documentation
T-S Corporate Park—Sherwood, Oregon

## Photo G:

Looking north along west edge of Wetland A; Sample point \#19 visible at right.

Photo taken: April 28, 2017

## Photo H

Looking north into eastern edge of Wetland $A$, with primarily reed canarygrass, tall fescue and
Himalayan blackberry in foreground.

Photo taken: December 13, 2019



Photo documentation
T-S Corporate Park—Sherwood, Oregon


## Photo I:

Looking south across Wetland C in southern portion of study area; wetland continues offsite.

Photo taken: March 7, 2017

Photo J:
Looking northwest along the
western boundary of Wetland C .

Photo taken: October 15, 2019


Photo documentation
T-S Corporate Park—Sherwood, Oregon

## Appendix D Wetland Definitions and Methodology



## WATERS OF THE STATE AND WETLAND DEFINITION AND CRITERIA

## Regulatory Jurisdiction

Wetlands and water resources in Oregon are regulated by the Oregon Department of State Lands (DSL) under the Removal-Fill Law (ORS 196.800-196.990) and by the U.S. Army Corps of Engineers (COE) through Section 404 of the Clean Water Act.

The primary source documents for wetland delineations within Oregon is the Corps of Engineers Wetlands Delineation Manual, Technical Report Y-87-1 (Environmental Laboratory 1987) and the Interim Regional Supplement to the Corps of Engineers Wetland Delineation Manual: Western Mountains, Valleys, and Coast Region, which are recognized by both DSL and COE.

## Waters of the State and Wetland Definition

Waters of the State are defined as "natural waterways including all tidal and non-tidal bays, intermittent streams, constantly flowing streams, lakes, wetlands and other bodies of water in this state, navigable and non-navigable...". "Natural waterways" is further defined as waterways created naturally by geological and hydrological processes, waterways that would be natural but for human-caused disturbances (e.g. channelized or culverted streams, impounded waters, partially drained wetlands or ponds created in wetlands)..."(DSL, 2001).

Wetlands are defined as "those areas that are inundated or saturated by surface or ground water at a frequency and duration sufficient to support, and that under normal circumstances do support, a prevalence of vegetation typically adapted for life in saturated soil conditions" (DSL 2001).

## Wetland Criteria

Based on the above definition, three major factors characterize a wetland: hydrology, substrate, and biota.

## Wetland Hydrology

Wetland hydrology is related to duration of saturation, frequency of saturation, and critical depth of saturation. The 1987 manual defines wetland hydrology as inundation or saturation within a major portion of the root zone (usually above 12 inches), typically for at least $12.5 \%$ of the growing season. The wetland hydrology criterion can be met, however, if saturation within the major portion of the root zone is present for only $5 \%$ of the growing season, depending on other evidence.

The growing season is defined as the portion of the year when soil temperatures at 12.0 inches below the soil surface are higher than biological zero ( 41 degrees Fahrenheit, 5 degrees Celsius), but also allows approximation from frost free days, based on air temperature. The growing season for any given site or location is determined from US Natural Resources Conservation Service, (formerly Soil Conservation Service) data and information.

Wetland hydrologic indicators include the following: visual observation of inundation or saturation, watermarks, drift lines, sediment deposits, and/or oxidized rhizospheres with living roots. Oxidized rhizospheres are defined as yellowish-red zones around the roots and rhizomes of some plants that grow in frequently saturated soils. Other indicators of hydrology, including algal mats or crust, iron deposits, surface soil cracks, sparsely vegetated concave surface, salt crust, aquatic invertebrates, hydrogen sulfide odor, reduced iron, iron reduction in tilled soils, and stunted or stressed plants can also be used to determine the presence of wetland hydrology.

## Wetland Substrate (Soils)

Most wetlands are characterized by hydric soils. Hydric soils are those that are ponded, flooded, or saturated for long enough during the growing season to develop anaerobic conditions. Periodic saturation of soils causes alternation of reduced and oxidized conditions, which leads to the formation of redoximorphic features (gleying and mottling). Mineral hydric soils will be either gleyed or will have bright mottles and/or low matrix chroma. The redoximorphic feature known as gley is a result of greatly reduced soil conditions, which result in a characteristic grayish, bluish or greenish soil color. The term mottling is used to describe areas of contrasting color within a soil matrix. The soil matrix is the portion of the soil layer that has the predominant color. Soils that have brightly colored mottles and a low matrix chroma are indicative of a fluctuating water table.

Hydric soil indicators include: organic content of greater than $50 \%$ by volume, and/or presence of redoximorphic features and dark soil matrix, as determined by the use of a Munsell Soil Color Chart. This chart establishes the chroma, value and hue of soils based on comparison with color chips. Mineral hydric soil must meet one of the 16 definitions for hydric soil indicators, or be classified as a "problem soil" in the Regional Supplement.

## Wetland Biota (Vegetation)

Wetland biota is defined as hydrophytic vegetation. A hydrophyte is a plant species that is capable of growing in substrates that are periodically deficient in oxygen as a result of saturated soil conditions. The U.S. Fish and Wildlife Service, in the National List of Plant Species that Occur in Wetlands, has established five basic groups of vegetation based on their frequency of occurrence in wetlands. These categories, referred to as the "wetland indicator status", are as follows: obligate wetland plants (OBL), facultative wetland (FACW), facultative (FAC), facultative upland (FACU), and obligate upland (UPL). Table 1 gives a definition of the plant indicator codes.

## Table 1. Description of Wetland Plant Indicator Status Codes

## Indicator

Code Status

OBL Obligate wetland. Plants that always occur in standing water or in saturated soils.
FACW Facultative wetland. Plants that nearly always occur in areas of prolonged flooding or require standing water or saturated soils but may, on rare occasions, occur in non-wetlands.
FAC Facultative. Plants that occur in a variety of habitats, including wetland and mesic to xeric non-wetland habitats but commonly occur in standing water or saturated soils.
FACU Facultative upland. Plants that typically occur in xeric or mesic non-wetland habitats but may frequently occur in standing water or saturated soils.
UPL Obligate upland. Plants that rarely occur in water or saturated soils.

Observations of hydrology, soils, and vegetation, were made using the "Routine On-site" delineation method as defined in the 1987 manual and the Regional Supplement for areas that were not currently in agricultural production. One-foot diameter soil pits were excavated to 20 inches and soil profiles were examined for hydric soil and wetland hydrology field indicators. In addition, a visual absolutecover estimate of the dominant species of the plant community was performed using soil pit locations as a center of reference. Dominant plant species are based on estimates of absolute cover for herbaceous, and shrub species within a 5 foot radius of the sample point, and basal area cover for tree and woody vine species within a 30 foot radius of the sample point. Plant species in each vegetative layer, which are estimated at less than $20 \%$ of the total cover, are not considered to be dominant. The wetland indicator status is then used to determine if there is an overall dominance (greater than $50 \%$ ) of wetland or upland plant species. If less than $50 \%$ of the dominant species are hydrophytic, then the prevalence index may be used to determine if the subdominant species are hydrophytic. If the prevalence index is less than or equal to 3 , hydrophytic vegetation criterion is met.

During data collection, the soil profiles were examined for hydric soil and wetland hydrology field indicators. Plant species and cover were recorded. Data was recorded on standard data sheets, which contain the information specified in the 1987 Corps Manual and the Regional Supplement.

## WETLAND DELINEATION / DETERMINATION REPORT COVER FORM

This form must be included with any wetland delineation report submitted to the Department of State Lands for review and approval. A wetland delineation report submittal is not "complete" unless the fully completed and signed report cover form and the required fee are submitted. Attach this form to the front of an unbound report or include a hard copy of the completed form with a CD/DVD that includes a single PDF file of the report cover form and report (minimum 300 dpi resolution) and submit to: Oregon Department of State Lands, 775 Summer Street NE, Suite 100, Salem, OR 97301-1279. A single PDF attachment of the completed cover from and report may be e-mailed to Wetland_Delineation@dsl.state.or.us. For submittal of PDF files larger than 10 MB, e-mail instructions on how to access the file from your ftp or other file sharing website. Fees can be paid by check or credit card. Make the check payable to the Oregon Department of State Lands. To pay the fee by credit card, call 503-986-5200.


| Project Name: Willamette Water Supply Program | Latitude: $\mathbf{4 5 . 3 6 5 0 3 3}$ | Longitude: -122.808787 |
| :---: | :---: | :---: |
| Proposed Use: Water Treatment Plant for Willamette Water Supply Program | Tax Map \# 2S128D000100 |  |
| Project Street Address (or other descriptive location): 12900 SW Tualatin-Sherwood Rd |   <br> Township 2S Range 1W <br> Tax Lot(s) 100  | Section 28 QQ SE/SE |
| City: Sherwood County: Washington | Waterway: None NWI Quad(s): | River Mile: -- |

## Wetland Delineation Information



# Wetland Delineation Report 

# Willamette Water Supply Program Willamette Water Supply System Water Treatment Plant Site 

Prepared for:


9600 SW Oak Street, Suite 238
Tigard, OR 97223

## Prepared by:



David Evans and Associates, Inc.
2100 SW River Parkway
Portland, Oregon 97201
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## 1 INTRODUCTION

The Willamette Water Supply Program (WWSP) has been identified by the Tualatin Valley Water District (TVWD) and the City of Hillsboro ((Hillsboro), collectively referred to as the Partners)) as the next infrastructure project to deliver drinking water to municipalities in Washington County by developing the mid-Willamette River at Wilsonville as an additional water supply source.

This delineation report is one of several produced by David Evans and Associates, Inc. (DEA) for the Program. The Program has been divided into work packages (i.e., construction projects) that cover pipeline alignments (including potential alternate alignments), water reservoirs (tanks), and a new water treatment plant facility. Separate delineation reports have been prepared to match each work package study area.

This wetland delineation report only covers the study area for the proposed Willamette Water Supply System Water Treatment Plant (WWSS WTP or WTP). DEA conducted an on-site Wetland Delineation for the proposed WTP site on November 3 and 4, 2016. The site is located in unincorporated Washington County, Oregon (Township 2 South, Range 1 West, Section 28D, Willamette Meridian) (see maps in Appendix A). The site is located east of the City of Sherwood at tax lot ID 2S128D00100. Seven wetlands and no waters were delineated within the study area. In addition, one inaccessible area may contain additional wetlands.

## 2 LANDSCAPE SETTING AND LAND USE

The 49.7-acre study area is located east of the City of Sherwood, south of SW Tualatin-Sherwood Road, and between Dahlke Lane and SW 120 th Avenue. A gravel operation and a future SW $124^{\text {th }}$ Avenue extension (currently under construction) lie to the east, and a farmed field lies north of the study area. The site is densely forested with undulating topography and rocky outcroppings. Upland vegetation consists primarily of Oregon oak (Quercus garryana), Pacific madrone (Arbutus menziesii), and Douglas fir (Pseudotsuga menziesii) trees, with extensive thickets of poison oak (Toxicodendron diversilobum) in the understory. Delineated wetlands were observed in depressions and are further described in Section 6. A power line corridor that crosses the south part of the study area is maintained as shrub habitat.

## 3 SITE ALTERATIONS

The study area is situated in an undulating, undeveloped area. At the time of survey, an extension of SW $124^{\text {th }}$ Avenue was being constructed along the eastern boundary.

A power line right of way crosses the study area diagonally across the southern half. Several trails existed previous to the survey, and more were established to access the areas of potential wetlands as described in Section 5.2 Field Methods.

## 4 PRECIPITATION DATA AND ANALYSIS

Wetland delineation field work occurred on November 3 and 4, 2016. Daily and monthly precipitation data was taken from the National Weather Service (NWS) station in Portland due to a lack of WETS climate data in Beaverton for these dates. Table 1 shows the two-week precipitation total prior to the field dates. The precipitation record reveals that precipitation was within the range of normal for the short term, but was well above normal in the medium and long term. August was well below normal for precipitation, September was normal, and October was well above normal (Table 2)

Precipitation for the water year at the time of survey was $258 \%$, well above normal (Table 3). Because most of the heavy precipitation occurred in early October, site conditions were assumed to be somewhat wetter than normal. Care was given to recognize abnormal hydrologic patterns in the field, but it was not apparent that the soil profile was saturated outside of typical areas.

Table 1: Precipitation for November 2016 Field Investigations and Two Weeks Prior, in Inches

| Oct 20 | Oct 21 | Oct 22 | Oct 23 | Oct 24 | Oct 25 | Oct 26 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0.26 | 0.48 | 0.04 | 0.07 | 0.07 | Trace | 0.67 |
| Oct 27 | Oct 28 | Oct 29 | Oct 30 | Oct 31 | Nov 1 | Nov 2 |
| 0.16 | 0.01 | 0.16 | 0.11 | 0.19 | Trace | 0.05 |
| Nov 3* | Nov 4* | 2-wk Total |  |  |  |  |
| $\mathbf{0 . 0 0}$ | $\mathbf{0 . 0 0}$ | $\mathbf{2 . 2 7}$ |  |  |  |  |

*Days of field investigation. Source: (NWS 2016)
Table 2: Percent of Normal Precipitation for the 3 Months Preceding the November Field Investigation

| Month | Normal <br> Precipitation for <br> Month <br> (Inches) | Observed <br> Precipitation for <br> Month <br> (Inches) | Departure <br> from Normal <br> (inches) | Within 30\% of <br> Normal <br> Precipitation for <br> Water Year? |
| :--- | :---: | :---: | :---: | :---: |
| August | 0.71 | 0.16 | -0.55 | No (23\% of normal) |
| September | 1.54 | 1.26 | -0.28 | Yes ( $82 \%$ of normal) |
| October | 3.54 | 10.11 | +6.69 | No (296\% of normal) |

Source: (NWS 2016)

Suitable required climatological data for wetland delineations is not available for the Sherwood area. Therefore, alternate nearby data were used as follows. Daily, monthly, and water-year precipitation data were obtained from the Portland, Oregon, National Weather Service climatological data (NWS 2016). As per Oregon Department of State Lands (DSL) methods, because the WETS table was not available for Sherwood, the closest location (BEAVERTON 2 SSW, OR0595) is provided in Appendix D. For consistency, the percent of normal totals for the above tables were taken from the Portland, Oregon NWS climatological data (NWS 2016) rather than from the WETS table. The NWS does not provide readily available compiled precipitation data for Beaverton.

Table 3: Percent of Normal Precipitation for the Water Year Preceding the November Field Investigations

| Month | Normal <br> Precipitation <br> (Inches) | Observed <br> Precipitation <br> (Inches) | Departure from <br> Normal <br> (inches) | Within 30\% of <br> Normal <br> Precipitation for <br> Water Year? |
| :--- | :---: | :---: | :---: | :---: |
| November 3 <br> and 4, 2016* | 3.95 | 10.19 | +6.24 | No $(258 \%$ of <br> normal) |

*No precipitation on these dates (NWS 2016).

## 5 METHODS

### 5.1 PRELIMINARY RESOURCE REVIEW

Reference materials were reviewed prior to the field investigation to provide information regarding the possible presence of wetlands, water features, hydric soils, wetland hydrology, and site topography. The materials reviewed are referenced in Appendix E, and included the following:

- Precipitation data for Portland, Oregon (National Weather Service 2016).
- ESRI, ArcGIS Online, USA area Topographic Maps, Sherwood Valley, Oregon 1961
- ESRI, World Imagery, Aerials Express, 2010
- US Fish and Wildlife Service (USFWS), NWI, Wetlands Mapper V2, 2016
- Natural Resource Conservation Service (NCRS). 2014. Web Soil Survey, Washington County Area, Oregon.
- Tax Lots for Washington County Area, Oregon (Metro RLIS Data, 2016).

The USGS Quadrangles were examined to determine water features and topography of the site and adjacent properties that might influence on-site conditions (Appendix A: Figure 1). Figure 2 displays the study area tax lot boundaries. The NWI maps (Appendix A: Figure 3) were examined to determine if wetlands are mapped on site. The local wetland inventory (LWI) for the Sherwood area did not extend to the study area and was not examined further (DEA 1992). The Soil Survey map (Appendix A: Figure 4) was reviewed to determine if any hydric soils are mapped on site. Table 4 summarizes the soils mapped within the study area.

Table 4: Soils Mapped (NRCS 2014) as Occuring in the Project Study Area

| Map <br> Unit | Soil Series | Hydric Status | SCS Drainage <br> Description |
| :---: | :--- | :--- | :--- |
| 37 C | Quatama loam, 7 to 12 percent <br> slopes | Non-hydric | Moderately well drained |
| 38 C | Saum silt loam, 7 to 12 percent <br> slopes | Non-hydric | Well drained |
| 38 E | Saum silt loam, 20 to 30 percent <br> slopes | Non-hydric | Well drained |
| 47 D | Xerochrepts-Rock outcrop <br> complex | Non-hydric | Well drained |

### 5.2 FIELD METHODS

Wetland areas were delineated on November 3 and 4, 2016, according to the Level 2 Routine On-Site Method described in the 1987 U.S. Army Corps of Engineers (USACE) Wetland Delineation Manual (Environmental Laboratory 1987) and the Regional Supplement to the USACE Wetland Delineation Manual: Western Mountains, Valleys, and Coast Region (USACE 2010). This method requires an area to possess a prevalence of hydrophytic vegetation, hydric soils, and wetland hydrology. Under normal circumstances, positive indicators of each of these three parameters must be present for an area to satisfy the criteria for jurisdictional wetlands. Soils, vegetation, and hydrologic indicators were inspected throughout the site and were documented at six data plots (Appendix B: Wetland Delineation Data Forms). Representative site photographs are included in Appendix C: Ground Level Color Photographs). Methods and information specific to the site are provided below.

A preliminary site reconnaissance revealed that the thick shrub-level vegetation included a significant component of poison oak to the extent that foot travel through the site was not possible beyond existing trails. Therefore, field investigation was based on preliminary resource review to a greater extent than usual. In addition to wetland inventory and soils inventory, detailed two-foot contour LIDAR topography was reviewed. Areas mapped as inventoried wetland, as hydric soils, and/or simply low-lying areas based on LIDAR topography (Figure 6) were considered potential wetlands requiring field inspection. Due to the extremely rocky geology and shallow soils on the site, it appeared unlikely for wetlands to form elsewhere on the site, and due to the thick coverage of poison oak it was not possible to inspect areas unlikely to contain wetlands. The potential wetland areas were accessed on existing trails where possible. Locations with no access route were investigated by clearing new trails using equipment.

### 5.2.1 Hydrology

All data was collected during the growing season, which ends November 24, and no problematic conditions prevail. The entire project study area was examined for indicators of hydrology as established by the Corps 1987 Manual and Supplement.

It was recognized that precipitation was high for the water year prior to survey, and that saturation may have occurred in upland areas.

### 5.2.2 Soils

Soils were inspected throughout the site and documented in each data plot (Appendix B). Soil pits were dug to a depth of 20 inches, when not hindered by the presence of rock or hardpan. Soil was analyzed for color using the Munsell Soil Color Charts (Munsell Color 2009).

### 5.2.3 Vegetation

Vegetation was inspected and identified throughout the site, and was documented in each data plot (Appendix B) in order to define wetland boundaries and document homogenous vegetation communities. In accordance with the USACE 1987 Manual (Environmental Laboratory 1987), vegetation plots were established in areas supporting a single plant community. Plant species observed were identified using Flora of the Pacific Northwest (Hitchcock and Cronquist 1973) and assigned their indicator status using the Western Mountains, Valleys, and Coast indicators from the 2016 National Wetland Plant List (Lichvar et al. 2016).

Percent cover of each plant species was visually estimated. Plots were sized at 5 -foot radius for herbaceous layer and 30 -foot radius for shrubs, saplings, vines, and trees. Plot sizes and shapes, however, were altered to assure that they represented only a single plant community as identified in descriptions below. Overhanging tree and shrub canopies were not documented if the trees were rooted in a different community.

### 5.2.4 Atypical Situations

The 1987 Manual and Regional Supplement define wetlands in which wetland indicators for one or more of the three parameters are absent due to recent human activities as "atypical situations". Although vegetation had been cleared in the site for access, all parameters were intact and the site had not undergone recent significant disturbance near the wetland boundaries. Therefore, the procedures for atypical situations as described in the Supplement Chapter 5 were not needed.

## 6 WETLANDS AND NON-WETLAND WATERS

Seven wetlands and one potential wetland were delineated in the study area (Wetlands A, B, C, D, E, F, G, and PW-H) and are described below. Their locations are shown on Figure 6 in Appendix A. One inaccessible area was mapped as potential wetland (i.e. PW-H). PW-H was not accessed at the time of survey due to a lack of a cleared access route through the extremely dense poison oak and the uncertainty of future project-related disturbance. No other waters (e.g. creeks) occur within the study area.

All wetlands (A through G ) would be classified as depressional wetlands according to the HGM classification system. The wetlands receive hydrology from precipitation runoff from surrounding slopes and groundwater discharge. Based on the closed depression topography of PW-H, it is assumed that it would also be classified as depressional. Wetland C continues offsite to the north and west. Wetland C is likely jurisdictional to both DSL and USACE because of its connectivity to other waters off-site to the north. All other wetlands appear to be isolated and are thus under the jurisdiction of DSL but likely not the USACE.

### 6.1 WETLAND A

Wetland A ( 0.1 acre ) was delineated in the north central portion of the study area (Appendix A: Figure 6). The wetland is isolated in a depression, roughly half of which is surrounded by rocky slopes and rock overhangs. Wetland A would be classified as a palustrine scrub-shrub wetland with some forested canopy based on the Cowardin classification system (Cowardin et al. 1979). Wetland A is dominated by hardhack (Spiraea douglassii), Oregon ash (Fraxinus latifolia), Pacific crabapple (Malus fusca), and slough sedge (Carex obmupta). Also present in the wetland plant community were swamp rose (Rosa pisocarpa) and Pacific willow (Salix lasiandra). Soils were determined to be hydric based on the histic epipedon (A2) indicator (USACE 2010) as represented by Plot 1. The wetland boundary was determined by the steep slopes leading down to the wetland and the change from hardhack to Douglas-fir, salal (Gaultheria shallon), and dull Oregon-grape (Mahonia nervosa) in the adjacent upland community (Plot 2), as well as hydrologic indicators (the extent of saturation and high water table, which were lacking in Plot 2).

### 6.2 WETLAND B

Wetland B ( 0.1 acre) was delineated northwest of Wetland A in the northwest portion of the study area. The wetland is isolated in a depression, consists largely of unvegetated ponded water over two feet deep, and is surrounded by rocky outcroppings, steep rock faces, and boulders. Wetland B would be classified as a palustrine unconsolidated bottom wetland (Cowardin et al 1979). Plot 3 is representative of the vegetated portion of Wetland B and was dominated by reed canarygrass (Phalaris arundinacea), colonial bentgrass (Agrostis capillaris), and swamp rose. Wetland vegetation generally occurred in patches of soil within the inundated perimeter. The vegetation outside of the inundated perimeter is non-hydrophytic and is dominated by Oregon white oak, Douglas-fir, serviceberry poison oak, licorice fern (Polypodium glycyrrhiza), and dovefoot geranium (Geranium molle). The wetland boundary was determined by the steep rock slopes and the change from reed canarygrass to serviceberry, Douglas-fir, and licorice fern in the adjacent upland community (Plot 4), as well as hydrologic indicators (the extent of saturation and high water table, which were lacking in Plot 4). Approximately half of the wetland boundary is defined by nearly vertical rock slopes.

### 6.3 WETLAND C

Wetland C ( 0.3 acre) lies in the far northwest corner of the study area and continues off-site to the north and east. It appears to flow northeasterly (off-site) and constitutes "Oregon ash swale" habitat in some stretches. Wetland C would be classified as a palustrine forested and scrub-shrub wetland (Cowardin et al 1979). Plots 6 and 7 represent Wetland C. Plot 6 was dominated by Oregon ash. Also present in this wetland plant community were swamp rose, hardhack, and slough sedge. Plot 6 met the hydric soils criteria by the histic epipedon indicator, and met the hydrology indicator by the presence of surface water. Bitter cherry (Prumus emarginata) and salal were dominant in the adjacent upland community described by Plot 5 .

Plot 7 was representative of the scrub-shrub portions of Wetland C and was dominated by hardhack, swamp rose, and Oregon ash. Plot 7 met the loamy mucky mineral (F1) hydric soil indicator and was underlain by a rock layer. Standing water and saturation were present throughout.

The wetland boundary was determined by the topographic break, a sharp difference between upland and hydrophytic vegetation communities, and the presence of hydrology indicators.

### 6.4 WETLAND D

Wetland $\mathrm{D}(0.2$ acre) occurs near the center of the study area and is a closed depression with standing water. Wetland $D$ is a palustrine scrub-shrub wetland but also has a large palustrine unconsolidated bottom component of open water (Cowardin et al. 1979).

The scrub-shrub component is represented by Plot 9 , which is dominated by hardhack, Pacific crabapple, and swamp rose. Wetland D unconsolidated bottom is represented by Plot 11, which was primarily sixinch deep water with no vegetation except Oregon Ash canopy. Both plots showed the histic epipedon hydric soil indicator and had standing water within the plot. Plots 10 and 12 represent the transition to an upland Oregon white oak and Douglas-fir community upslope from Wetland D. The wetland boundary was determined by a transition in plant community, a topographic change, and a lack of wetland hydrology upslope.

### 6.5 WETLAND E

Wetland $\mathrm{E}(0.1$ acre $)$ is located near the center of the study area and is an inundated closed depression. Wetland E would be classified as a palustrine scrub-shrub wetland (Cowardin et al. 1979).

The plot was dominated by hardhack with smaller amounts of Nootka rose (Rosa nutkana), Pacific crabapple, and Oregon white oak present. Plot 13 is representative of the wetland, which was primarily six-inch deep water with dense hardhack. The histic epipedon (A2) hydric soil indicator was present. Plot 14 represents the adjacent upland with no hydrologic indicators and a Douglas-fir and Oregon white oak community. The wetland boundary was determined by a transition in plant community, a topographic change, and a lack of wetland hydrology upslope.

### 6.6 WETLAND F

Wetland F ( 0.2 acre) occupies a closed depression in the northeast portion of the study area. It would be classified primarily as palustrine forested wetland, with some areas of palustrine scrub-shrub and emergent (Cowardin et al. 1979). Wetland F is represented by plots 15 and 17. Plot 15 is dominated by Oregon ash. Plot 17 represents an area of past disturbance by tree and shrub clearing, and is dominated by colonial bentgrass, toad rush (Juncus bufonius), and Oregon ash saplings. Both plots meet the depleted matrix (F3) hydric soil indicator, while Plot 15 also met the loamy mucky mineral (F1) hydric soil indicator. Both had hydrology indicators of a high water table (A2) and saturation (A3) to the surface. The wetland boundary was determined by a transition in plant community, a topographic change, and a lack of wetland hydrology upslope.

### 6.7 WETLAND G

Wetland G (4.7 acres) is a large depression with three different palustrine plant communities occurring in roughly concentric circles: forested along the wetland/upland boundary, scrub-shrub transition, and persistent emergent and scrub-shrub vegetation in the central inundated areas. Some portions of the center of Wetland G would be considered muddy unconsolidated bottom. The northern half of the wetland, especially, had steep rocky terrain with slopes up to $100 \%$ from between 5 and 30 feet from the wetland boundary. The remainder of the wetland had more gentle slopes with wider wetland transition areas.

Plots 19, 23, and 25 are representative of the scrub-shrub and forested communities and are dominated by Pacific willow, hardhack, swamp rose, and Pacific crabapple. Plots 21 and 24 are nearer the center of the wetland and are dominated by hardhack, Pacific willow, tall mannagrass (Glyceria striata), and cattail (Typha latifolia). Plots 20,22, and 26 represent the adjacent upland communities and were dominated by Oregon white oak, beaked hazelnut (Corylus cornuta), Pacific madrone, Douglas-fir, serviceberry, cascara (Rhamnus purshiana), and sword fern (Polystichum munitum). Plots 19, 21, 23, and 24 showed the histic epipedon (A2) hydric soil indicator, while Plot 25 showed the loamy mucky material (F1) hydric soil indicator. The wetland boundary was determined by a transition in plant community, a topographic change, and a lack of wetland hydrology upslope.

### 6.8 POTENTIAL WETLAND H

Review of LIDAR 2-foot contour topography showed this location to be a closed depression similar to those that were found to contain Wetlands A, B, D, E, and F. However, this location was not inspected due to particularly thick growth of poison oak surrounding it, and because the steep slopes on the north, east, and west sides made it difficult to clear trail access using equipment. As shown on Figure 6, it is assumed for now that wetland conditions prevail throughout the entire closed depression. It may be possible to access Area H in the future by clearing a trail through the powerline corridor with permission of that utility.

## 7 DEVIATION FROM LWI OR NWI

The LWI for Sherwood, OR, did not extend to the study area. (David Evans and Associates 1992). The NWI shows a seasonally flooded persistent palustrine emergent wetland (PEM1C) in the approximate location of Wetland G (USFWS 2016). No waters are described on the NWI.

## 8 MAPPING METHOD

All features were collected by DEA biologists using a handheld Trimble GPS unit with typical horizontal accuracy of three feet or better..

## 9 ADDITIONAL INFORMATION

No additional information.

## 10 RESULTS AND CONCLUSIONS

Seven wetlands and one potential wetland were delineated within the study area, and are summarized in Table 5. All wetlands had some inundation, but no waters were delineated within the study area. Although rainfall was exceptionally high for the month of October, there were no indications of excess precipitation influencing the wetland boundary determination. Of the seven wetlands, only Wetland C continues offsite.

Table 5: Summary of Wetlands within the Study Area

| ID | Size <br> (acres) | Cowardin Class | HGM Class | Preliminary <br> Jurisdictional <br> Determination |
| :---: | :---: | :--- | :--- | :--- |
| Wetland A | 0.1 | Palustrine scrub-shrub/ Palustrine <br> unconsolidated bottom | Depression | DSL |
| Wetland B | 0.1 | Palustrine scrub-shrub/ Palustrine <br> emergent | Depression | DSL |
| Wetland C | 0.3 | Palustrine forested/ Palustrine scrub-shrub | Depression | DSL and |
| Wetland D | 0.2 | Palustrine scrub-shrub/ Palustrine <br> unconsolidated bottom | Depression | DSL |
| Wetland E | 0.1 | Palustrine scrub-shrub | Depression | DSL |
| Wetland F | 0.2 | Palustrine forested/ Palustrine scrub-shrub/ <br> Palustrine emergent | Depression | DSL |
| Wetland G | 4.7 | Palustrine forested/ Palustrine scrub-shrub/ <br> Palustrine emergent | Depression | DSL |
| Potential <br> Wetland H | 0.3 | Unknown- appears to be Palustrine scrub- <br> shrub | Depression | DSL |
| Total | 6.0 |  |  |  |

## 11 DISCLAIMER

This report documents the investigation, best professional judgment, and conclusions of the investigator. It is correct and complete to the best of my knowledge. It should be considered a Preliminary Jurisdictional Determination of wetlands and other waters and used at your own risk until it has been reviewed and approved in writing by the Oregon Department of State Lands in Accordance with OAR 141-090-0005 through OAR 141-090-0555.

## 12 PREPARERS AND CONTRIBUTORS

John Macklin, DEA Biologist, and Tony Vingiello, DEA Biologist, performed the site delineation. Mr. Vingiello is the primary author of the report, and Mr. Macklin provided quality assurance review. Dawn Afman, DEA Project Assistant, provided editing assistance. Sara Gilbert, DEA Geographic Information System Specialist, prepared the report graphics.

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## 14 APPENDICES

## APPENDIX A: FIGURES



Figure 1 Vicinity Map




Willamette Water Supply Program
Water Treatment Plant (WWSP WTP)
Wetland Delineation
Figure 4
Soil Survey
Study Area
Soil Unit
Hydric Soil

NRCS Soil Units within the Study Area
37C Quatama loam, 7 to 12 percent slopes
38 C Saum silt loam, 7 to 12 percent slopes 38E Saum silt loam, 20 to 30 percent slopes 47D Xerochrepts-Rock outcrop complex

ESRI, ArcGIS Online, Word Imagery. Microsoft 2010. Portand, Oregon. Natural
Resources Consenation Senvice (NRCS). 2014. Soil Survey Geographic (SSURGO Resources Conservation Senvice (NRCS). 2014. Soill Survey Geograph
database for Clackamas County Area \& Washington County Oregon.



Figure 5
Aerial Photo

## Legend

$\underbrace{\text { Legend }}_{\text {Study Area }}$


Willamette Water Supply Program Water Treatment Plant (WWSP WTP)

Figure 6
Delineated Wetlands

| Delineated Features |  | Study Area |
| :---: | :---: | :---: |
| 3 Wetland | - | Photo location and direction |
| Wetland Area | * | Wetland extends beyond study area |
|  | $\bigcirc$ | Upland Data Plot |
|  | $\triangle$ | Wetland Data Plot |
|  |  | Power ROW |
|  |  | Taxlot |
|  |  | Contour (20 ft intervals) |
|  |  | Contour (2 ft intervals) |

On-site features (wetlands, ditches, streams, culverts, and data plots) were mapped with a Trimble Pathfinder GEO XH receiver with typical accuracy of 3 feet or better. Off-site boundaries ar approximate and were mapped based on field review from aderent pubis included where jurisdictional features, with the
asterisk was exclusion of upland ditches, extend off site. Only taxlots which ntersect the study area are labeled. Imagery: USDA NAIP 2016.

ale: 1266 ROIT Time: $6: 38: 30$ PM User Name: eiro

APPENDIX B: DATA FORMS

| Plot <br> ID | Latitude | Longitude | PLSS | Soil ID | Soil Type | Wetland ID | Wetland Type | City |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 45.365377 | -122.808450 | T2S R1W S28 | 47D | Xerochrepts-Rock outcrop complex | $\rightarrow$ | - | Unicorp. WA. Cly |
| 2 | 45.365365 | -122.808423 | T2S R1W S28 | 47D | Xerochrepts-Rock outcrop complex | -- | - | Unicorp. WA. Cly |
| 3 | 45.365629 | -122.809030 | T2S R1W S28 | 47D | Xerochrepts-Rock outcrop complex | - | - | Unicorp. WA. Cly |
| 4 | 45,365562 | -122.809049 | T2S R1W S28 | 47D | Xerochrepts-Rock outcrop complex | -- | -- | Unicorp. WA. Cty |
| 9 | 45.364263 | -122.807795 | T2S R1W S28 | 47D | Xerochrepts-Rock outcrop complex | -- | - | Unicorp. WA. Cty |
| 10 | 45.364325 | -122.807739 | T2S R1W S28 | 47D | Xerochrepts-Rock outcrop complex | - | - | Unicorp. WA. Cty |
| 11 | 45.363939 | -122.807763 | T2S R1W S28 | 47D | Xerochrepts-Rock outcrop complex | - | - | Unicorp. WA. Cty |
| 12 | 45.363875 | -122.807879 | T2S R1W S28 | 47D | Xerochrepts-Rock outcrop complex | - | - | Unicorp. WA. Cty |
| 13 | 45.364971 | -122.808576 | T2S R1W S28 | 47D | Xerochrepts-Rock outcrop complex | $\cdots$ | - | Unicorp. WA. Cty |
| 14 | 45.365004 | -122.808584 | T2S R1W S28 | 47D | Xerochrepts-Rock outcrop complex | - | - | Unicorp. WA. Cty |
| 15 | 45.365218 | -122.806620 | T2S R1W S28 | 47D | Xerochrepts-Rock outcrop complex | - | - | Unicorp. WA. Cly |
| 16 | 45.365165 | -122.806662 | T2S R1W S28 | 47D | Xerochrepts-Rock outcrop complex | - | - | Unicorp. WVA. Cty |
| 17 | 45.365362 | -122.806388 | T2S R1W S28 | 38 C | Saum silt loam, 7 to 12 percent slopes | -- | - | Unicorp. WA. Cty |
| 18 | 45.365432 | -122.806264 | T2S R1W S28 | 38 C | Saum silt loam, 7 to 12 percent slopes | - | - | Unicorp. WA. Cty |
| 19 | 45.363206 | -122.806596 | T2S R1W S28 | 47D | Xerochrepts-Rock outcrop complex | PEM1C | Freshwater Emergent Wetland | Unicorp. WA. Cty |
| 20 | 45.363224 | -122.806469 | T2S R1W S28 | 38E | Saum silt loam, 20 to 30 percent slopes | PEM1C | Freshwater Emergent Wetland | Unicorp. WA. Cly |
| 21 | 45.363221 | -122.806748 | T2S R1W S28 | 47D | Xerochrepts-Rock outcrop complex | PEM1C | Freshwater Emergent Wetland | Unicorp. WA. Cly |
| 22 | 45.363202 | -122.808071 | T2S R1W S28 | 47D | Xerochrepts-Rock outcrop complex | - | - | Unicorp. WA. Cly |
| 23 | 45.363110 | -122.808041 | T2S R1W S28 | 47D | Xerochrepts-Rock outcrop complex | - | -- | Unicorp. WA. Cty |
| 24 | 45.363071 | -122.807986 | T2S R1W S28 | 47D | Xerochrepts-Rock outcrop complex | PEM1C | Freshwater Emergent Wetland | Unicorp. WA. Cly |
| 25 | 45.364454 | -122.806724 | T2S R1W S28 | 47D | Xerochrepts-Rock outcrop complex | - | -- | Unicorp. WA. Cly |
| 26 | 45.364484 | -122.806784 | T2S R1W S28 | 47D | Xerochrepts-Rock outcrop complex | - | - | Unicorp. WA. Cty |



## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? Hydric Soil Present? <br> Wetland Hydrology Present? | Yes $\qquad$ No $\qquad$ <br> Yes $\qquad$ No $\qquad$ <br> Yes $\qquad$ No $\qquad$ | Is the Sampled Area within a Wetland? | Yes $\checkmark$ | No |
| :---: | :---: | :---: | :---: | :---: |
| Remarks: <br> Wetland plot in Wetland A at to | lope. Precipitation is high | ater year. |  |  |

## VEGETATION - Use scientific names of plants.



Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)
Depth
(inches)

Remarks:
Some fibrous peaty nodules present (1"). Large rocks occasionally present in matrix, but less restrictive than in upland plots.

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? Hydric Soil Present? <br> Wetland Hydrology Present? | Yes <br> Yes <br> Yes | No $\qquad$ No $\qquad$ $\square$ | Is the Sampled Area within a Wetland? |  | No $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Remarks: Plot 2 is 2 ft . higher than Plot 1 and on face of slope. Precipitation is high for water year. |  |  |  |  |  |

VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)
Mepth
Linches)
Color (moist)

Remarks:

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.



VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)

| Depth <br> (inches) | Color (moist) |
| :--- | :--- | :--- |

Remarks:
Underlain by rock, not mineral soil; soils similar to Plot 1 in color and texture

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.



VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)
Matrix
(inches)

Remarks:
no significant moisture between solid rock layer and surface.

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.



VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? Hydric Soil Present? <br> Wetland Hydrology Present? | Yes $\qquad$ No $\qquad$ <br> Yes $\qquad$ No $\qquad$ <br> Yes $\qquad$ No $\qquad$ | Is the Sampled Area within a Wetland? | Yes $\checkmark$ | No |
| :---: | :---: | :---: | :---: | :---: |
| Remarks: Precipitation is high for water | lots 6 and 7 are represe | of Wetland C . |  |  |

## VEGETATION - Use scientific names of plants.

| Tree Stratum (Plot size: 30 ft | Absolute \% Cover | Dominant Species? | Indicator Status | Dominance Test worksheet: Number of Dominant Species |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1. Fraxinus latifolia | 80 | Y | FACW | That Are OBL, FACW, or FAC: | 4 | (A) |
| 2. |  |  |  | Total Number of Dominant |  |  |
| 3. |  |  |  | Species Across All Strata: |  | (B) |
|  | 80 | $=$ Total Cov |  | Percent of Dominant Species <br> That Are OBL FACW or FAC: | 100 |  |
| Sapling/Shrub Stratum (Plot size: 30 ft ) $\quad$ - Total Cover |  |  |  |  |  |  |
| 1. Rosa pisocarpa | 10 | $Y$ | FAC | Prevalence Index worksheet: |  |  |
| 2. Spiraea douglasii | 5 | Y | FACW | Total \% Cover of: Multiply by: |  |  |
|  |  |  |  | OBL species $\qquad$ $\times 1=$ $\qquad$ <br> FACW species $\qquad$ $\times 2=$ $\qquad$ |  |  |
|  |  |  |  |  |  |  |
|  |  |  |  | FAC species $\qquad$ | $3=$ |  |
|  | 15 | $=$ Total Cov | er |  | 4 |  |
| Herb Stratum (Plot size: 5 ft |  |  |  | FACU species <br> UPL species $\qquad$ | $5=$ |  |
| 1. Carex obnupta | 15 | Y | OBL | UPL species <br> Column Totals: $\qquad$ |  |  |
|  |  |  |  |  |  |  |
| 3. |  |  |  | Prevalence Index $=\mathrm{B} / \mathrm{A}$ |  |  |
|  |  |  |  | Hydrophytic Vegetation Indicators:Dominance Test is $>50 \%$$\qquad$ Prevalence Index is $\leq 3.0^{1}$$\qquad$ Morphological Adaptations ${ }^{1}$ (Provide supporting data in Remarks or on a separate sheet)$\qquad$ Wetland Non-Vascular Plants ${ }^{1}$$\qquad$ Problematic Hydrophytic Vegetation ${ }^{1}$ (Explain) ${ }^{1}$ Indicators of hydric soil and wetland hydrology must be present, unless disturbed or problematic. |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
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|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| 11. |  |  |  |  |  |  |  |  |
| Woody Vine Stratum (Plot size: 30 ft ) $\quad$ = Total Cover |  |  |  | Hydrophytic <br> Vegetation <br> Present? |  |  |
| 1. Rubus ursinus. | Trace | N | FACU |  |  |  |
| 2. |  |  |  |  |  |  |
| \% Bare Ground in Herb Stratum 85 | $\underline{0}$ = Total Cover |  |  |  |  |  |
| Remarks: Bare ground is leaf litter and inundated substrate. |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |

Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)
Depth
(inches)

Remarks:

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? Hydric Soil Present? <br> Wetland Hydrology Present? | Yes $\qquad$ No $\qquad$ <br> Yes $\qquad$ No $\qquad$ <br> Yes $\qquad$ No $\qquad$ | Is the Sampled Area <br> within a Wetland? <br> Yes $\qquad$ | No |
| :---: | :---: | :---: | :---: |
| Remarks: Precipitation is high for water | lots 6 and 7 are represe | of Wetland C which continues off-site. |  |

VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)
Depth
(inches)

Remarks:
Underlain by rock, not mineral soil; soil similar to other histic plots.

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.



VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? Hydric Soil Present? <br> Wetland Hydrology Present? | Yes $\qquad$ No $\qquad$ <br> Yes $\qquad$ No $\qquad$ <br> Yes $\qquad$ No $\qquad$ | Is the Sampled Area within a Wetland? | Yes $\checkmark$ | No |
| :---: | :---: | :---: | :---: | :---: |
| Remarks: <br> Precipitation is high for water | did not alter indicators. |  |  |  |

VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)

| Depth <br> (inches) | Color (moist) |
| :--- | :--- | :--- |
| O-2 |  |

Remarks:
defined redox mineral layer below histic muck

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? Hydric Soil Present? <br> Wetland Hydrology Present? | Yes <br> Yes <br> Yes |  | Is the Sampled Area within a Wetland? |  | No |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Remarks: |  |  |  |  |  |
| Precipitation is high for water year. |  |  |  |  |  |

VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)
Depth
(inches)

Remarks:

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? Hydric Soil Present? <br> Wetland Hydrology Present? | Yes $\qquad$ No $\qquad$ <br> Yes $\qquad$ No $\qquad$ <br> Yes $\qquad$ No $\qquad$ | Is the Sampled Area within a Wetland? | Yes $\checkmark$ | No |
| :---: | :---: | :---: | :---: | :---: |
| Remarks: <br> Precipitation is high for water | did not alter indicators. |  |  |  |

VEGETATION - Use scientific names of plants.

| Tree Stratum (Plot size: 30 ft | Absolute \% Cover | Dominant Species? | Indicator Status | Dominance Test worksheet: <br> Number of Dominant Species |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1. Fraxinus latifolia | 5 | Y | FACW | That Are OBL, FACW, or FAC: | 1 | (A) |
| 2. |  |  |  | Total Number of Dominant |  |  |
| 3. |  |  |  | Species Across All Strata: |  | (B) |
|  | 5 | $=$ Total Cov |  | Percent of Dominant Species <br> That Are OBL, FACW, or FAC: | 100 | (A/B) |
| Sapling/Shrub Stratum (Plot size: 30 ft |  |  |  |  |  |  |
|  |  |  |  | Prevalence Index worksheet: |  |  |
|  |  |  |  |  |  |  |
|  |  |  |  | OBL species $\qquad$ $\times 1=$ $\qquad$ |  |  |
|  |  |  |  | FACW species $\qquad$ <br> FAC species $\qquad$ | $2=$ |  |
|  |  |  |  |  |  |  |
|  |  | = Total Cov |  | FACU species | $4=$ |  |
| Herb Stratum (Plot size: $\qquad$ <br> 1. Geranium molle | Trace |  | UPL | UPL species |  |  |
| 1. Geranium molle | Trace |  |  | Column Totals: | A) |  |
|  |  |  |  |  |  |  |
| 3. |  |  |  |  | Prevalence Index $=\mathrm{B} / \mathrm{A}=$ |  |
|  |  |  |  | Hydrophytic Vegetation Indicators: <br> $\checkmark$ Dominance Test is $>50 \%$ |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  | _. Prevalence Index is $\leq 3.0^{1}$ |  |  |
| 7. |  |  |  | Morphological Adaptations ${ }^{1}$ (Provide supporting data in Remarks or on a separate sheet) |  |  |
|  |  |  |  | Wetland Non-Vascular Plants ${ }^{1}$ |  |  |
|  |  |  |  |  |  |  |  |  |
| 10. |  |  |  | 'Indicators of hydric soil and wetland hydrology must |  |  |
|  |  |  |  | be present, unless disturbed or problematic. |  |  |
| Woody Vine Stratum (Plot size: 30 ft |  |  |  |  |  |  |
|  |  |  |  | Hydrophytic <br> Vegetation <br> Present? |  |  |
|  |  |  |  |  |  |  |  |
| \% Bare Ground in Herb Stratum 99 | 0 | $=$ Total Cover |  |  |  |  |  |
| Remarks: |  |  |  |  |  |  |
| Bare ground cover is inundated substrate |  |  |  |  |  |  |

Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.



VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)
Mepth
(inches)

Remarks:

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.



VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)

| Depth <br> (inches) | Color (moist) |
| :--- | :--- | :--- |
| O-8 |  |

Remarks:

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.



VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)
Mepth
(inches)
Color (moist)

Remarks:

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.



## VEGETATION - Use scientific names of plants.



Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)
Depth
(inches)

Remarks:

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.



VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)
Mepth
(inches)

Remarks:

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.



VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)
Depth
(inches)

Remarks:

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? Hydric Soil Present? <br> Wetland Hydrology Present? | Yes <br> Yes <br> Yes |  | Is the Sampled Area within a Wetland? |  | No |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Remarks: |  |  |  |  |  |
| Precipitation is high for water year |  |  |  |  |  |

VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)
Mepth
(inches)

Remarks:

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.



VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)
Depth
(inches)

Remarks:

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? <br> Hydric Soil Present? <br> Wetland Hydrology Present? | Yes <br> Yes <br> Yes |  | Is the Sampled Area within a Wetland? |  | No $\qquad$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Remarks <br> Precipitation is high for water year |  |  |  |  |  |
|  |  |  |  |  |  |

VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)

| Depth <br> (inches) | Color (moist) |
| :--- | :--- | :--- |
| O-1 |  |

Remarks:

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.



## VEGETATION - Use scientific names of plants.



Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)


Remarks:

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? Hydric Soil Present? Wetland Hydrology Present? |  | No $\qquad$ <br> No $\qquad$ No $\qquad$ | Is the Sampled Area within a Wetland? |  | $\checkmark$ $\qquad$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Remarks: Precipitation is high for water year. Southwest side of Wetland G; immediately north of powerlines. |  |  |  |  |  |

VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)
Mepth
(inches)

Remarks:

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.

| Hydrophytic Vegetation Present? Hydric Soil Present? <br> Wetland Hydrology Present? | Yes $\qquad$ No $\qquad$ <br> Yes $\qquad$ No $\qquad$ <br> Yes $\qquad$ No $\qquad$ | Is the Sampled Area within a Wetland? | $\text { Yes } \quad \text { No }$ |
| :---: | :---: | :---: | :---: |
| Remarks: <br> Precipitation is high for water year. Plot at southwest part of Wetland $G$ near boundary. Paired with upland plot 22. |  |  |  |

## VEGETATION - Use scientific names of plants.



Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)
Depth
(inches)

Remarks:

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.



## VEGETATION - Use scientific names of plants.



Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)
Depth
(inches)

Remarks:

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.



VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)

| Depth <br> (inches) | Color (moist) |
| :--- | :--- | :--- |
| O-14 |  |

Remarks:

## HYDROLOGY




## SUMMARY OF FINDINGS - Attach site map showing sampling point locations, transects, important features, etc.



VEGETATION - Use scientific names of plants.


Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)
Depth
(inches)

Remarks:

## HYDROLOGY



## APPENDIX C: SITE PHOTOGRAPHS



Photo 1: Facing east at the inundated portion of Wetland A (November 3, 2016).


Photo 2: Facing north at Wetland B (November 3, 2016).


Photo 3: Facing west at Wetland B (November 3, 2016).


Photo 2: Facing north at the west side of the Wetland $C$ boundary marked by orange flagging. (November 3, 2016).


Photo 5: Facing west towards Wetland C (November 3, 2016


Photo 6: Facing southwest along the Wetland C boundary. (November 3, 2016).


Photo 7: Facing south toward the inundated portion of Wetland D (November 3, 2016).


Photo 8: Facing northeast at Wetland D (November 3, 2016).


Photo 9: Facing east at the western end of Wetland $E$ (November 3, 2016).


Photo 10: Facing northwest toward the northern boundary of Wetland F (November 3, 2016).


Photo 11: Facing southwest at the southern end of Wetland F (November 3, 2016).


Photo 12: Facing southwest toward the northern boundary of Wetland F (November 3, 2016).


Photo 13: Facing west at the eastern boundary of Wetland G (November 4, 2016).


Photo 14: Facing southwest toward the center of Wetland G (November 4, 2016).


Photo 15: Facing east from the eastern boundary of Wetland G toward Plot 20 (November 4, 2016).


Photo 16: Facing west from within Wetland G toward the western boundary (November 4, 2016).

## APPENDIX D: WETS TABLE

|  <br> Latitude: . $4527 \cdots$. . Tangitude: . $12249 \ldots .$. . Elevation: . 00270II <br> State FIFS County(FIFS) : . $41067 \cdots$ County Name: Weshingtanf <br> Start yr. - 1971.. End yr. - 2000 Jl |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
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|  | daily | deily | ... | ... | n. | then . . | $\cdots \mathrm{cr}$ | fall |
|  | 461. | . 33.8 | 40.0 | 5.83 | 3.53 | 7.07 |  | 0.6 |
|  | 50.7 | - 35.3 | - 43.0 | 4.84 | 3.06 | -5.84 | 12. | 0.7 91 |
| Februacy Merch | 56.1 | - 37.3 | - 46.7 | - 4.06 | - 3. 03 | $\cdots 4.74$ | 11. | -0.1. |
| Mench Apmil | 61.1 | - 40.2 | - 50.7 | - 2.79 | -1.90. | - 3.32 | 9. | -0.0 |
| April <br> May | 672 | - 45.4 | - 56.3 | - 2.25 | $\cdots 1.40$ | - 2.72 | $\cdots$ | $\cdots 0.0$ |
| May | 72.7 | -50.5 | -61.6 | $\cdots 1.62$ | $\cdots 1.02$ | $\cdots 1.95$ | - 5 | $\cdots 0$. |
| July | 792 | - 543 | -66.8 | $\cdots 068$ | $\cdots 0,27$ | $\cdots 0.84$. | - 2 | $\cdots 0.0$ 9 |
| August | 79.9 | -54.3 | - 67.1 | -0.84 | $\cdots 0.22$ | -0.98 | . 2 | $\cdots$ |
| Septenber. | 74.8 | - 50.3 | - 62.6 | $\cdots 1.64$ | $\cdots 0.70$ | - 2.03 | 5 | 0.0 |
| Optcher | 63.8 | $\bigcirc 43.4$ | - 53.6 | $\cdots 2.92$ | $\cdots 1.52$ | - 3.57 | - 8 | $\cdots$ |
| Noveniner | 52.0 | - 38.5 | - 45.3 | $\cdots 6.07$. | $\cdots 4.08$ | $\cdots 7.25$ | 13 | 0.5 |
| Deceniber. | 46.0 | - 34.5 | - 40.3 | . 6.41 | 4.42. | - 7.64 | 12 | 0.5 |
| $\cdots$ Anmal. |  |  |  |  |  |  |  |  |
| - Aterage. | . 62.5 | 2 |  |  |  |  |  |  |
| $\cdots$ Average |  |  |  | 39.95 |  |  |  | 2.2.91 |
| GRONIN GESON [ATEST |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| ..... Probebi lity . . |  |  |  |  |  |  |  |  |
| . . . . . . . . . . . . . . . . |  | Eeginning and Ending Dates <br> Growing Seesan Length |  |  |  |  |  |  |
| . . . . . . . . . . . . . . . . . . |  | If |  |  |  |  |  |  |
| ...... 50 percent *.. |  |  |  |  |  |  |  |  |
|  |  |  |  |  | … . . . . . . . . . . 12 |  |  |  |
| …... 70 percent *. |  | 1/20 to 12/30 343 days |  |  | $\begin{aligned} & 2 / 20 \cdot \text { to } 12 / 5 \\ & \cdots \cdot 287 \cdot \text { deys } \end{aligned}$ |  | $\cdots 222$ | $\begin{aligned} & \text { to } 11 / 1211 \\ & \text { days }- \text { II } \end{aligned}$ |

# Natural Resource Assessment for T-S Corporate Park in Sherwood, Oregon 

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PHS Project Number: 6163
January 9, 2020
[Revised March 19, 2020]


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### 1.0 INTRODUCTION

Pacific Habitat Services, Inc. (PHS) conducted a Natural Resources Assessment (NRA) on an existing, undeveloped light industrial property on SW Tualatin-Sherwood Road in Sherwood, Washington County, Oregon (Township 2 South, Range 1 West, Section 28D, tax lot 1100). The property, proposed for T-S Corporate Park, is a former farm site, with fallow fields along TualatinSherwood Road and forest lands proven to be too rocky to cultivate in the south.

The parcel south of the site was recently delineated by David Evans and Associates (DEA) in conjunction with the Willamette Water Supply Program pipeline and water treatment facility. The DEA delineation was approved by DSL in 2017 (DSL DET\#2017-0008). Copies of both PHS' and DEA's delineation reports are being provided to CWS as part of the Natural Resources Assessment (NRA) submittal.

This report presents the definitions and the methodology used to assess the natural resources within the project site as required by CWS. Figure 1 shows the project location; Figures 2-2D include existing conditions, including slopes and the corresponding limits of vegetated corridor (VC); Figures 3-3D includes a detail of the VC plant communities, data and photo point locations; and Figures 4-4C includes the location of proposed development actions and activities, as well as project related encroachments, mitigation and enhancement areas. All figures are located in Appendix A.

### 2.0 EXISTING CONDITIONS

The study area is bordered on the north by SW Tualatin-Sherwood Road; to the west by a partially developed site including a municipal water storage facility; on the east by SW 124th Avenue; and on the south by a partially developed industrial storage site. Nearby land uses include partially landscaped rural residential lots, aggregate mining operations, and small-scale commercial/industrial activities. Until the summer of 2019 , when they were removed, the site included a farmhouse and other structures in the west-central portion of the site.

Vegetation communities upslope of the formerly farmed areas along Tualatin-Sherwood Road have formed in the relatively rocky, hilly terrain of the Tonquin Scablands, an area between Sherwood and Tualatin that was scoured by the enormous Bretz flood events during the Pleistocene era. More recent human disturbance (both logging and farming) has also helped shape the current vegetation cover.

The mostly forested to shrubby upland areas upslope of the agricultural fields are comprised of a relatively young to mature overstory of Douglas fir (Pseudotsuga menziesii), with bigleaf maple (Acer macrophyllum), Oregon white oak (Quercus garryana), and madrone (Arbutus menziesii) also present. The shrub understory is dense and commonly dominated by poison oak (Toxicodendron diversilobum), tall Oregon grape (Mahonia aquifolium), oceanspray (Holodiscus discolor), snowberry (Symphoricarpos albus), and Saskatoon serviceberry (Amelanchier alnifolia). Sword fern (Polystichum munitum) is a common groundcover species. Invasive shrubs such as Himalayan blackberry (Rubus armeniacus) and Scotch broom (Cytisus scoparius) are common in more recently disturbed edge habitats.

### 3.0 DISCUSSION OF WATER QUALITY SENSITIVE AREAS

PHS delineated sensitive areas within the study area based on the presence of wetland hydrology, hydric soils, and hydrophytic vegetation; in accordance with the Routine On-site Determination, as described in the Corps of Engineers Wetland Delineation Manual, Wetlands Research Program Technical Report Y-87-1 ("The 1987 Manual") and the Regional Supplement to the Corps of Engineers Wetland Delineation Manual: Western Mountains, Valleys, and Coast Region, May 2010). The study area was originally delineated on March 2 and March 26, 2017, with additional site data collected on April 28 and June 28, 2017. In order to submit the results of the delineation to the agencies for concurrence, the wetland boundaries were confirmed and/or additional data collected on October 15, November 8 and December 13, 2019.

As the results of the delineation are included in a separate report provided to CWS, only a brief description of each sensitive area is included below.

## Wetland A

Wetland A (2.34 acres) is a broad seasonal swale that extends northward to Tualatin-Sherwood Road. The swale originates from a band of hillside seeps or springs along its southern boundary. Its origin may include percolation through porous soils from the large depression further upslope to the south (Wetland C). Several shallow incised channels extend northward from the seeps through the broad swale, ultimately to form a single larger channel near Tualatin-Sherwood Road. Wetland conditions extend for some distance to either side of the channels, supporting a mature stand of Oregon ash. The seasonally charged surface flows are culverted beneath Tualatin-Sherwood Road, ultimately flowing to Hedges Creek (a Tualatin River tributary).

The swale's Cowardin class ranges from palustrine emergent through scrub-shrub and forested, saturated/semipermanent/seasonal (PEMY/PSSY/PFOY) wetland, while the hydrogeomorphic (HGM) class is Slope, largely due to its moderate to shallow gradient and upslope seepage/groundwater spring origins.

## Wetland B

Wetland B is a small isolated concave wetland ( 0.03 acre) on a gentle slope in the now fallow field west of Wetland A. This location was actively farmed prior to 2017, but appears to have been left fallow since at least Fall 2017. The wetland appears to be fed by seasonally charged upslope groundwater seepage and overland sheet flow. Its Cowardin class is palustrine emergent, saturated/ semipermanent/ seasonal (PEMY) wetland, while the HGM class is Slope-Flats. It is dominated by weakly emergent, mostly non-native species, including a hybrid clover (Trifolium sp.), perennial ryegrass (Lolium perenne), and lesser hawkbit (Leontodon saxatilis).

## Wetland C

Wetland C is a 1.29 acre depressional feature at the south end of the site that extends outside property boundaries (the southern portion of the wetland was delineated by DEA in 2017). The depression is likely an old scour feature from the Bretz flood events, with relatively steep sideslopes on the west and east sides. The north edge is comparatively low in elevation, but topography rises several feet just to the north, sufficient to contain seasonal rainfall accumulations and act as an impoundment. Its Cowardin class is primarily forested, seasonally flooded/saturated (PFOE) wetland, while the HGM class is Depressional Closed Non-Permanent (DCNP).

Vegetation within Wetland C is dominated by a mostly mature Oregon ash stand, with relatively sparse understory in many places due to prolonged seasonal ponding. Scattered Pacific willow (Salix lasiandra) trees are also present within the depression. Shrubs include willows, hardhack spirea (Spiraea douglasii), and clustered rose (Rosa pisocarpa). Emergent cover is sparser at the north end of the wetland, due to increased duration and depth of inundation, in addition to a dense ash overstory. Observed vegetation is limited to small percentages of shiny geranium (Geranium lucidum), annual bluegrass (Poa anпиа) and bedstraw (Galium aparine). The south end includes spreading rush (Juncus patens), taperfruit shortscale sedge (Carex leptopoda), largeleaf avens (Geum macrophyllum), shiny geranium, and slough sedge (Carex obnupta).

### 4.0 VEGETATED CORRIDOR ASSESSMENT

The following assessment includes VCs associated with each of the onsite wetlands.

### 4.1 Vegetated Corridor Width Determination

The slopes adjacent to the sensitive areas were assessed to determine the regulated width of the VC. The location of the VC, adjacent slopes and corridor widths are shown on Figures 2-2D. The regulated VC widths of identified sensitive areas were determined as follows:

Table 1. Summary of VC Widths

| Sensitive Area | VC Width | Justification |
| :---: | :---: | :--- |
| Wetland A | 50 feet | $\bullet>0.5$ acres <br> $\bullet$ Slopes $<25 \%$ |
| Wetland B | 25 feet | $\bullet \leq 0.5$ acres and isolated <br> $\bullet$ Slopes $<25 \%$ |
| Wetland C | 50 feet or <br> greater | $\bullet>0.5$ acres <br> $\bullet$ Slopes variable; $>$ and $<25 \%$ |

Slopes are generally quite gentle across the north end of the site but increase to the south. Slopes adjoining Wetland C are steeper, but generally still less than 25 percent. There is one narrow point where slopes exceed 25 percent over the first 50 feet, but are less than over the next 25 feet. At this location a break in slope has been identified and the full setback of 35 feet from the break has been identified (Figure 2C).

### 4.2 Vegetated Corridor Plant Communities

There are three plant communities within the project area (Figures 3-3D). A brief discussion of each community is below. A table of all species documented at each sample point selected as representative of the community it is located in is provided in Appendix B, along with photo documentation. Assessment data includes wetland delineation upland sample plots as well as additional data collected in areas where delineation sample points were not taken.

Plant Community A ( 85,392 square feet / 1.96 acre) encapsulates the herbaceous dominated areas including the formerly farmed lands adjoining Wetlands A and B (Sample Point VC1). The community itself has no trees but it does have a measureable tree canopy in some areas where it adjoins Communities B and C, as well as forested portions of Wetland A. Himalayan blackberry is common with variable cover and not always present; its percent cover generally decreases with distance from Wetland A. Common herbaceous species include soft velvetgrass (Holcus lanatus), tall fescue (Schedonorus arundinaceus), bentgrass (Agrostis sp.), Queen Anne's lace (Daucus carota), red clover (Trifolium pratense) and tansy ragwort (Senecio jacobaea).

Plant Community B ( 62,366 square feet / 1.43 acre) includes the native forested areas surrounding much of Wetland C. The tree canopy is rather dense with two to four species present at each assessment site. Douglas fir is most common, with big leaf maple, madrone, Oregon white oak, bitter cherry (Prunus emarginata) and Sitka willow (Salix sitchensis) also documented. The understory shrub cover averaged nearly 100 percent and was dominated by natives including beaked hazelnut (Corylus cornuta), trailing blackberry (Rubus ursinus), poison oak, serviceberry, tall Oregon grape and snowberry. Herbaceous cover was sparse, largely because of the extent of tree and shrub cover. Taperfruit shortscale sedge was the single most common native plant, with shiny geranium, a somewhat invasive species, a dominant where it is present.

Plant Community C ( 23,086 square feet / 0.53 acres) is a shrub community bordering portions of Wetland A as well as portions of VC north and east of Wetland C. It is typified by non-native shrubs and small trees, with variable herbaceous cover. Himalayan blackberry is the single most common species, though English hawthorn (Crataegus monogyna), Sitka willow, apple (Malus pumila), sweet cherry (Prunus avium) and hazelnut trees and shrubs are also present throughout. Where this community borders the formerly farmed portions of the property herbaceous species are common, though not in formerly forested areas where blackberry seems to have moved in to dominate since logging activities in 2003 and more recently in 2016.

### 4.3 Vegetated Corridor Plant Community Condition

Table 2 summarizes the condition of the plant communities in accordance with Clean Water Services' standards.

Table 2. Summary of Plant Communities

|  |  | Plant Communities |  |  |
| :--- | :---: | :---: | :---: | :---: |
| Corridor Condition | A | B | C |  |
| Good | $>80 \%$ cover of native plants, and <br> $>50 \%$ tree canopy |  | $\mathbf{8 2 \%}$ native plants <br> $\mathbf{8 3} \%$ tree canopy | $\mathbf{5 2 \%}$ tree canopy |
| Marginal | $50 \%-80 \%$ cover of native plants, and <br> $26-50 \%$ tree canopy |  |  |  |
| Degraded | $<50 \%$ cover of native plants, and <br> $\leq 25 \%$ tree canopy | $\mathbf{3 \%}$ native plants <br> $\mathbf{6 \%}$ tree canopy |  | $\mathbf{2 7 \%}$ native plants |

The condition of VC is defined by the percentages of native species and canopy cover.

- Community A is in degraded corridor condition, as the community lacks adequate tree canopy and is overwhelmingly dominated by non-native herbaceous species.
- Plant Community B has both a good native tree canopy, and high overall coverage of native species. As such, this community is in good corridor condition.
- Plant Community C is comprised of only 27 percent native species but has a variable tree canopy. As a result of this variability the tree canopy is 53 percent, just enough to fall within the lower range of good condition. The variability of tree canopy relative to the lower percent cover of plants justifies a corridor condition of marginal for Community C.


### 5.0 PROPOSED PROJECT

The purpose of the project is to construct light industrial buildings within the City of Sherwood's industrial area. The proposed project will construct five light industrial buildings totaling greater than 525,000 square feet, with associated infrastructure and utilities, parking, and stormwater treatment, within the City of Sherwood (Figure 4). The stormwater plan will adhere to CWS' design and construction standards (D\&C Standards).

### 5.1 Proposed Vegetated Corridor Encroachments

Approximately 10,699 square feet of permanent VC encroachment will result from site development (Figures 4-4C); to facilitate proposed grades. Individual encroachments are associated with the construction of Cipole Place (the onsite extension of Cipole Road), the access drive across the north side of Building D, a small area of grading west of Building E, as well as grading and a retaining wall behind (south of) Building C.

The total area of permanent encroachment also includes 100 square feet associated with each of three separate rip rap stilling basins associated with the site's stormwater outfalls. As each is a minor encroachment associated with utility infrastructure, and not more than 100 square feet in size, replacement mitigation is not necessary (per current CWS D\&C Standards, Chapter 3, Section 3.05 .5 c and d).

Temporary encroachments will be limited to a trio of stormwater outfall lines that lead to rip rap stilling basins; one each to the west and east sides of Wetland A, just south of Tualatin-Sherwood Road, and one at the south end of Wetland A. The alignments of associated pipelines have been sited to facilitate proper drainage. The installation of these pipelines will require a combined area of temporary encroachment of 4,917 square feet. The footprint of temporary encroachment is defined by a 20 foot wide construction corridor centered roughly along the proposed pipe alignments. Each of the three rip rap pads for the storm outfalls will require permanent encroachment of 100 square feet.

### 5.2 Vegetated Corridor Mitigation

Encroachment of 10,699 square feet for site development will be mitigated for through the expansion of an equivalent area of VC (Figures 4-4C). Though mitigation is not required for the 300 square feet of riprap stilling basins, mitigation will nonetheless be provided for these
encroachments. In total, 35,654 square feet of VC expansion will be provided. This includes a 1 to 1 replacement for proposed encroachments, as well as an additional 24,955 square feet of mitigation; proposed as a water quality benefit to the project.

VC expansion will occur within six separate areas. The largest and widest is located southeast of Wetland A. There are three additional areas west of Wetland A; the two smaller ones also adjoining VC associated with Wetland B. The two remaining areas will expand existing VC north and northwest of Wetland C. Proposed expansions will widen existing VC by up to 105 feet. As enhancements will be required throughout the first 50 feet of existing VC, enhancement of the proposed mitigation and water quality benefit expansion areas will occur concurrent with other invasive species control and plant installation improvements.

### 5.3 Tier 2 Alternative Analysis

As discussed above, the purpose of the project is to develop five light industrial building sites at the proposed T-S Corporate Park within the City of Sherwood. Site design has taken great efforts to avoid even temporary impacts to sensitive areas but due to constraints resulting from the required access point and onsite topography, avoidance of VCs is not achievable. Due to the depth of necessary encroachments along the proposed alignment of Cipole Place, a Tier 2 Alternatives Analysis is required. The proposed project will meet all Tier 2 Alternative Analysis criteria, as detailed below.

The proposed reductions in the width of VC meet the following criteria, as required under a Tier 2 analysis:

## 1. The proposed encroachment area is mitigated in accordance with Section 3.08.

As previously discussed, mitigation for permanent impacts to the VC will be achieved through onsite VC expansion at a replacement ratio of 1:1, as outlined in Section 3.08c of CWS D\&C Standards. An additional 24,955 square feet of VC expansion contiguous with existing VC is also proposed. The extent of mitigation is intended to protect water quality for public benefit.

Temporary encroachment to facilitate installation of site utilities will be mitigated in place. As the vicinity of these utilities is in degraded or marginal condition and dominated by non-native species, disturbance of native vegetation will not occur. All temporary encroachments will take place prior to the onset of enhancement actions, with disturbed areas to be vegetated with a native seed mix upon completion of project related activities. Remaining efforts for enhancement of the VC will occur in accordance with the site's landscape plan.

## 2. The replacement mitigation protects the functions and values of the Vegetated Corridor and Sensitive Area.

The VC to be impacted includes corridor in marginal or degraded condition. The replacement mitigation areas will be located immediately adjacent to the outer boundary of existing VC, functionally expanding the VC where much of the area is currently dominated by Himalayan blackberry. All proposed mitigation and required enhancements on-site will protect the functioning of adjoining VC and sensitive areas.
3. Enhancement of the replacement area, if not already in Good Corridor Condition, and either the remaining Vegetated Corridor on the site or the first 50 feet of width closest to the resource, whichever is less, to a Good Corridor Condition.

A replacement mitigation area of 10,699 square feet will be enhanced to "good" corridor condition in accordance with CWS standards. An additional 24,955 square feet of mitigation is proposed as a water quality benefit to the project. The proposed replacement areas were identified based upon the location and limits of the existing VC.

Enhancement to good condition will occur as required for all remaining areas of VC within 50 feet of identified sensitive areas. In total, 193,203 square feet ( 4.4 acres ) of VC will be mitigated or enhanced.

## 4. A District Stormwater Connection Permit is likely to be issued based on proposed plans.

The applicant reasonably expects to obtain a District Stormwater Connection Permit based on proposed plans for the project, which were designed in accordance with CWS standards.

## 5. Location of development and site planning minimizes incursion into the Vegetated Corridor.

Encroachment into the onsite VC has been minimized to the maximum extent practicable. Vegetated corridor encroachments are limited to those necessary for site access improvements, construction of the buildings as proposed, as well as to accommodate access roads, parking areas, stormwater treatment, and other required infrastructure.

The overall development has sought to maximize the developable area of the site. The applicant has taken great strides to avoid impacts to the wetlands themselves but unfortunately complete avoidance of VCs was not possible.

Preliminary site design confirmed that the areas where encroachments would be the most difficult to avoid were along Cipole Place, which is required for site access, and south of Building C.

- Building C. The initial site design including the utilization of steep slopes to avoid impacts to Wetland C, while still providing a stable slope above which the building could be constructed. The necessary contouring required 4,843 square feet of VC encroachment. It was later determined that a retaining wall could be utilized at that location, thereby reducing VC encroachments to 1,057 square feet. The retaining wall reduced the depth of encroachment from 47 feet down to a maximum depth of 15 feet. Similarly, the frontage length of encroachment was reduced from 150 feet, down to 100 feet.
- Cipole Place. This access street is necessary for site development and as a result of the existing intersection of Cipole Road and Tualatin Sherwood Road the applicant cannot alter the location where it enters the site. A typical intersection alignment would result in the perpendicular alignment of Cipole Place off of Tualatin Sherwood Road. The street alignment would then be modified south of that point, to facilitate access to the remainder of the site. Alternative Alignment Option 1 (on Figure 1) reveals what a typical alignment

[^25]scenario would entail. The result would have been the elimination of Wetland B and significant encroachment within the VC. The applicant however determined early on in the site design process to avoid all impacts to wetlands. The proposed roadway alignment shifts westward to avoid Wetland B and minimize encroachments to adjoining VC. The roadway design incorporates a retaining wall to eliminate the need for embankment slopes east of the street, as even with the modified alignment, Wetland B could not have been avoided without a wall, and VC encroachments would increase without a wall. The most recent minimization efforts associated with Cipole Place include the elimination of a sidewalk on the east side of the road. This change could only be achieved through an Engineering Design Modification with the City as their standards require sidewalks on both sides of the street.

The remaining encroachments proposed for the development are required to adequately site the proposed access roads, buildings, parking areas, stormwater treatment, and other required infrastructure within the developable portion of the site. The development as proposed on-site is dimensioned to meet the requirements of needed light industrial buildings within the City of Sherwood. Any changes to the site plan as proposed will impact the ability to maximize the site's usage for light industrial development. As such, proposed encroachments are limited to the greatest practical extent to make this project feasible. Section 6, below, details the reasons why this property was chosen for development of the buildings, and why encroachment into the VC is necessary.

## 6. No practicable alternative to the location of the development exists that will not disturb the Sensitive Area or Vegetated Corridor.

Oregon Statewide Planning Goals and Guidelines provide a framework for Oregon's municipalities to balance various public policies through a series of goals and guidelines that cover a variety of broad subjects, including economic development. Statewide Planning Goal 9 requires municipalities to "provide adequate opportunities throughout the state for a variety of economic activities, vital to the health, welfare, and prosperity of Oregon's Citizens." Among the guidelines for Goal 9 is that Comprehensive Plans for urban areas "provide for at least an adequate supply of sites of suitable sizes, types, locations, and service levels for a variety of industrial and commercial uses with plan policies."

The 2018 Sherwood Economic Opportunities Analysis (EOA) compares demand and supply of employment lands to evaluate the land inventory over a 20-year period. This report indicates that within the 282-acre Tonquin Employment Area, 144 acres are constrained (by factors such as wetlands), a category that includes the subject site. The City of Sherwood's (City) 20-year demand for vacant employment land is 116 acres and the 20 -year supply is 127 acres. The subject site is included in the City's inventory of land anticipated to meet the demand for industrial land.

The applicant has investigated additional properties within the Sherwood area. This corridor along Tualatin-Sherwood Road is developing rapidly, attesting to the market need for additional industrial property in the greater Sherwood and Tualatin areas. Existing underutilized industrial properties in this area are generally smaller parcels that will not support the larger sized industrial buildings proposed as part of the T-S Corporate Park.

Sites of this size are in short supply throughout the Portland Metropolitan Region, a situation that has been the subject of extensive analysis by Metro and the State of Oregon. As part of the broader
state and regional studies, the City of Sherwood has prepared and updated its Economic Opportunities Analysis, which inventories and discusses land availability and constraints within its jurisdiction. This analysis indicates that subdivision of larger industrial sites (including those in the Tonquin Employment Area) or development of multiple buildings to accommodate small and midsized sites will help ensure opportunities for small and mid-sized businesses. The applicant's proposed T-S Corporate Park is entirely consistent with this approach as it will result in five buildings with sizes ranging from 56,000 to 183,000 square feet.

The goal of the development is generally twofold: 1) to supply the existing strong demand for 50,000 to 180,000 square foot industrial buildings that accommodate light manufacturing, regional suppliers of parts and services (region to include the Willamette and Tualatin Valleys), and related or standalone office uses; and 2) to create quality employment opportunities within the City of Sherwood in a location that has access to essential services, alternative modes of transportation, and existing housing. The City of Sherwood views the development of the Tonquin Employment Area (which includes this site) as a critical piece of its economic development strategy.

The size of the buildings and the orientation of the site development plan have been chosen to create an efficient use of the usable portion of the property. Smaller buildings or multi-story buildings that might occupy smaller parcels would not address the market in the area, which is for single story industrial buildings in the 50,000 square foot and larger range. It is typical that many of the lessees or purchasers of buildings of this type are locally owned or regional offices of national companies. Employees may live throughout the region, but the presence of nearby housing in Sherwood encourages living nearby. Companies that would occupy these buildings would choose this location for its access to Tualatin-Sherwood Road, Oregon Highway 99W, and I-5, which allow for rapid transportation of goods and services to customers in the Willamette and Tualatin Valleys without the delays of Portland freeway congestion.

The site layout as proposed will result in a very efficient use of the site and allow the construction of buildings that meet current marketplace demands and accomplish planning goals within the City of Sherwood.

Three alternatives were considered for the site. The primary encroachments for site development are associated with the proposed alignment of Cipole Place; Alternatives 2 and 3 focus on that element of the project. As the County is not allowing access to SW 124th Avenue, access from the intersection of Cipole Road and Tualatin Sherwood Road is the only option. All development options must balance the constraints of site development with avoidance of existing sensitive areas and VC.

Alternative 1: "No build" alternative. The no build alternative means that the project would not be constructed. Needed light industrial buildings within the City of Sherwood area would not be constructed, and there would continue to be limited light industrial buildings available for lease or purchase within the City of Sherwood.

Alternative 2: The Applicant considered alternative layouts for the project design to meet the Applicant's needs, while minimizing, to the extent feasible, the impact to VC on the project site. As mentioned above, the location of the intersection of Cipole Place with the development site cannot
be modified. The intersection is determined by the existing location of Cipole Road relative to Tualatin-Sherwood Road. Avoiding the VC entirely for the construction of Cipole Place would result in a horizontal curve on the proposed street that would be too tight to reasonable accommodate the anticipated semi-trailer trucks that will access the street. The eastbound to southbound right turn moment from Tualatin-Sherwood onto Cipole is particularly problematic due the large radius of the WB-67 truck trailers that will frequent the site. Additionally, shifting the alignment of Cipole Place west of the proposed location to avoid VC encroachment would also alter the location of a public utility easement to provide future utility access for water, sewer, and storm from Tualatin Sherwood Road to Blake Road required for future development (by others) south of the site. The utility easement is a 35 -foot public easement required by the City of Sherwood. The installation of these utilities would require future encroachment through the regulated VC and proposed VC mitigation area (see Figure 2, Alternative Alignment Option 2). The utility easement alignment is shown as a straight line to match the geometry of the easement requested by WWSP (Willamette Water Supply Program), who is designing Blake Road and the water treatment facility to be constructed south of this project.

Additional reduction of encroachment through narrowing of the roadway profile was also investigated. The cross-section for the public street is determined by City Engineering standards, providing few options for minimization. A few opportunities were identified and are discussed in Alternative 3. The applicant also explored re-aligning Cipole Place, but other alignments did not ultimately allow for the construction of a safe, functional alignment that would also reduce VC encroachments.

Alternative 3: Preferred alternative. Alternative 3 is the construction of five industrial buildings, required parking, and stormwater treatment, as described above in Section 5. This alternative proposes VC encroachment sufficient to facilitate the project's five industrial buildings with sizes ranging from 56,000 square feet to 183,000 square feet and VC expansion mitigation. The five buildings will accommodate a range of light industrial, manufacturing, or warehouse/distribution uses. This alternative proposeds an alignment of Cipole Place that avoids wetland impacts and minimizes VC encroachment.

The proposed design avoids roadside LIDA facilities, as stormwater will be managed in centralized facilities in Tracts B, C, and E (outside the VC). City Engineering staff has indicated that street parking would not be allowed unless the roadway were widened. Since a wider roadway would either increase VC encroachment or cause ripple effects on the site plan that compromise the functionality of the industrial park, the applicant chose to eliminate on-street parking. City of Sherwood Engineering staff initially indicated that they would not approve eliminating a sidewalk on the east side of Cipole Place, and as a result that minimization option was not available at the time of initial submittal. The City is now willing to review an Engineering Design Modification and the elimination of the eastern sidewalk is now reflected in the revised development plan figures (Figures 4 through 4C). The original and current designs utilize retaining walls between the sidewalk and Wetland A rather than 3:1 side slopes (see Figure 2 Cipole Street Section). The applicant did examine shifting the cul-de-sac bulb southward to minimize or eliminate VC impacts from the Building D/Building E driveway, but due to the site's steep topography, this had negative impacts on the site design (e.g., building orientation changes, impacts on finished floor elevations, and changes to site grades and dimensions that inhibit truck circulation).

## 7. The proposed encroachment provides public benefits.

The public benefit of VC encroachment includes supporting Regional, County, and City Goals for employment growth via construction within the urban growth boundary (UGB). The site is a designated Industrial area in Metro's Urban Growth Management Functional Plan (UGMFP) Title 4 Industrial and Other Employment Areas map (October 2014). Section 3.07.410 of the UGMFP stipulates in part that "To improve the economy, Title 4 seeks to provide and protect a supply of sites for employment..." Accordingly, after the Tonquin Employment Area (TEA) was brought into the UGB, the City designated the TEA for industrial development and established the Employment Industrial (EI) zone with limits on the size and scope of non-industrial uses. Now that the property has been annexed into the City and zoned EI, the applicant seeks to maximize opportunities for industrial development and employment. The proposed VC encroachment will provide a public benefit by allowing for increased building sizes than could not be accommodated without the encroachment. Because usable square footage will be higher, employment opportunities will also increase for area residents.

Full build out of this site will allow the City to maximize development within the City's UGB. This is a financial benefit to the City, as the City will accrue taxes from the proposed development. The taxes can be used to fund public schools, infrastructure, police and fire, and other expenses related to City management.

Currently, the majority of Sherwood's workforce commutes to locations outside the City. The social benefits to the City include the development of additional industrial development opportunities in an area already dedicated to that use. The site will not result in increased traffic within an existing residential area or degrade the quality of life in an adjoining residential or commercial area as there are none in the immediate vicinity. The site is, however, located within just few miles of residential and retail areas in Sherwood, to the west, and Tualatin, to the east.

Close in development (i.e. within the UGB) allows people to work in close proximity to their residence. This reduces the need for longer commutes, reducing air and water pollutants generated by auto travel. Air and water quality benefits are possible because the project will offer additional local jobs, creating an opportunity for local employment and a potential decrease in the generation of pollutants associated with auto travel to job opportunities elsewhere in the Portland Metro area.

Allowing encroachment into the VCs allows for maximum build out of the site, which requires adequate space for the buildings, parking, stormwater, and additional infrastructure necessary to construct needed light industrial buildings within the City of Sherwood.

Finally, in order to provide an additional water quality benefit beyond the required 10,699 square feet of VC mitigation, the VC will be expanded by an additional 24,955 square feet to encompass a combined mitigation area of 35,654 square feet, which is above and beyond the mitigation area required for project related encroachments.

### 5.4 Discussion of Wetland and Vegetated Corridors Functions and Values

As a requirement of the Tier 2 analysis, a function and values assessment is required for the sensitive areas and VCs on site. The functions and values of Wetlands A, B, and C, as well as the
adjoining VCs were assessed within the study area using the Hydrogeomorphic (HGM) Classification Judgmental Assessment Method.

## Water Quality and Quantity

Functions: Water Storage and Delay, Sediment Stabilization and Phosphorus Retention, Nitrogen Removal

Groundwater and precipitation contribute to hydrology within all three wetlands. Observed flooding in Wetland C would suggest an upslope source of seasonal hydrology though it is possible that much of the water is sourced by groundwater discharge from the adjoining rocky substrate. Wetland B is functionally isolated due to its size and distance from the other two wetlands. There is no opportunity for water storage and delay functions in Wetland B as it is a slope/flat wetland with no opportunity to intercept or detain water from upslope sources. Wetland A has only a limited opportunity for water retention as there is a ditch along its west side that conveys water directly to the culvert under Tualatin-Sherwood Road. Wetland C does however provide a water quality function as it does retain floodwaters seasonally.

Water quality functionality is currently limited for Wetlands A and C as stormwater runoff is from largely undeveloped areas and water quality is presumed to be quite good. As development increases to the south the opportunity for the filtration and removal of sediment and pollutants will increase.

The VCs to be impacted by this project rate low for water quality because they are rarely inundated with water; the forested portions of the VCs along Wetland C provide shade but as the wetland is already forested, and inundation occurs in the winter when many of the trees lack leaves, the VC provides little benefit than what can be found within the wetland itself.

## Fish and Wildlife

Functions: Primary Production, Thermoregulation, Resident Fish Habitat Support, Anadromous Fish Habitat Support, Invertebrate Habitat Support, Amphibian and Turtle Habitat, Breeding Waterbird Support, Wintering and Migratory Waterbird Support, Songbird Habitat Support

The mix of open and wooded or shrub areas next to Wetlands A and B, make those areas of limited use for waterbird support, or amphibians and turtles, though seasonal use by a couple pair of ducks or geese may be possible.

Community B surrounding Wetland C includes a variety of native food sources. The density of tree and shrub cover would provide habitat for large and small mammals, and bird species. There is no habitat support for fish or waterfowl, due to the limited seasonality of surface water and extent of woody cover and steep, rocky conditions bordering the wetland.

The mix of herbaceous and shrub communities to the north, with proximity to forested areas to the south, make the entire site good for songbird habitat support. Forested conditions exist along the channel through Wetland A, as well as within and adjoining Wetland C, but the limitation of water presence to just the winter and spring means there is little opportunity for these forested resources to provide a thermoregulation benefit; by the time that water cooling is of benefit these features no longer retain surface water. Though the grasslands likely provide good habitat for common
invertebrate species, the extent of historical disturbance and lack of unique habitat make it of no particular benefit to more than common species.

## Native Plant Communities and Species Diversity

## Function: Support of Characteristic Vegetation

Wetland A includes some native tree and shrub species but otherwise Wetlands A and B and their adjoining VCs are dominated by non-native species. Wetland C on the other hand is predominantly comprised of native tree and shrub species, as is VC Community B that borders it.

## Recreation and Education

The site is located on private property. Additionally, Himalayan blackberry and poison oak are both common on the site, creating conditions that make accessing the wetlands difficult. As such, recreation and educational opportunities are limited.

### 5.5 Vegetated Corridor Enhancement

As required, enhancement of all areas within 50 feet of existing sensitive areas will be enhanced. A short section of regulated VC near the south end of the site does exceed 50 feet in width and will not be enhanced. In total, 157,549 square feet of enhancement will occur on the lot.

The overall goal of enhancement is to improve the corridor to 'Good' condition. Enhancement activities will be consistent with Clean Water Services’ standards (Appendix A: Planting Requirements of CWS current D\&C standards). Required enhancement measures for the marginal and degraded portions of the VC include removing invasive/noxious vegetation, with a focus on removing Himalayan blackberry, Scotch broom, and lesser amounts of thistle and reed canarygrass. Invasive removal will occur in all communities, including Community B, which is already in good condition. Following invasive species removal it will be necessary to plant native trees, shrubs and groundcover.

As Community B is already in good condition, it is not anticipated that supplemental tree or shrub plantings will be necessary, though groundcover species may be appropriate in limited areas greater than 25 square feet in size following invasive species removal. Communities A and C include few native trees or shrubs, so unless indicated otherwise by a landscape architect, standard CWS densities for trees and shrubs should be utilized, with groundcover species in the form of a native grass seed mix recommended following invasive species removal and the installation of woody species.

### 6.0 REFERENCES

Clean Water Services, 2019. Design and Construction Standards (R\&O 19-5 as Amended by R\&O 19-22).
US Geologic Survey, The National Map Viewer, 2014 (Sherwood quadrangle).

## Appendix A

Figures


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General Location and Topography T-S Corporate Park - Sherwood, Oregon United States Geological Survey (USGS), Sherwood, Oregon, 7.5 Quadrangle, 2014
(viewer/nationalmap.gov/basic)

FIGURE 1



LEGEND

-     - Study Area Boundary

(Site Total 159,518 sf / 3.66 ac)
- 8 Vegetated Corridor

신(170,844 sf/3.92 ac)
$0^{\prime}$
$\%$
Slope Measurement


|  | Existing Conditions T-S Corporate Park - Sherwood, Oregon | FIGURE 2A <br> 12-26-2019 |
| :---: | :---: | :---: |

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|  | Vegetated Corridor Plant Communities Overview and Sheet Index T-S Corporate Park - Sherwood, Oregon | FIGURE 3 |
| :---: | :---: | :---: |
| Pacific Habitat Services, Inc 9450 SW Commerce Circle, Suite 180 Wilsonvile, Oregon 97070 Phone: (503) 570-0800 Fax (503) 570-0855 |  | 1-3-2020 |

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Site Development Plan and Vegetated Corridor Encroachment Fick



## Appendix B

## Vegetated Corridor Sample Points Table and Photo documentation



T-S Corporate Park - Vegetated Corridor Sample Sites

| Plant Community Sample Point | Community A - Herbaceous |  |  |  |  |  |  | Community B - Native Forest |  |  |  |  |  | Community C-Scrub Shrub |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3 | 7 | 11 | 16 | 17 | 24 |  | 27 | 30 | 32 | vc-1 | DEA-5 |  | 8 | 21 | 23 | VC-2 |  |
| TREES |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Native |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  | 20 |  |  | 30 |  |  |  |  |  |  |  |
| Arbutus menziesii |  |  |  |  |  |  |  |  | 5 |  | 5 |  |  |  |  |  |  |  |
| Fraxinus latifolia |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 5 |  |
| Prunus emarginata |  |  |  |  |  |  |  |  |  |  |  | 35 |  |  |  |  |  |  |
| Pseudotsuga menziesii |  |  |  |  |  |  |  | 20 |  | 25 | 15 | 15 |  |  |  |  |  |  |
| Quercus garyanna |  |  |  |  |  |  |  |  | 20 | 15 | 25 |  |  |  |  |  |  |  |
| Salix lasiandra |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Salix sitchensis |  |  |  |  |  |  |  |  |  | 10 |  | 10 |  |  |  | 50 | 10 |  |
| Non native |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Crataegus monogyna |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 20 | 60 |  |  |
| Malus pumila (apple) |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 30 |  |  |  |
| Prunus avium |  |  |  |  |  |  |  |  |  | 15 |  |  |  | 15 |  |  |  |  |
| SHRUBS \& SAPLINGS |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Native |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Amelanchier alnifolia |  |  |  |  |  |  |  |  | 40 |  | 5 |  |  |  |  |  |  |  |
| Corylus cornuta |  |  |  |  |  |  |  | 50 | 20 | 60 | 35 | 10 |  | 5 |  | 15 |  |  |
| Fraxinus latifolia |  |  | 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Gautheria shallon |  |  |  |  |  |  |  |  | 10 |  |  | 25 |  |  |  |  |  |  |
| Holodiscus discolor |  |  |  |  |  |  |  |  |  | 2 | 10 | 15 |  |  |  |  |  |  |
| Mahonia aquifolium |  |  |  |  |  |  |  | 10 |  | 10 |  |  |  |  |  |  |  |  |
| Oemleria cerasiformis |  |  |  |  |  |  |  | 5 |  |  |  |  |  |  |  | 1 |  |  |
| Polystichum munitum |  |  |  |  |  |  |  |  | 10 | 2 |  |  |  |  |  |  |  |  |
| Rosa pisocarpa |  |  |  |  |  |  |  |  |  |  | 5 |  |  |  |  |  |  |  |
| Rubus ursinus |  |  |  |  |  |  |  | 5 | 10 | 20 | 10 | 10 |  |  |  | 25 |  |  |
| Symphoricarpos albus |  |  |  |  |  |  |  | 20 |  |  |  | 10 |  |  |  |  |  |  |
| Toxicodendron diversilobum |  |  |  |  |  |  |  |  | 10 | 5 | 60 |  |  |  | 20 |  |  |  |
| Invasive |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Rubus armeniacus | 10 |  | 40 | 15 | 5 | 5 |  | 5 |  | 10 | 5 |  |  | 10 | 10 | 20 | 100 |  |
| Non native |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Crataegus monogyna |  |  | 2 |  |  |  |  |  |  |  |  |  |  |  |  | 20 |  |  |
| Prunus avium |  |  |  |  |  |  |  |  |  |  |  |  |  | 15 |  |  | 10 |  |
| HERBS |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Native |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Agrostis exarata |  |  |  |  |  |  |  |  |  |  |  |  |  | 40 |  |  |  |  |
| Carex hendersonii |  |  |  |  |  |  |  | 5 |  |  |  |  |  |  |  |  |  |  |
| Carex leptopoda |  |  |  |  |  |  |  |  |  | 25 | 5 |  |  |  |  |  |  |  |
| Equisetum arvense |  |  | 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Galium aparine | 20 |  |  |  |  |  |  |  |  |  |  |  |  |  | 5 |  |  |  |
| Maiathemum stellatum |  |  |  |  |  |  |  | 5 |  |  |  |  |  |  |  |  |  |  |
| Polypodium glycyrrriza |  |  |  |  |  |  |  |  |  |  | 50 |  |  |  |  |  |  |  |
| Invasive |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Cirsium anvense |  |  | 35 | 5 |  |  |  |  |  |  |  |  |  | 25 | 10 |  |  |  |
| Non Native |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Agrostis capillaris | 20 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Agrostis stolonifera |  | 25 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Anthoxanthum odoratum |  |  |  |  |  | 5 |  |  |  |  |  |  |  |  |  |  |  |  |
| Cardamine oligosperma | 5 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Daucus carota |  | 5 | 10 | 10 | 10 |  |  |  |  |  |  |  |  | 15 |  |  |  |  |
| Epilobium ciliatum | 5 |  |  |  |  | 1 |  |  |  |  |  |  |  |  |  |  |  |  |
| Festuca rubra |  | 10 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Geranium lucidum | 20 |  |  |  |  |  |  |  | 30 | 70 | 90 |  |  |  |  |  |  |  |
| Geranium molle |  |  |  |  |  |  |  |  |  |  |  | 5 |  |  |  |  |  |  |
| Holcus lanatus |  | 20 | 60 | 20 | 45 | 20 |  |  |  |  |  |  |  | 5 |  |  |  |  |
| Hypericum perforatum | 10 | 5 |  |  |  | 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| Jacobaea vulgaris |  |  |  | 20 | 10 | 5 |  |  |  |  |  |  |  |  |  |  |  |  |
| Leontodon saxatilis |  |  |  |  |  | 5 |  |  |  |  |  |  |  |  |  |  |  |  |
| Lolium sp. | 10 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Lotus corriculatus |  |  |  | 5 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Plantago lanceolata |  |  |  | 5 |  | 1 |  |  |  |  |  |  |  |  |  |  |  |  |
| Poa annua |  |  |  |  |  | 5 |  |  |  |  |  |  |  |  |  |  |  |  |
| Poa pratensis |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 85 |  |  |  |
| Rumex acetosella |  |  | 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Rumex crispus | 5 |  |  |  |  | 5 |  |  |  |  |  |  |  | 5 |  |  |  |  |
| Schedonorus arundinaceus |  |  |  | 30 | 5 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Taraxacum officinale |  | 5 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Trifolium pratense |  |  |  |  | 10 | 20 |  |  |  |  |  |  |  |  |  |  |  |  |
| Vica sp. |  | 20 |  |  | 15 |  |  | Community B |  |  |  |  |  |  | Community C |  |  |  |
|  | Community A |  |  |  |  |  | Average |  |  |  |  |  | ${ }_{\text {Average }}{ }_{83}$ | 70 | Comm | nity C | 10 | Average |
| \% Native Species | 19 | 0 | 1 | 18 | 0 | 0 | $\stackrel{3}{15}$ | ${ }^{97}$ | 81 | 65 | 73 | 96 | $\frac{82}{2}$ | 33 | 14 | 48 | ${ }^{12}$ | ${ }_{32}^{27}$ |
| Total cover | 105 | 90 | 150 | 110 | 100 | ${ }^{87}$ | 15 | 145 | 155 | 269 | 350 | 135 | 2 | 135 | 180 | 191 | 125 | 32 |

Canopy cover includes trees located beyond 30 feet from the assessment area; including trees upslope and within or across adjoining sensitive areas. Tall shrubs and small trees up to 20 feet tal


## Photo A:

Looking south, west of Wetland A. To the right is Community A , and to the left, Community B.

Photo taken: November 8, 2019

## Photo B:

Looking northeast; east of Wetland A .

Photo taken: November 8, 2019



## Photo C:

Looking southwest along Wetland A. Community A is located in the left foreground. The more contiguous thicket of blackberries is the beginning of Community C .

Photo taken: November 8, 2019

## Photo D:

Looking east across Wetland B in agricultural field, toward Wetland A. This is an older photo but it still accurately represents the transition from herbaceous Community A to Community C (the shrubs in the background).

Photo taken: April 28, 2017


Photo documentation
T-S Corporate Park - Sherwood, Oregon


## Photo E:

Looking north into Community C north of Wetland C .

Photo taken: October 15, 2019

Photo F:
Looking east along edge of Wetland C ; depression dries out later in spring. Community B begins beyond the limits of inundation, which corresponds to the edge of wetland in this area.

Photo taken: March 23, 2017


Project \#6163
12/27/2019


Photo documentation
T-S Corporate Park - Sherwood, Oregon


## Photo G:

Looking north along the west edge of Wetland C. Community B begins at the wetland edge.

Photo taken: October 15, 2019

## DロWL

## Preliminary Drainage Report

T-S Corporate Park
2322.14347.01


Prepared for
Trammell Crow Company
1300 SW $5^{\text {th }}$ Avenue, Suite 3050
Portland, OR 97201

March 4, 2020

| Prepared for | Trammell Crow Company |
| :--- | :--- |
| Project Name | Drainage Report |
| Job Number | 2322.14347 .01 |
| Date | March 4,2020 |

DOWL
720 SW Washington Street, Suite 750
Portland, Oregon
97205

Telephone: 971-280-8641
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| Name | Title |  | Date | Revision |
| :---: | :---: | :---: | :---: | :---: |
| Reviewer |  |  |  |  |
| Mike Gillette | Civil Designer | $01 / 15 / 2020$ | 0 | Ryan Halvorson |
| Mike Gillette | Civil Designer | $03 / 4 / 2020$ | 1 | Ryan Halvorson |

## Executive Summary

The proposed project will develop a new industrial park on the undeveloped land at the southwest corner of SW Tualatin-Sherwood Road and SW $124^{\text {th }}$ Avenue in Sherwood, Oregon. The proposed development includes five new buildings, associated parking, landscape, utility connections, and stormwater facilities.

The proposed storm design will meet the requirements of the Clean Water Services as Design and Construction Standards for Sanitary Sewer and Surface Water Management, December 2019. The project proposes to create over $1,000 \mathrm{sf}$ of impervious area; therefore, a hydromodification assessment is required. Using Clean Water Services Hydromod Planning Tool, the development is located within an expansion area, and the site has a hydromodification risk of both low and high (resulting in a high-risk designation). Since the site development is approximately 32.02 acres, the development is classified as a Category 3 large project based on Clean Water Services Table 4-2. As a result, the development is required to have at least 30 percent of the proposed impervious area be treated and detained in LIDA facilities.

The proposed site is split into three overall main drainage basins, West, Cipole, and East (see appendix Figure 2). On-site water quality treatment and water quantity flow control will be provided by three proposed extended dry basin LIDA facilities, treating and detaining for 100 percent of the proposed development. Water quantity detention and flow control systems were designed to limit the 2,5 , and $10-$ year 24-hour storms to discharge per Clean Water Services standards. Under the hydromodification requirements, the $2-y r$ post developed flow must be released equal to or less than $50 \%$ of the $2-\mathrm{yr}$ predeveloped flow. In addition, the 5-year and 10-year post developed storm events are released to less than the respective 5 -year and 10-year predeveloped storm event flows. The proposed private conveyance system will be designed using the 25 -year storm event in the final drainage report.
Treated and detained runoff will exit the extended basin facilities at two locations both connecting to the existing storm lines in SW Tualatin-Sherwood Rd. The west basin (development west of Cipole Place) consists of lots 1,2 , and 3 , discharging to an existing 18 " storm line which goes under Tualatin-Sherwood north. This 18 " storm line connects to an existing conveyance system in Wildrose Place traveling north more than $1 / 4$ mile from the site. The central overall basin (Cipole Place) and the east basin (development east of Cipole) discharge to the existing wetland. Stormwater from the existing wetland is conveyed by a 36 " storm line under Tualatin-Sherwood to the north side, where it enters a large road side ditch. Stormwater is conveyed east and is collected in an 18 " storm line and connects to the existing 24 " storm line in Tualatin-Sherwood Rd. A final downstream analysis will be provided in the final stormwater report.
The proposed private conveyance will be designed using the 25 -year storm event in the final drainage report. The proposed public conveyance will be designed to convey the 25 -year storm event without surcharging in the final drainage report.
The proposed storm design will meet the requirements of both the City of Sherwood and Clean Water Services as listed in the Design and Construction Standards for Sanitary Sewer and Surface Water Management issued December 2019.

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## 1 Project Overview

### 1.1 Project Overview

The proposed project will develop a new industrial park on the undeveloped land at the southwest corner of SW Tualatin-Sherwood Road and SW $124^{\text {th }}$ Avenue. The proposed development includes five new buildings, associated parking, landscape, utility connections, and stormwater facilities.

### 1.2 Location

The project is located at the southwest corner of SW Tualatin-Sherwood Road and SW $124^{\text {th }}$ Avenue in Sherwood, Oregon (See Figure 1-1 - Vicinity Map).

Figure 1-1 Vicinity Map


### 1.3 Methodology

The proposed storm design will meet the requirements of Clean Water Services as listed in the Design and Construction Standards for Sanitary Sewer and Surface Water Management, December 2019.

## 2 Existing Conditions

### 2.1 Topography

The existing property is undeveloped pervious area with grass, trees, dense brush, or wetlands. The site has gradual slopes between 1 and $8 \%$ generally draining north. The highest elevation of 255 ' is located on the west side of the property. The lowest elevation of $185^{\prime}$ is located along the north property line.

## $2.2 \quad$ Climate

The site is in Sherwood, Oregon and is located approximately 52 miles inland from the Pacific Ocean. There is a gradual change in seasons with defined seasonal characteristics. Average daily temperatures range from $48^{\circ} \mathrm{F}$ to $71^{\circ} \mathrm{F}$. Record temperatures recorded for this region of the state are $7^{\circ} \mathrm{F}$ and $108^{\circ} \mathrm{F}$. Average annual rainfall recorded in this area is 40 -inches. Average annual snowfall is approximately 5.3inches between December and February.

## $2.3 \quad$ Site Geology

The underlying soil types on the site, as classified by the United States Department of Agriculture Soil Survey of Washington County, Oregon are identified in Table 2-1 (See Technical Appendix: Hydrologic Soils Map - Washington County).
Table 2-1 Soil Characteristics

| Soil Type | Hydrologic Group |
| :---: | :---: |
| Aloha Silt Loam | $\mathrm{C} / \mathrm{D}$ |
| Huberly Silt Loam | $\mathrm{C} / \mathrm{D}$ |
| Quatama loam, 7 to $12 \%$ slopes | C |
| Saum silt loam, 7 to $12 \%$ slopes | C |
| Xerochrepts-Rock outcrop complex | D |

The site is assigned a soil Group D. Group D soils have very slow infiltration rates when thoroughly saturated and a very slow rate of water transmission.

### 2.4 Curve Number

The curve number represents runoff potential from the soil. The major factors for determining the curve number values are hydrologic soil group, cover type, treatment, hydrologic condition and antecedent runoff condition. The pervious curve number of 77 representing a brush, weed, and grass mixture was used at the site. (See Technical Appendix: Table 2-2c - Runoff Curve Numbers for Other Agricultural Lands).

## $2.5 \quad$ Time of Concentration

The time of concentration $\left(T_{C}\right)$ as described in NEH4 Chapter 15 is defined as the time for runoff to travel from the furthermost point of the watershed to the point in question, and the time from the end of excess rainfall to the point of inflection on the trailing limb of the unit hydrograph. Time of concentration can be estimated from several formulas. Clean Water Services guidelines which are based on the NRCS method were used in this analysis. A time of concentration was calculated for the site in existing conditions and found to be 26 and 31 minutes for Basins 1 and 2. (See Technical Appendix: Time of Concentration).

### 2.6 Hydrology

Stormwater runoff from the existing site that does not infiltrate generally drains to two outlet locations on the site. Basin 1 is on the west side and drains north to a ditch in the ROW that connects to the public storm system. Basin 2 generally drains to the wetlands in the center of the site. At the lower end of the wetlands, there is a 36" concrete storm pipe that runs north under Tualatin Sherwood Road into a ditch on the north side of the road.

### 2.7 Basin Area

Impervious and pervious surface areas for the existing site are shown in Table 2-2. The site is $100 \%$ pervious is existing conditions (See Technical Appendix: Figure 1 - Existing Conditions).
Table 2-2 Existing Basin Areas

| Basin ID | Impervious Area, ac | Pervious Area, ac | Total Area, ac |
| :---: | :---: | :---: | :---: |
| Basin 1 | 0.00 | 10.28 | 10.28 |
| Basin 2 | 0.00 | 21.74 | 21.74 |

## 3 Proposed Conditions

## $3.1 \quad$ Curve Number

The pervious curve number of 80 was used for the landscaped areas, and an impervious curve number of 98 will be used for proposed roofs, asphalt, and concrete surfaces.

### 3.2 Time of Concentration

A time of concentration of 5 minutes was used for the developed basins.

### 3.3 Basin Area

Impervious and pervious surface areas for proposed conditions are shown in Table 3-1. The site is $83 \%$ impervious in proposed conditions (See Technical Appendix: Figure 2 - Proposed Conditions).

Table 3-1 Proposed Basin Areas

| Basin | Impervious Area (ac) | Pervious Area (ac) | Total Area (ac) |
| :---: | :---: | :---: | :---: |
| Lot 1 | 4.51 | 0.80 | 5.31 |
| Lot 2 | 3.16 | 0.55 | 3.71 |
| Lot 3 | 7.40 | 1.53 | 8.93 |
| Lot | 6.71 | 1.81 | 8.52 |
| Lot 5 | 3.85 | 0.66 | 4.51 |
| SW Cipole Pl | 0.91 | 0.13 | 1.04 |
| SW 124th Ave | 1.13 | 0.11 | 1.24 |
| Total | $\mathbf{2 7 . 6 7}$ | $\mathbf{5 . 5 9}$ | $\mathbf{3 3 . 2 6}$ |

### 3.4 Hydrology

On-site runoff will be collected in trapped catch basins before being routed to stormwater LIDA facilities sized to meet CWS water quality and flow control requirements. Runoff will be treated and detained through these systems before exiting the site at one of two locations (See Technical Appendix: Figure 2 Proposed Conditions). The stormwater system was designed to meet CWS hydromodification requirements, which classify this as a Category 3 (large) project. To meet this requirement, LIDA facilities must be used to treat at least $30 \%$ of the impervious area on site and post developed release rates must match predeveloped release rates for $50 \%$ of the $2-\mathrm{yr}$, $5-\mathrm{yr}$, and $10-\mathrm{yr} 24$ hours storms (Table $4-7$ in the CWS Design and Construction Standards, December 2019).

On-site treatment will be provided in three LID stormwater treatment and detention ponds designed per Section 4.09.2 of the CWS Design and Construction Standards, December 2019 (See Technical Appendix: Figure 2 - Proposed Conditions). Flow control structures will be at the outlet end of the ponds, with orifice controls and standpipes designed to meet CWS requirements for post developed release rates.
Treated and detained runoff from the Tract B pond will exit the site at the existing Basin 1 outlet into an existing 18 " pipe that runs under Tualatin-Sherwood Road and eventually connects to an existing ditch north of SW Wildrose Place. Treated and detained runoff from the Tract C and E ponds will exit the site at the existing Basin 2 outlet into the existing 36 " concrete pipe that runs under Tualatin-Sherwood Road and outlets to a ditch along the north side of the road. A full analysis of the downstream conditions will be provided in the final storm report.

## 4 Hydrologic and Hydraulic Analysis

### 4.1 Design Guidelines

The analysis and design criteria used for stormwater management described in this section will follow the Clean Water Services Design and Construction Standards for Sanitary Sewer and Surface Water Management, December 2019. Section 5.04.2 describes the allowable flow determination methods including the selected TR-55 NRCS method.

## $4.2 \quad$ Hydrologic Method

Naturally occurring rainstorms dissipate over long periods of time. The most effective way of estimating storm rainfall is by using the hydrograph method. The NRCS Curve Number method is described in the NRCS National Engineering Handbook - Section 4. The NRCS runoff method equation is:
$Q=\frac{\left(P-I_{a}\right)^{2}}{\left(P-I_{a}\right)+S}$
Where:
$\begin{array}{llrl}\mathrm{Q}= & \text { Runoff (cfs) } & \mathrm{P}= & \text { Rainfall (inches) } \\ \mathrm{S}= & \text { Potential maximum retention after runoff begins } & \mathrm{I}_{\mathrm{a}}= & \text { Initial abstraction }\end{array}$
During the development of a runoff hydrograph, the above equation is used to compute the incremental runoff depth for each time step from the incremental runoff depth given by the design storm hydrograph.

The runoff function of XPSWMM generates surface and subsurface runoff based on design or measured rainfall conditions, land use and topography. XPSWMM version 16.1 was used for our hydrology and hydraulics analysis. XPSWMM is based on the public EPA SWMM program and is an approved method of analysis by Clean Water Services.

### 4.3 Design Storm

The rainfall distribution to be used within the Clean Water Services jurisdiction is the design storm of 24hour duration based on the standard Type 1A rainfall distribution. Table 4-1 shows total precipitation depths for different storm events. The CWS Design Storm Distribution for a type 1A 24-hour rainfall distribution for a 25-year storm event is shown in Figure 4-1.

Table 4-1 Precipitation Depth

| Recurrence interval (years) | Total Precipitation Depth (in) |
| :---: | :---: |
| 2 | 2.50 |
| 5 | 3.10 |
| 10 | 3.45 |
| 25 | 3.90 |
| 100 | 4.50 |

## Figure 4-1 25-Year Clean Water Services Type 1A Rainfall Ditribution



### 4.4 Basin Runoff

Table 4-2 lists the runoff rates for proposed conditions for the entire site during the 2,5,10 and 25-year storm events as calculated from the XPSWMM model. These values do not include on site detention.

Table 4-2 Proposed Discharge Flow Rates

| Recurrence interval <br> (years) | Existing Peak Runoff <br> Rate (cfs) | Proposed Peak Runoff <br> Rate (cfs) |
| :---: | :---: | :---: |
| 2 | 2.64 | 11.79 |
| 5 | 4.88 | 15.06 |
| 10 | 6.34 | 16.99 |
| 25 | 8.32 | 17.74 |

## 5 Conveyance Analysis

### 5.1 Design Guidelines

The analysis and design criteria described in this section will follow the Clean Water Services as listed in the Design and Construction Standards for Sanitary Sewer and Surface Water Management, December 2019. The manual requires storm drainage system and facilities be designed to convey the 25 -year storm event without surcharge.

## $5.2 \quad$ System Capacity

The proposed conveyance system will be designed to convey and contain the peak runoff from a 25 -year design storm. The proposed conveyance system will have sufficient capacity to handle all storm events up to and including the 100 -year storm event.

## $5.3 \quad$ System Performance

A full conveyance analysis will be provided in the final report.

## 6 Water Quality

### 6.1 Design Guidelines

The proposed water quality facilities were designed per the requirements set forth in the Clean Water Services Design and Construction Standards for Sanitary Sewer and Surface Water Management, December 2019. The facilities were designed using a rainfall depth of 0.36 " over a 4 -hour period with a return period of 96 -hours. Per Section 4.08 .5 , the water quality volume and flow rate are calculated according to the equations below:
Water Quality Volume $(\mathrm{cf})=\frac{0.36(\mathrm{in}) \times \text { Area }(\mathrm{sf})}{12(\mathrm{in} / \mathrm{ft})} \quad$ Water Quality Flow $=\frac{\mathrm{WQV}(\mathrm{cf})}{14,400}$
Clean Water Services requires pre-treatment prior to proposed water quality facilities. Trapped catch basins are an approved pre-treatment facility and will provide utilized on the site.

### 6.2 Water Quality Facilities

Water quality treatment will be provided in the proposed LIDA ponds, which will be designed as extended dry basins per Section 4.09.5. See Table 6-1 below for facility information. Hydrographs and pond stage graphs can be found in the Technical Appendix

Table 6-1 Extended Dry Basin Table

| Facility | Bottom <br> Area (sf) | Side <br> Slopes | WQ <br> Storage <br> Depth (ft) | Total <br> Storage <br> Depth (ft) | Required WQ <br> Detention <br> Volume (cf) | WQ Orifice <br> Diameter (in) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Pond B | 40,075 | $9 \mathrm{ft} @ 3: 1$ | 1.00 | 2.5 | 19,693 | 1.75 |
| Pond C | 8,145 | $3 \mathrm{ft} @ 3: 1$ | 0.75 | 1.0 | 1,189 | 0.50 |
| Pond E | 25,177 | $9 \mathrm{ft} \mathrm{@} \mathrm{3:1}$ | 1.00 | 2.5 | 13,800 | 1.50 |

## $7 \quad$ Water Quantity

### 7.1 Design Guidelines

The water quantity facilities were designed in accordance with Section 4.03.5(c) of the Clean Water Services Design and Construction Standards for Sanitary Sewer and Surface Water Management, December 2019. The detention standards require the post-developed runoff rates do not exceed the predeveloped runoff rates as listed in Section 4.08.6(c) and as shown in Table 7-1 below. In accordance with section 4.09 .2(c), the water quantity facility will be combined with the water quality facility.

Table 7-1 Required Release Rates

| Post-Development <br> Peak Runoff Rate | Pre-Development Peak <br> Runoff Rate Target |
| :---: | :---: |
| 2-year, 24-hour | 50\% of 2-year, 24-hour |
| 5-year, 24-hour | 5-year, 24-hour |
| 10-year, 24-hour | 10-year, 24-hour |

### 7.2 Water Quantity Facilities

Table 7-2 lists the predeveloped and the proposed design release rates generated at each detention pond. In all cases, the proposed release rates meet the criteria listed in Section 4.08.6(c).

Table 7-2 Existing and Proposed Release Rates

| Basin ID | Storm Event | Existing Flow (cfs) |  | Released Rate (cfs) |
| :---: | :---: | :---: | :---: | :---: |
|  |  | 0.88 | $>$ |  |
|  | 5 | 1.62 | $>$ | 1.21 |
|  | 10 | 2.09 | $>$ | 1.63 |
| Basin 2 | 2 | 1.79 | $>$ | 0.77 |
|  | 5 | 3.32 | $>$ | 1.32 |
|  | 10 | 4.30 | $>$ | 1.85 |

Each pond will have its own control structure with orifice and weir controls as described in Table 7-3. In each control structure, the bottom orifice was designed as the WQ orifice and sized using the criteria listed in Section 4.09.5. The second orifice and overflow weir were designed to meet the flow control standard. Max stage during the 25 -year storm event does not exceed the minimum freeboard requirement of 1.0 ' listed in Section $4.09 .2(\mathrm{c})$ (See Technical Appendix: XPSWMM Results).

Table 7-3 Control Structures

| Facility | Orifice/Weir <br> Size | Elevation (ft) |
| :---: | :---: | :---: |
| Pond B | $1.75^{\prime \prime}$ orifice | 189.00 |
|  | $3^{\prime \prime}$ orifice | 190.00 |
|  | $18^{\prime \prime}$ standpipe | 191.50 |
| Pond C | $0.5^{\prime \prime}$ orifice | 185.25 |
|  | $2^{\prime \prime}$ orifice | 185.75 |
|  | $12^{\prime \prime}$ standpipe | 186.25 |
| Pond E | 1.50 " orifice | 187.00 |
|  | $5{ }^{\prime \prime}$ orifice | 188.00 |
|  | $18^{\prime \prime}$ standpipe | 189.50 |

## 8 Downstream Analysis

### 8.1 Design Guidelines

Clean Water Services requires a review of the downstream conveyance system for sites that add greater than $12,000 \mathrm{sf}$ of new impervious area. Section $2.04 .2 \mathrm{~m} .4(\mathrm{~b})$ requires the downstream analysis meet the following standard:

- The analysis shall follow the conveyance system to the Point of Discharge and extend downstream for $1 / 4$ mile from the Point of Discharge, which is the Receiving Reach

The project is classified as hydromodification category 3 .

### 8.2 Hydraulic Analysis

The proposed project will discharge at two locations, one at the northwest corner of the site, and the other at the northeast corner of the site.

West Outlet: This basin includes the on-site Lot 1, Lot 2, and Lot 3 basins, and developed industrial properties north of SW Tualatin-Sherwood Rd. Treated and detained runoff exits the site to the northwest through an existing 18 " pipe, which runs north under SW Wildrose Place and daylights in a ditch roughly $1 / 4$ mile downstream from the site.

East Outlet: This basin includes the on-site Lot 4, Lot 5, Cipole Place, and SW $124^{\text {th }}$ Ave basins, and a portion of SW Tualatin-Sherwood Hwy. Treated and detained runoff exits the site to the northeast through the existing 36" concrete pipe, which then daylights to a ditch on the north side of the road (See Technical Appendix: Figure 2).

## $8.3 \quad$ Results

The downstream analysis was performed using XPSWMM to model the 25 -year storm event over the downstream contributing basin. The proposed development on the T-S Corporate Park site does not cause any deficiencies in the existing downstream system at either outfall location. Additional detention is not required for this project.

The downstream system was analyzed to $1 / 4$ mile downstream from the site. This corresponds with the end of the 27 " storm line in Wildrose Pl where the pipe outfalls to a ditch. The only inlet to the system downstream of the proposed development is from a ditch on the north end of Tax Lot 2S128A001800, which is undeveloped. This basin contributes roughly five acres of pervious area. Results from the XPSWMM model shows the conveyance system downstream of the T-S Corporate Park site does not surcharge during the 25 -year storm event (See Technical Appendix: XPSWMM Results).

The east outlet was analyzed through the 36 " culvert downstream of the wetland where both detention ponds outlet to. Results from the XPSWMM model shows the culvert has sufficient capacity to handle runoff from the site under proposed conditions during the $25-\mathrm{yr}$ storm event.

## 9 Summary

The proposed stormwater management design follows Clean Water Services Design and Construction Standards for Sanitary Sewer and Surface Water Management, December 2019.
On-site water quality treatment and flow control will be provided by proposed extended dry basin LIDA facilities. The proposed private conveyance system will be designed using the 25 -year storm event in the final drainage report. Treated and detained runoff will exit the site and connect to the existing storm systems in SW Tualatin-Sherwood Rd.
This project will meet the intent of the standards set forth by Clean Water Services.

## Technical Appendix

$>$ Figure 1 - Existing Basin Areas
$>$ Figure 2 - Proposed Basin Areas
> Hydrologic Soils Map - Washington County
$>$ Table 2-2a - Runoff Curve Numbers
$>$ Time of Concentration
> XPSWMM Results

- Schematic
- Basin 1
- Basin 2
- Pond Stage Graphs
- Flow Control Graphs
- Downstream Analysis
$>$ SW $124^{\text {th }}$ Ave \& SW Tualatin-Sherwood Rd Preliminary Geotechnical Report by GeoDesign dated February 6, 2018
$>$ SW $124^{\text {th }}$ Ave \& SW Tualatin-Sherwood Rd Geotechnical Data Memorandum by GeoDesign dated December 23, 2019





## MAP LEGEND



## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.
Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)
Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.
This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.
Soil Survey Area: Washington County, Oregon
Survey Area Data: Version 14, Sep 16, 2016
Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jul 8, 2010—Aug 23 2014

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident

## Hydrologic Soil Group

| Hydrologic Soil Group— Summary by Map Unit — Washington County, Oregon (OR067) |  |  |  |  |
| :--- | :--- | :--- | ---: | ---: |
| Map unit symbol | Map unit name | Rating | Acres in AOI | Percent of AOI |
| 1 | Aloha silt loam | C/D | 16.0 | $36.5 \%$ |
| 22 | Huberly silt loam | C/D | 2.3 | $5.1 \%$ |
| $37 C$ | Quatama loam, 7 to 12 <br> percent slopes | C | 13.5 | $30.8 \%$ |
| $38 C$ | Saum silt loam, 7 to 12 <br> percent slopes | C | 0.5 | $1.1 \%$ |
| 47 D | Xerochrepts-Rock <br> outcrop complex | D | 11.6 | $26.5 \%$ |
| Totals for Area of Interest | $\mathbf{4 3 . 9}$ | $\mathbf{1 0 0 . 0 \%}$ |  |  |

## Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

## Rating Options

Aggregation Method: Dominant Condition
Component Percent Cutoff: None Specified
Tie-break Rule: Higher

## Chapter 2

## Estimating Runoff

Table 2-2c Runoff curve numbers for other agricultural lands $1 /$

| Cover description | Hydrologic condition | Curve numbers for hydrologic soil group |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | A | B | C | D |
| Pasture, grassland, or range-continuous forage for grazing. ${ }^{2 /}$ | Poor | 68 | 79 | 86 | 89 |
|  | Fair | 49 | 69 | 79 | 84 |
|  | Good | 39 | 61 | 74 | 80 |
| Meadow-continuous grass, protected from grazing and generally mowed for hay. | - | 30 | 58 | 71 | 78 |
| Brush—brush-weed-grass mixture with brush the major element. ${ }^{3 /}$ | Poor | 48 | 67 | 77 | 83 |
|  | Fair | 35 | 56 | 70 | 77 |
|  | Good | 304 | 48 | 65 | 73 |
| Woods-grass combination (orchard or tree farm). $5 /$ | Poor | 57 | 73 | 82 | 86 |
|  | Fair | 43 | 65 | 76 | 82 |
|  | Good | 32 | 58 | 72 | 79 |
| Woods. ${ }^{6 /}$ | Poor | 45 | 66 | 77 | 83 |
|  | Fair | 36 | 60 | 73 | 79 |
|  | Good | $30{ }^{4}$ | 55 | 70 | 77 |
| Farmsteads-buildings, lanes, driveways, and surrounding lots. | - | 59 | 74 | 82 | 86 |
| 1 Average runoff condition, and $\mathrm{I}_{\mathrm{a}}=0.2 \mathrm{~S}$. |  |  |  |  |  |
| 2 Poor: <50\%) ground cover or heavily grazed with |  |  |  |  |  |
| Fair: 50 to $75 \%$ ground cover and not heavily gra |  |  |  |  |  |
| Good: > 75\% ground cover and lightly or only occ |  |  |  |  |  |
| 3 Poor: < $50 \%$ ground cover. |  |  |  |  |  |
| Fair: 50 to $75 \%$ ground cover. |  |  |  |  |  |
| Good: >75\% ground cover. |  |  |  |  |  |
| 4 Actual curve number is less than 30 ; use $\mathrm{CN}=30$ for runoff computations. |  |  |  |  |  |
| 5 CN's shown were computed for areas with $50 \%$ woods and $50 \%$ grass (pasture) cover. Other combinations of conditions may be computer from the CN's for woods and pasture. |  |  |  |  |  |
| 6 Poor: Forest litter, small trees, and brush are destroyed by heavy grazing or regular burning,Fair: Woods are grazed but not burned, and some forest litter covers the soil.Good: Woods are protected from grazing, and litter and brush adequately cover the soil. |  |  |  |  |  |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |

## Chapter 2

Estimating Runoff
Technical Release 55
Urban Hydrology for Small Watersheds

Table 2-2a
Runoff curve numbers for urban areas $1 /$


## Time of Concentration

| SUBJECT | Time of Concentration - Basin 1 |  |  |  |  |
| :--- | :---: | :--- | :--- | :--- | :--- |
| PROJECT NO. | 14347 | BY | MSG | DATE | 1/7/2020 |


|  | T-S Corporate Park |  |
| :---: | :---: | :---: |
| SHEET FLOW |  |  |
| INPUT | VALUE |  |
| Surface Description | Type | 6 |
| Surface Description | Grass (dense) |  |
| Manning's "n" | 0.24 |  |
| Flow Length, L (<300 ft) | 300 | ft |
| 2-Yr 24 Hour Rainfall, $\mathrm{P}_{2}$ | 2.5 | in |
| Land Slope, s | 0.064 | $\mathrm{ft} / \mathrm{ft}$ |
| OUTPUT |  |  |
| Travel Time | 0.41 | hr |
| SHALLOW CONCENTRATED FLOW |  |  |
| INPUT | VALUE |  |
| Surface Description | Unpaved |  |
| Flow Length, L | 400 | ft |
| Watercourse Slope*, s | 0.04 | $\mathrm{ft} / \mathrm{ft}$ |
| OUTPUT |  |  |
| Average Velocity, V | 3.23 | $\mathrm{ft} / \mathrm{s}$ |
| Travel Time | 0.034 | hr |
| CHANNEL FLOW |  |  |
| INPUT | VALUE |  |
| Cross Sectional Flow Area, a | 0 | $\mathrm{ft}^{2}$ |
| Wetted Perimeter, $\mathrm{P}_{\mathrm{w}}$ | 0 | ft |
| Channel Slope, s | 0 | ft/ft |
| Manning's "n" | 0.013 |  |
| Flow Length, L | 0 | ft |
| OUTPUT |  |  |
| Average Velocity | 0.00 | $\mathrm{ft} / \mathrm{s}$ |
| Hydraulic Radius, $\mathrm{r}=\mathrm{a} / \mathrm{P}_{\mathrm{w}}$ | 0.00 | ft |
| Travel Time | 0.00 | hr |
| Watershed or Subarea $\mathrm{T}_{\mathrm{c}}=$ | 0.44 | hr |
| Watershed or Subarea $\mathrm{T}_{\mathrm{c}}=$ | 26 | minutes |

## Time of Concentration

| SUBJECT | Time of Concentration - Basin 2 |  |  |  |  |
| :--- | :---: | :--- | :--- | :--- | :--- |
| PROJECT NO. | 14347 | BY | MSG | DATE | 1/7/2020 |


|  | T-S Corporate Park |  |
| :---: | :---: | :---: |
| SHEET FLOW |  |  |
| INPUT | VALUE |  |
| Surface Description | Type | 6 |
| Surface Description | Grass (dense) |  |
| Manning's "n" | 0.24 |  |
| Flow Length, L (<300 ft) | 300 | ft |
| 2-Yr 24 Hour Rainfall, $\mathrm{P}_{2}$ | 2.5 | in |
| Land Slope, s | 0.064 | $\mathrm{ft} / \mathrm{ft}$ |
| OUTPUT |  |  |
| Travel Time | 0.41 | hr |
| SHALLOW CONCENTRATED FLOW |  |  |
| INPUT | VALUE |  |
| Surface Description | Unpaved |  |
| Flow Length, L | 1300 | ft |
| Watercourse Slope*, s | 0.04 | $\mathrm{ft} / \mathrm{ft}$ |
| OUTPUT |  |  |
| Average Velocity, V | 3.23 | $\mathrm{ft} / \mathrm{s}$ |
| Travel Time | 0.112 | hr |
| CHANNEL FLOW |  |  |
| INPUT | VALUE |  |
| Cross Sectional Flow Area, a | 0 | $\mathrm{ft}^{2}$ |
| Wetted Perimeter, $\mathrm{P}_{\mathrm{w}}$ | 0 | ft |
| Channel Slope, s | 0 | ft/ft |
| Manning's "n" | 0.013 |  |
| Flow Length, L | 0 | ft |
| OUTPUT |  |  |
| Average Velocity | 0.00 | $\mathrm{ft} / \mathrm{s}$ |
| Hydraulic Radius, $\mathrm{r}=\mathrm{a} / \mathrm{P}_{\mathrm{w}}$ | 0.00 | ft |
| Travel Time | 0.00 | hr |
| Watershed or Subarea $\mathrm{T}_{\mathrm{c}}=$ | 0.52 | hr |
| Watershed or Subarea $\mathrm{T}_{\mathrm{c}}=$ | 31 | minutes |

## XPSWMM Results: T-S Corporate Park

## Schematic Layout:



## XPSWMM Results: T-S Corporate Park

Basin 1 Predeveloped Area: 10.28 ac pervious
Basin 1 Predeveloped Hydrograph


Basin 1 Post Developed Area: 15.07 ac impervious; 2.88 ac pervious
Contributing Basins: Lot 1, Lot 2, and Lot 3


## XPSWMM Results: T-S Corporate Park

## Tract B Detention System Sizing:

Pond Bottom Area: 40,075 SF
WQ Orifice: 1.75 " diameter at pond bottom
2 -yr Orifice: 3 " diameter at 1.0 ' above pond bottom
Bypass Standpipe: $18^{\prime \prime}$ diameter at $2.5^{\prime}$ above pond bottom
Tract B Pond Stage


Tract B Pond Flow Control:


## XPSWMM Results: T-S Corporate Park

Basin 2 Predeveloped Area: 21.74 ac pervious
Basin 2 Predeveloped Hydrograph


Basin 2 Post Developed Area: 12.60 ac impervious; 2.71 ac pervious
Contributing Basins: Lot 4, Lot 5, SW Cipole Pl, and SW $124^{\text {th }}$ Ave


## XPSWMM Results: T-S Corporate Park

## Tract C Detention System Sizing:

Pond Bottom Area: 8,145 SF
WQ Orifice: $0.5 "$ diameter at pond bottom
2 -yr Orifice: 2 " diameter at 0.5 ' above pond bottom
Bypass Standpipe: 12 " diameter at $1.0^{\prime}$ above pond bottom
Tract C Pond Stage


## Tract C Pond Flow Control:



## XPSWMM Results: T-S Corporate Park

## Tract E Detention System Sizing:

Pond Bottom Area: 25,177 SF
WQ Orifice: 1.50 " diameter at pond bottom
2-yr Orifice: $5^{\prime \prime}$ diameter at $1.0^{\prime}$ above pond bottom
Bypass Standpipe: $18^{\prime \prime}$ diameter at $2.5^{\prime}$ above pond bottom
Tract E Pond Stage


## Tract E Pond Flow Control:



## XPSWMM Results: T-S Corporate Park

## Downstream Analysis:

Schematic:


## XPSWMM Results: T-S Corporate Park

West Outlet Pipe Network during 25-yr event


East Outlet Pipe Network during $25-\mathrm{yr}$ event


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Attention: Kirk Olsen

# Preliminary Geotechnical Engineering Services <br> Orr Property 

SW $124^{\text {th }}$ Avenue and SW Tualatin-Sherwood Road
Washington County, Oregon
GeoDesign Project: TrammellCr-74-01

## INTRODUCTION

This report presents the results of our preliminary geotechnical engineering services for the Orr Property project located in Washington County, Oregon. This report has been prepared in general accordance with our revised proposal dated September 19, 2018. The approximately 47.3 -acre site is located south of the intersection at SW $124^{\text {th }}$ Avenue and SW Tualatin-Sherwood Road. The location of and existing conditions surrounding the site are shown on Figure 1. The boundaries of the site, topography, and location of site explorations are shown on Figure 2.

Based on conceptual plans provided by DOWL, the proposed project will consist of six buildings with an aggregate footprint of 458,155 square feet. Several options have been considered in the past. The options generally involve two approaches: (1) complete the development with a relative consistent grade across the site (reference as the one-tier approach) and (2) two benches (referenced as the two-tier approach). We understand that the current focus is on the one-tier approach, with an estimated finished rough grade of elevation 210 feet (excluding utility cuts). Retaining walls around the site perimeter and paved access roads and parking areas will be required depending on the development plans adopted.

We understand the buildings will be one story and likely concrete tilt-up structures. Foundation loads were not available at the time of this report; however, we have assumed maximum column loads will be less than 150 kips and maximum wall loads will be less than 4.5 kips per linear foot. We understand the distributed slab live load is unknown at this time; however, 350 pounds per square foot is assumed and will be confirmed later by the proposed end user.

## PURPOSE AND SCOPE

The purpose of our geotechnical services was to develop preliminary geotechnical recommendations for planning purposes and preliminary cost estimating of the proposed development. The specific scope of our services is summarized as follows:

- Reviewed readily available published geologic data and our in-house files for existing information on subsurface conditions in the site vicinity.
- Reviewed reports of previous geotechnical studies completed at and near the site.
- Coordinated and managed the field investigation, including utility locates and scheduling subcontractors and GeoDesign field staff.
- Completed two test pits to depths of 2.5 and 5.0 feet below ground surface (BGS). The test pits were terminated at final depths when practical refusal was encountered.
- Classified the materials encountered in the explorations and maintained a detailed log of each exploration.
- Observed groundwater conditions in the explorations.
- Conducted in situ seismic refraction surveying at the site to measure P-wave velocity of the geologic units. We performed 22 refraction lines, which comprise 5 subsurface profiles.
- Analyzed the collected seismic refraction data and interpreted the P-wave velocities to estimate bedrock depth and potential means of rock excavation.
- Prepared this preliminary geotechnical report summarizing the findings of the abovereference work.

This preliminary report will be followed by a design-level geotechnical report following the development of the site layout, grading plan, and estimated foundation loads.

## BACKGROUND

GeoDesign has completed several projects in the area, including a 2014 geotechnical report (GeoDesign, 2014) for the SW $124^{\text {th }}$ Avenue extension that runs along the eastern edge of the site boundary (construction recently completed). We also completed several phases for the Koch Corporate Center located at SW $115^{\text {th }}$ Avenue and SW Boones Ferry Road, approximately 3,000 feet east of the site. In addition, you provided us with a geotechnical memorandum for the site, dated April 13, 2016, which included subsurface information from 48 borings completed in March 2016 (GRI, 2016). Our recent explorations are presented in Attachment A, our seismic refraction surveying results are presented in Attachment B, the 2016 GRI memorandum is presented in Attachment C, and excerpts from the 2014 GeoDesign report are presented in Attachment D.

## SITE CONDITIONS

## SURFACE CONDITIONS

The proposed site is bordered by SW Tualatin-Sherwood Road to the north, the extension to SW $124^{\text {th }}$ Avenue to the east, municipal property to the west, and undeveloped property to the south. The property immediately south of the site is currently being developed as a water treatment plant by the Willamette Water Supply Program.

An existing house with several small outbuildings are present in the north-central portion of the site. The remainder of the site is undeveloped. The northeast half of the site slopes gently to the northeast and is currently covered with tall grass and brush and was likely used for agricultural pasture. A small drainage ravine with small- to medium-sized trees and heavy brush bisects the site and trends from south to northeast. The ravine forms steep slopes along the southern portion of the site and widens to the northeast into a pasture area.

The southwest portion of the site is currently covered with small- to medium-sized trees and heavy brush with a few trails cleared through the vegetation. The topography rises steeply to the south from the edge of the tree line and flattens to form a low ridge trending from the drainage west to the site boundary. The ground surface contains isolated areas of scattered boulders covered by vegetation and thin soil cover. The shallow ridge contains rough, rocky ground with boulder patches and evidence of shallow bedrock with little-to-no soil cover. Boulders up to 5 feet in diameter are prevalent across the ground surface in this area. The approximate extent of near-surface (or surface) boulders and bedrock is shown on Figure 2.

## GEOLOGIC SETTING

## Regional Setting

The site is located in the Tualatin Basin physiographic province, which is a northwest-southeast trending, pull-apart sub-basin of the Willamette Valley (Wilson, 1998). The Tualatin Basin is separated from adjacent sub-basins of the Willamette Valley by slightly folded and faulted basalt flows of the Columbia River Basalt Group (CRBG), which form topographic divides between adjacent basins (Popowski, 1997). The Coast Range and Chehalem Mountains bound the Tualatin Basin to the west and south, respectively, and the Tualatin Mountains (Portland Hills) bound the Portland Basin to the north and east. The region has undergone large-scale and localized tectonic activity, which has contributed to form the hills and valleys in the Willamette Valley.

## Site Geology

The generalized geologic profile of the site consists of recent alluvium, catastrophic Missoula flood deposits, and basalt bedrock of the CRBG. The mapped geologic units are generally composed of unconsolidated sediments derived from transport and deposition processes and from in-place weathering of volcanic bedrock. The CRBG underlies the sedimentary deposits along the proposed alignment and is considered the basement material for the site (Burns et al., 1997; Schlicker and Deacon, 1967).

The following sections describe the specific geologic units that are mapped at the site and were also described in subsurface explorations conducted by others on the site.

## Recent Alluvium

Holocene alluvium consists of unconsolidated gravel, sand, silt, and clay soil deposited in the last 10,000 years along stream and river drainages and is found within the site vicinity in the Tualatin Valley and along Rock and Coffee Lake creeks.

## Missoula Flood Deposits

The recent alluvium is underlain by Pleistocene Age (15,500 to 13,000 years before present) catastrophic Missoula flood deposits, which consist of poorly consolidated, fine- to coarsegrained sand, silt, and clay. The Missoula flood deposits resulted from a series of catastrophic late Pleistocene glacial outburst floods. During this time interval, enormous floods would periodically flow across eastern Washington and down the Columbia River Valley caused by failures of a glacial ice dam that impounded a large lake located in southwestern Montana (Lake Missoula). Floodwater would inundate the Willamette Valley and Tualatin Basin, leaving deposits of gravel, sand, and silt to elevations ranging from 250 to 400 feet.

In the general vicinity of the site, the Missoula flood waters were large enough to overtop the preexisting topographic divide between the Tualatin Valley and Willamette Valley near Sherwood, Oregon. High velocity flood waters carved deep channels into the CRBG in the area, creating what is known as the Tonquin Scablands (Wilson, 1998). In places, the floodwaters removed decomposed and weathered basalt and eventually down cut and entrenched into less weathered material. Evidence of numerous scoured bedrock channels near the site are identifiable using LiDAR data.

Based on subsurface explorations located in the site vicinity, fine sand and silt (fine facies) of the Missoula flood deposits are located in the pasture areas south of SW Tualatin-Sherwood Road. The flood deposits are generally thin and lap onto the weathered surface of the CRBG, which occupies higher elevations at the site.

## CRBG

Underlying the alluvium and flood deposits is the middle Miocene Age ( 20 million to 10 million years before present) CRBG. The CRBG represents the oldest geologic unit encountered at the site, which is exposed in outcrops and quarry excavations on the site and forms many of the topographic highlands within the Tualatin Valley (Wilson, 1998). The CRBG is up to 1,000 feet thick within the Tualatin Valley (Schlicker and Deacon, 1967) with individual flows ranging between 10 to 100 feet thick. The CRBG is composed of a series of basalt flows erupted from linear vent systems in southeastern Washington that flowed down the course of the ancestral Columbia River until reaching the Pacific Ocean. Some of these lava flows ponded and cooled in the northern Willamette Valley, resulting in a stacked series of basalt units. Sediments deposited on the surface of an individual basalt flow would be covered by subsequent flows, resulting in a stacked sequence of basalt flows and sedimentary interbeds. These thick flows were subsequently folded and faulted by compressional tectonics in the region.

An idealized CRBG lava flow consists of two sub-units, termed the flow top and flow interior. The flow top is often a porous, vesicular zone resulting from gas bubbles trapped during rapid cooling of the lava surface. This zone is typically intensely to moderately fractured or brecciated, the result of rapid cooling, and both vesicles and fractures may be partially filled by secondary mineralization. The flow bottom is similar to the flow top, except the weathering may not be as severe. The flow interior typically consists of very dense, moderately fractured basalt with a high mechanical strength due to crystalline mineral formation resulting from slower cooling of the lava.

A hiatus between lava flow emplacements can create conditions of deep weathering of the basalt, resulting in a breakdown of the rock minerals to clay components forming a soil horizon (saprolite). The hiatus periods may have resulted in thick sections of severely weathered basalt and deposition of sedimentary interbeds between basalt flow units. Unweathered exposures of Columbia River basalt flow interiors are excellent sources of crushed aggregate. A number of active quarries in the CRBG are located north and east of the study area (Tigard Sand and Gravel Quarry and Knife River-Coffee Lake Quarry). Where the CRBG was exposed for an extensive period of time, the rock is decomposed to form a thick, lateritic soil consisting of clayey gravel or clayey sand containing cobbles and boulders.

## SUBSURFACE CONDITIONS

## General

The subsurface conditions are summarized based on the information from several sources. We explored subsurface conditions at the site by excavating two test pits (TP-1 and TP-2) to depths of 2.5 and 5.0 feet BGS. The trackhoe used to complete the above-referenced test pits was primarily used to clear the heavy brush along the refraction lines. The exploration locations are shown on Figure 2 and associated exploration logs are presented in Attachment A.

We completed a seismic wave refraction survey at the site. The purpose of the seismic wave refraction survey was to further characterize the extent and characteristics as well as estimate the depth and rippability of the basalt rock at the site. The location of the five subsurface profiles generated from this survey are shown on Figure 2 and a detailed summary of our seismic wave refraction survey is presented in Attachment $B$. Important aspects of the five subsurface profiles (A-A', B-B'", C-C', C''-C'", and D-D'), presented in Attachment B) generated in this study include the following:

- The shallowest velocity layer corresponds to the transition between the soil (alluvium) unit, which is interpreted to have a P-wave velocity less than 3,000 feet per second (fps).
- The subsequent layers showing increasing P-wave velocity are interpreted to represent the top of the underlying volcanic bedrock.
- The increase in P-wave velocity with depth shown on the profiles corresponds to a transition from low strength, intensely weathered, and intensely fractured bedrock to higher strength, fresh, intact bedrock.
- The deepest refracting layer corresponds to the highest velocity layer detected in the refraction survey line.

The GRI borings were drilled to depths of between 15 and 30 feet BGS using open-hole, air rotary methods to determine the depth to bedrock and to estimate the weathering and hardness of the bedrock encountered. The April 13, 2016 memorandum is presented in Attachment C and the locations of the explorations are shown on Figure 2.

GeoDesign prepared a geotechnical report for the SW 124 ${ }^{\text {th }}$ Avenue Extension Project located along the eastern edge of the project site (GeoDesign, 2014). Five borings from this report were completed adjacent to the eastern edge of the site, at the approximate location shown on Figure 2. The site plan, exploration logs, and laboratory test results from this report are presented in Attachment D.

Subsurface conditions generally consist of alluvium overlying decomposed basalt and weathered basalt to fresh basalt. The following sections provide a summary of the subsurface units encountered.

## Topsoil/Tilled Zone

A tilled zone was encountered in borings B-1, B-2, and INF-1 (GeoDesign 2014, Attachment D) that extended to approximately 12 inches BGS. This zone included a surficial topsoil layer having a thickness of approximately 6 inches and an associated root zone of approximately 3 inches. Thick stripping and topsoil layers are anticipated in the heavily vegetated areas of the site. In addition, we anticipate up to 3 - to 4 -foot-deep root wads in the treed area of the site.

## Alluvium

Alluvial deposits were generally encountered below the topsoil. These deposits extend to a depth of approximately 16 feet BGS in the northeast corner of the site and gradually decrease in thickness toward the southwest until they taper to less than 1 foot thick near rock outcroppings located in the southwest corner of the site. The shallowest velocity layer from the seismic wave refraction survey likely corresponds to the transition between the soil and weathered rock units, which is interpreted to have a P-wave velocity less than $3,000 \mathrm{fps}$. Boulders are likely in the $3,000 \mathrm{fps}$ material, as encountered at TP-1.

Alluvial deposits generally consist of soft to very stiff silt and clay with varying amounts of fine sand, but also note that very loose, silty sand was encountered at INF-1. Very loose, silty sand deposits were observed within the deeper deposits located along the eastern edge of the site boundary. Laboratory testing indicates that the alluvial deposits in the eastern portion of the site had moisture contents in the range of 27 to 36 percent and a dry density of 94 pounds per cubic foot. Atterberg limits testing indicates the alluvium generally has a low to medium plasticity.

## Decomposed Basalt

Decomposed basalt was generally encountered below the alluvium, The depth to decomposed basalt varies considerably across the site, with depths greater than 26.5 feet BGS in the northeast corner of the site, and gradually decreases toward the southwest where it tapers to at/near the ground surface toward the southwest corner of the site (as mapped on Figure 2).

The decomposed basalt generally consists of a medium dense to very dense, brown and gray, silty gravel; medium dense to dense, clayey to silty sand; and stiff, blue-gray, sandy clay. The decomposed basalt is interpreted to represent the deeply weathered surface of a lava flow of the CRBG. The decomposed basalt may have been generated through a variety of processes, including weathering of in-place basalt, weathering of flow top breccia derived from the CRBG, and erosion and deposition of CRBG material close to the original flow. The weathering of this material is variable and dependent upon the ability of surface water and groundwater to penetrate the unit and chemically break it down. The decomposition process can include highly variable amounts of relatively resistant gravel- to boulder-sized clasts in a matrix of silt and clay.

The velocity ranging from approximately 3,000 to $5,000 \mathrm{fps}$ is interpreted to represent the top of the weathered bedrock exhibiting relatively low strength, intense weathering, and intense fracturing. The fractures may have wide separations with soil filling that result in a slower P -wave velocity. This layer may also represent a transition from tightly packed boulders in a clay or silt matrix to weathered, intact rock structure.

Laboratory testing indicates that the decomposed basalt located along the eastern portion of the site had moisture contents ranging from 11 to 63 percent, with the higher moisture contents corresponding to more clay-rich soil.

## Weathered Basalt and Basalt Bedrock (Fractured and Competent)

We encountered weathered basalt and fresh basalt bedrock underlying the alluvium and decomposed basalt along the eastern edge of the site boundary (GeoDesign, 2014). The weathered basalt generally consists of very dense, brown, gray, and red-brown gravel or silty to clayey sand that is generally characterized by a significant change to hard, consistent drilling and refusal SPT blow counts. The weathered basalt is a transition zone from the overlying relatively softer, decomposed basalt to generally medium hard to hard, slightly weathered to fresh basalt observed in the continuous rock core samples.

Rock coring was completed in boring B-4 (GeoDesign 2014) where mud rotary drilling met practical refusal. In general, the rock cores consist of soft (R2) to medium hard (R3), gray, finegrained basalt. The basalt cores exhibit varying degrees of weathering from decomposed to moderately weathered, which is usually associated with position of the core within a lava flow and the degree of contact with groundwater. The basalt cores also exhibit a varying degree of fracturing and jointing, which is reflected on the log as percent rock quality designation (RQD). Rock cores with low RQD values generally exhibit an intense to very intense fracture density. Competent basalt generally has moderate to very slight fracture density and high RQD values greater than 50 percent. Typical standards for rippability are presented in Attachment E. Note that marginal rippability is encountered in basalt rock at velocities of 6,500 to 7,500 fps (standard D8 CAT with single- or multiple-shank rippers). The lowest refraction layer detected in most of the profiles was a maximum P-wave velocity of $6,000 \mathrm{fps}$; however, profile B-B' detected a hard basalt layer (greater than $7,000 \mathrm{fps}$ ) in the northwest portion of the site where we encountered shallow basalt in test pit TP-2.

## Groundwater

Groundwater was observed within the explorations across the site at depth ranging from 0.4 foot to 29.5 feet BGS, corresponding to elevations (NAVD 88) ranging from 192.1 to 246.2 feet. These groundwater observations likely reflect perched groundwater conditions. According to the estimated depth to groundwater mapping published by the U.S. Geological Survey (Snyder, 2008), the regional groundwater table is located at a depth of approximately 53.5 feet BGS, corresponding to an elevation (NAVD 88) of 150.5 feet. Perched groundwater zones are likely to occur in the upper soil at the site, particularly during extended periods of wet weather. The depth to groundwater may fluctuate in response to prolonged rainfall, seasonal changes, changes in surface topography, and other factors not observed during this study.

## CONCLUSIONS

We anticipate the following geotechnical factors will have an impact on design and construction of the proposed development:

- We completed seismic wave refraction survey along five subsurface profiles (A-A', B-B'’, C-C', C''-C'", and D-D'), as shown on Figure 2. An expanded discussion on the methodologies and results are presented in Attachment B.
- As discussed in further detail above and in Attachment B:
- The shallowest velocity layer from the seismic wave refraction survey likely corresponds to the transition between the soil and weathered rock units, which is interpreted to have a P-wave velocity less than $3,000 \mathrm{fps}$.
- The velocity ranging from approximately 3,000 to 5,000 fps is interpreted to represent the top of the weathered bedrock exhibiting relatively low strength, intense weathering, and intense fracturing. The fractures may have wide separations with soil filling that result in a slower P-wave velocity. This layer may also represent a transition from tightly packed boulders in a clay or silt matrix to weathered, intact rock structure.
- Marginal rippability basalt is generally encountered in basalt rock at velocities of 6,500 to $7,500 \mathrm{fps}$ (standard D8 CAT with single- or multiple-shank rippers). The lowest refraction layer detected in most of the profiles was a maximum P-wave velocity of 6,000 fps; however, profile B-B' detected a hard basalt layer (greater than 7,000 fps).
- Boulders may occasionally be encountered in the material with P-wave velocities less than $3,000 \mathrm{fps}$ material and are anticipated in the material above $3,000 \mathrm{fps}$.
- As discussed above, unweathered exposures of CRBG are excellent sources of crushed aggregate. A number of active quarries in the CRBG are located north and east of the study area (Tigard Sand and Gravel Quarry and Knife River-Coffee Lake Quarry). However, where the CRBG was exposed for an extensive period of time, the rock is decomposed to form a thick, lateritic soil consisting of clayey gravel or clayey sand containing cobbles and boulders. The seismic wave refraction survey suggests that both conditions should be expected.
- Tilled zones should be anticipated in the agricultural areas located in the eastern portion of the site. Tilled and topsoil zones not removed from cuts and site stripping will need to be removed or stabilized. Scarification and compaction of the tilled and topsoil zones will likely not be possible unless completed during the summer dry period. Removal and replacement of the tilled and topsoil zones with granular material or cement amendment will be necessary if stabilization through moisture conditioning is not possible.
- The fine-grained soil at the site can be sensitive to small changes in moisture content and difficult, if not impossible, to adequately compact during wet weather or when the moisture content of the soil is more than a few percent above the optimum required for compaction. The moisture content of the soil encountered at the site is above that required for compaction and drying will likely be required for use as structural fill. Accordingly, the onsite soil can typically only be placed as structural fill during dry summer months.
- Fine-grained soil present on this site is easily disturbed during the wet season. If not carefully executed, site earthwork can create extensive soft areas and significant repair costs can result. Subgrade protection will be required when the subgrade is wet.
- Cobbles and boulders are present at the surface and shallow depths below the ground surface. The presence of cobbles and boulders may make excavations difficult and will likely need pre-processing if crushing is attempted.


## LIMITATIONS

We have prepared this preliminary report for use by Trammell Crow Company and members of the design and construction teams for use in cost estimating and preliminary design. The data and report can be used for estimating purposes, but our report, conclusions, and interpretations should not be construed as a warranty of the subsurface conditions and are not applicable to other sites.

The scope of our services does not include services related to construction safety precautions, and our recommendations are not intended to direct the contractor's methods, techniques, sequences, or procedures, except as specifically described in our report for consideration in design.

Within the limitations of scope, schedule, and budget, our services have been executed in accordance with the generally accepted practices in this area at the time this report was prepared. No warranty or other conditions, express or implied, should be understood.

We appreciate the opportunity to be of continued service to you. Please call if you have questions concerning this report or if we can provide additional services.

Sincerely,

## GeoDesign, Inc.



Gregory J. Schaertl, P.E. (California)
Project Engineer


George Saunders, P.E., G.E.
Principal Engineer


EXPIRES: 6/30/20

[^26]
## REFERENCES

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FIGURES



ATTACHMENT A

## ATTACHMENT A

## FIELD EXPLORATIONS

## GENERAL

We explored subsurface conditions at the site by excavating two test pits (TP-1 and TP-2) at the approximate locations shown on Figure 2. Excavation services were provided by Dan J. Fischer Excavating, Inc. of Forest Grove, Oregon, on December 10, 2018. A member of our geology staff observed the explorations. The exploration locations were determined in the field using a Trimble hand-held differential global positioning system (GPS) unit with sub-meter accuracy and should be considered accurate to the degree implied by the methods used. The exploration logs are presented in this attachment.

## SOIL CLASSIFICATION

We collected samples of the soil encountered at representative intervals. The soil samples were classified in accordance with the "Explorations Key" (Table A-1) and "Soil Classification System" (Table A-2), which are presented in this attachment. The exploration logs indicate the depths at which the soils or their characteristics change, although the change could be gradual. If the change occurred between sample locations, the depth was interpreted. Classifications are shown on the exploration logs.


RELATIVE DENSITY - COARSE-GRAINED SOIL

| Relative Density | Standard Penetration <br> Resistance | Dames \& Moore Sampler <br> (140-pound hammer) | Dames \& Moore Sampler <br> (300-pound hammer) |
| :---: | :---: | :---: | :---: |
| Very Loose | $0-4$ | $0-11$ | $0-4$ |
| Loose | $4-10$ | $11-26$ | $4-10$ |
| Medium Dense | $10-30$ | $26-74$ | $10-30$ |
| Dense | $30-50$ | $74-120$ | $30-47$ |
| Very Dense | More than 50 | More than 120 | More than 47 |
| CONSISTENCY - FINE-GRAINED SOIL |  |  |  |




## ATTACHMENT B

## ATTACHMENT B

## SEISMIC REFRACTION SURVEY

## GENERAL

We conducted a seismic wave refraction survey by collecting seismic data along 22 refraction lines, which comprise five subsurface profiles (A-A', B-B'", C-C', C''-C'", and D-D'), as shown on Figure 2. The survey was conducted to expand and correlate with subsurface data collected from GRI's and our test pit explorations. The seismic refraction study was conducted on December $10,13,19$, and $21,2018$.

## FIELD METHODOLOGY

The field work consisted of collecting 22 P-wave refraction survey lines that comprised five subsurface profiles (A-A', B-B'", C-C', C''-C’", and D-D'), as shown on Figure 2. The location and alignment of each profile was chosen based on areas of deep cuts for the development. The refraction survey alignments were cleared of brush consisting of thick blackberry vines, down trees, limbs, poison oak, small trees, and shrubs using a trackhoe operated by Dan J. Fischer Excavating. Survey alignments were adjusted in the field to minimize vegetation removal, avoid groups of large trees, avoid rough or difficult terrain, and avoid a group of abandoned cars. The larger obstructions resulted in gaps for survey profiles B and C, as shown on Figure 2.

Field measurements were completed using a 300-foot tape. Cross section endpoints were located using a Trimble hand-held differential GPS unit with sub-meter accuracy. The surfaces for individual lines ranged from flat-lying to moderately steep slopes with elevation changes across survey alignments ranging from 7 to 37 feet. We corrected the refraction data for change in topography using the site survey elevation data from the drawings. The individual survey lines were laid out using geophone spacings of 7 and 9 feet. The survey lines used for each profile were overlapped by two geophone spacings to maintain continuity between individual refraction lines. We observed boulders and the top of basalt exposed in portions of the survey lines, indicating that the top of the bedrock varies in elevation across the site.

Compressional waves used in this refraction survey were produced by striking an approximately 1 -foot-square steel plate that was placed on the ground surface. For each refraction line, an array of 16 vertical geophones spaced 7 or 9 feet apart was placed along the profile alignment. The waveforms were recorded on a 16-channel Geode seismograph manufactured by Geometrics, Inc. Waveforms were recorded for five separate source locations. Two of the source locations were near the ends of the geophone array and three source locations were at the onequarter, mid-point, and three-quarter point of the array. Acquisition of the P -wave refraction data followed the general methods and procedures provided in ASTM D5777-00'.

## DATA REDUCTION AND INTERPRETATION

The refraction data for each survey line was reduced and subsurface depth profiles generated using the Seislmager software provided by Geometrics, Inc. The direct P-wave arrivals were manually picked from the geophone waveform display for the five source locations and were

[^27]used to generate travel time curves for each of these source positions. Slope breaks observed on the travel time curves define the direct P-wave propagating within the surface low velocity layer (LVL) of the soil unit and within the high velocity layer ( HVL ) of the underlying bedrock. The slope breaks from the travel time curves were used to generate an initial two-layer velocity model for each refraction line.

A tomographic inversion was used on the two-layer velocity model to generate a set of gradual velocity layers representing the transition of the surficial LVL to the underlying HVL interpreted from subsurface explorations to represent intact, weathered basalt. The velocity layers for each survey line model were used to construct five subsurface profiles (A-A', B-B'", C-C', C''-C'", and D-D'), which are presented in this attachment. Note:

- The shallowest velocity layer corresponds to the transition between the soil (alluvium) unit, which is interpreted to have a P-wave velocity less than $3,000 \mathrm{fps}$.
- The subsequent layers showing increasing P-wave velocity are interpreted to represent the top of the underlying volcanic bedrock.
- The increase in P-wave velocity with depth shown on the profiles corresponds to a transition from low strength, intensely weathered, and intensely fractured bedrock to higher strength, fresh, intact bedrock.
- The deepest refracting layer corresponds to the highest velocity layer detected in the refraction survey line.

The depth of detection for a survey line is based on the length of the total individual refraction line and geophone spacing. In general, the practical depth of signal return is estimated to be one-third of the total distance of the geophone spread, which is approximately 35 feet BGS and is shown as the signal limit line on the subsurface profiles. The P-wave velocities for each refraction profile are presented in Table C-1.

Table C-1. Refraction P-Wave Velocities

| Survey Profile | LVL <br> Soil Unit <br> (fps) | HVL <br> Basalt Bedrock <br> (fps) |
| :---: | :---: | :---: |
| A-A' | $<3,000$ | 3,000 to $6,000+$ |
| B-B'" | $<3,000$ | 3,000 to $7,000+$ |
| C-C' | $<3,000$ | 3,000 to $6,000+$ |
| C''-''" | $<3,000$ | 3,000 to $6,000+$ |
| D-D' | $<3,000$ | 3,000 to $6,000+$ |

In general, the P-wave velocity for the soil layer appeared to be less than $3,000 \mathrm{fps}$. The P-wave velocity ranging from approximately 3,000 to $5,000 \mathrm{fps}$ is interpreted to represent the top of the variably weathered bedrock exhibiting relatively low strength, intense weathering, and intense fracturing. The fractures may have wide separations with soil filling that result in a slower P-wave velocity. This layer may also represent a transition from tightly packed boulders in a clay or silt
matrix to weathered, intact rock structure. Refraction layers with P-wave velocities greater than 5,000 fps are interpreted to represent moderately strong, intact rock that is moderately weathered to fresh and moderately fractured to unfractured. The lowest refraction layer detected in most of the profiles appeared to exhibit a maximum P-wave velocity of $6,000 \mathrm{fps}$. However, refraction profile B-B' detected a hard basalt layer (greater than $7,000 \mathrm{fps}$ ) in the northwest portion of the site where we encountered shallow basalt in test pit TP-2.

The varying P-wave velocities observed within the volcanic rock, both vertically and horizontally, are interpreted to be caused by relative changes in rock density resulting from weathering, rock joint and fracture separation and density, relative rock hardness, flow contacts, and bedrock geometries or conditions that were not observed from the subsurface explorations.

In general, the thickness of the overlying LVL soil layer ranged from approximately less than 2 to 10 feet. The range in depth to the top of the hard basalt layer (lowest HVL detected) along each profile is shown in Table C-2. The velocity model profiles indicate that an HVL with P-wave velocities exceeding 6,000 fps underlies the soil layer generally within 5 to 7 feet BGS. Proposed earthwork activity may encounter bedrock or difficult excavation conditions depending on the depth of cut.

Table C-2. Seismic Refraction Layer Depths

| Survey Profile | LVL <br> Top of Bedrock <br> (feet BGS) | Lowest HVL <br> Intact Basalt <br> (feet BGS) |
| :---: | :---: | :---: |
| A-A' | $2-9$ | $5-11$ |
| B-B'" | $2-6$ | $5-22$ |
| C-C' | $2-8$ | $5-21$ |
| C'-','" | $2-8$ | $5-13$ |
| D-D' | $2-10$ | $7-18$ |

The depth to intact basalt observed in the GRI boring data (GRI, 2016) and depth to refusal in our test pits appear to correlate with the depth to basalt estimated from the seismic refraction. The transition from soil to weathered, intact basalt exhibits a rapid change with depth, as observed in the refraction profiles. The top of the intact, weathered basalt (lowest HVL) depicted in the refraction surveys generally appeared as a gradual increase in P-wave velocity over a distance of 3 to 16 feet below the soil layer.

## RIPPABILITY

Rippability is the ease with which soil or rock can be mechanically excavated. Rippability is influenced by numerous rock parameters, including uniaxial compressive strength, degree of weathering, abrasiveness, and spacing of discontinuities. Seismic refraction is commonly used to indirectly determine the degree of rippability. Rock that is too hard to be ripped is typically fragmented with explosives and excavated.

Basalt rock is considered as potentially rippable by a Caterpillar model D-8 bulldozer with a single- or double-ripping shank when the P-wave velocity is less than approximately 6,500 to $7,000 \mathrm{fps}$ and by a Caterpillar model D-9 bulldozer with a single- or double-ripping shank when the P-wave velocity is less than approximately 7,000 to $7,500 \mathrm{fps}^{2}$. This relationship is only appropriate if the bulldozer is operating in an open, unrestrained area where boulders that are too hard to rip can be isolated and removed by other means. P-wave velocity relationships are typically not valid for other types of equipment and situations, such as an excavator digging a utility trench. In general, the seismic refraction method used for this project does not discriminate between rippable weathered, intensely fractured rock and boulders located in the decomposed basalt. Excavations with small or confined limits, such as utility trenches or footings may encounter boulders requiring limited rock excavation methods or over-excavation.

Rock rippability depends on both the mechanical strength of the rock material and the degree of fracturing of the rock mass. The P-wave velocity of a rock provides an indirect measure of both the rock material strength and the degree of fracturing. The results of this geophysical survey can be used to evaluate where difficult excavation conditions exist and whether additional geophysical or direct subsurface investigation is warranted.

[^28]TrammellCr-74-01-FB1_B5.docx Print Date: 2/6/19

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TrammellCr-74-01-FB1_B5.docx Print Date: 2/6/19


## ATTACHMENT C

## ATTACHMENT C

## EXPLORATIONS BY OTHERS

The site map with depth to bedrock data completed for the site by GRI (GRI, 2016) is presented in this attachment.

From: Michael Reed, PE, GE; Brian Bayne, PE; and Seth Reddy, PhD, EIT
Re: Preliminary Subsurface Investigation for Pre-purchase Due Diligence 90-Acre Site
12900 SW Tualatin-Sherwood Road Sherwood, Oregon

At your request, GRI has conducted a preliminary subsurface investigation as part of a pre-purchase due diligence evaluation for a 90 -acre site at 12900 SW Tualatin-Sherwood Road in Sherwood, Oregon. Our services included a review of existing geotechnical data for the area and limited subsurface explorations. This memorandum describes the work accomplished and provides a site plan with table showing approximate depths to basalt and groundwater encountered in the borings.

## PROJECT DESCRIPTION

Ken Leahy Construction, Inc. (Leahy) is considering acquiring the 90 -acre site for development into multiple buildable lots for commercial development. Preliminary site grades for the lots are unavailable.

## SITE DESCRIPTION

## Topography and Surface Conditions

The existing ground surface elevation varies significantly across the site from about elevation 192 ft (North American Vertical Datum of 1988 [NAVD88]) on the northern edge of the site to about elevation 280 ft near the southeast corner of the site. An existing farmhouse and several out-buildings are located near the north edge of the property. The northeast portion of the site is covered with grass in an area that was previously used for agricultural purposes. The remainder of the site is heavily wooded with mature trees and shrubs. Basalt outcroppings were observed most predominately near the northwest quarter and middle of site, but are present throughout the heavily wooded areas. Cobbles and boulders are present at the ground surface in the wooded areas.

## Geology

This site is at the northern edge of an area known as the Tonquin Scablands, where Pleistocene-age catastrophic floods from the Columbia River scoured away the soil, leaving rock exposed at the ground surface or covered by only a thin layer of soil. Portions of the area may be mantled with a thin layer of Pleistocene-age lacustrine (floodplain) deposits of the Columbia River, consisting of interlayered sand, silt, and gravel. Below the thin zone of surficial soil, the site is underlain by Columbia River Basalt, a thick
sequence of dark gray, basalt lava flows of mid-Miocene age. Based on our experience with other nearby projects and our observations while onsite, we anticipate basalt is present at relatively shallow depths.

## SUBSURFACE CONDITIONS

## General

Subsurface materials and conditions were investigated on a preliminary basis on March 28 and 29, 2016, with 48 borings, designated B-1 through B-48. The borings were advanced to depths of 15 to 30 ft at the approximate locations shown on the Site Plan, Figure 1. The borings were completed by McCallum Rock Drilling, Inc. of Albany, Oregon using a track-mounted FRD Furukawa HCR 900-ES II drilling rig. The rock drilling rig used open-hole air-rotary impact drilling methods typically used for production blasting in aggregate quarries and large rock cuts. The driller was contracted directly by Leahy, and the exploration locations and depths of the borings were selected by a representative of Leahy. The drill cuttings were diverted to a cyclone to allow collection of disturbed samples of soil and rock. GRI was on site on a fulltime basis during drilling and recorded the GPS coordinates and depth to basalt at each boring location. Disturbed soil and rock cuttings were collected as bulk samples removed by hand from the cyclone on an intermittent basis, saved in airtight jars and bags, and returned to our lab for further examination. The depth to rock and estimates of rock weathering and hardness were approximated based on rate of advancement of the drill, color of the drill cuttings, and evaluation of the cuttings samples collected. Following drilling, each hole was left open to allow measurements of static groundwater.

## Subsurface Conditions

Based on the disturbed soil cuttings collected during drilling and our observations while onsite, the site is typically mantled with silt or sand soils with varying percentages of clay. We anticipate fill soils may be encountered locally. Basalt was encountered at the ground surface in borings B-28, B-29, B-31, B-33, $\mathrm{B}-35$, and $\mathrm{B}-39$ and beneath the silt and sand soils at depths ranging from 0.5 to 15 ft in borings $\mathrm{B}-5$ through B-8, B-12 through B-19, B-22, B-23, B-30, B-32, B-34, B-36 through B-38, and B-40 through B-48. Basalt was not encountered in borings B-1 through B-4, B-9 through B-11, B-20, B-21, and B-24 through B27. The approximate depths and relatively hardness of the basalt is presented in a table on Figure 1. Terms used to describe the soil and rock are defined on Tables 1 and 2. For the purpose of discussion, the basalt has been grouped into two categories: very soft (R1) to medium hard (R3), moderately weathered to predominantly decomposed basalt, and soft (R2) to hard (R4), slightly weathered to fresh basalt.

Based on the rate of advancement of the drill rig, color of the drill cuttings, and subsequent evaluation of the cutting samples collected during drilling, the surface of the basalt is typically very soft (R1) to soft (R2), moderately weathered to predominately decomposed, and likely contains some medium hard (R3) zones. Drill cuttings in the moderately weathered to predominately decomposed basalt are typically red-brown to brown and contain few angular pieces of basalt. Borings B-5, B-6, B-18, B-19, B-22, and B-23 were terminated in the moderately weathered to predominantly decomposed basalt at depths ranging from 15 to 20 ft . Zones of moderately weathered to predominantly decomposed basalt were encountered below fresh to slightly weathered basalt at depths of 6 to 21 ft in borings $\mathrm{B}-14, \mathrm{~B}-28, \mathrm{~B}-46$, and $\mathrm{B}-47$.

Fresh to slightly weathered, soft (R2) to medium hard (R3) basalt was generally encountered beneath the more weathered basalt at depths ranging from 9 to 18 ft and likely contains zones of hard (R4) basalt. Drill cuttings in the basalt were typically light gray silt- and sand-sized pieces with frequent small fine gravelsized rock fragments.

Following completion of the drilling, the holes were left open to allow measurements of depth to groundwater. Groundwater depths are provided on Figure 1 and vary considerably across the site. All groundwater measurements were taken in the afternoon of March 29 and indicate perched groundwater conditions.

## EXCAVATION METHODS

Final site grading and depth of utilities for the proposed development are currently unknown. We anticipate conventional excavation equipment can be used to excavate the silty and sandy soils overlying the basalt. We anticipate some of the near-surface very soft to soft ( R 1 to R2) basalt can be excavated with a sufficiently large track-mounted excavator equipped with a rock excavation bucket and rock teeth, or by ripping with a CAT D8 bulldozer, or equivalent, equipped with a single-shank ripper. It should be noted that although the slightly weathered to predominately decomposed basalt is typically very soft (R1) to soft (R2), zones of medium hard (R3) basalt are likely present within this unit. Rock excavation methods, such as hydraulic splitters and chippers or pneumatic hammers, may be needed to excavate the rock in these areas of medium hard (R3) rock. We anticipate the fresh to slightly weathered, soft (R2) to medium hard (R3) basalt with zones of hard (R4) basalt will likely require blasting or other rock excavation methods to excavate.

Rock hardness designations provided in this memorandum are based on visual observation of drilling spoils and the rate of drilling. If significant excavation into the basalt is planned, coring of the basalt should be performed to obtain samples for completion of compressive strength testing and to evaluate fracture spacing.

## OTHER CONSIDERATIONS

Properties to the south of this site have previously been quarried for aggregate production. We anticipate that some of the rock removed during site grading could be crushed for aggregate. In general, the quality of aggregate decreases as weathering of the source rock increases. The proportion of clay, silt, and sand produced during crushing for aggregate will typically increase as the weathering in the source rock increases. Reduced material strength and chemical changes in the rock mineralogy can result in decreased durability of aggregates produced from weathered rock. In general, a rock mass that is classified as moderately weathered using the relative rock weathering scale on Table 2 can be considered marginal to poor for aggregate production. Rock weathered to the range of predominantly decomposed or decomposed is unsuitable for aggregate production.

## LIMITATIONS

This preliminary memorandum has been prepared to aid in the pre-purchase evaluation of the subject property described herein. The findings, conclusions, and recommendations presented in this memorandum are based on our interpretation of the information obtained through the assessment procedures described in this memorandum, based on 48 widely spaced borings advanced at the locations shown on Figure 1. It should be noted that there are significant limitations associated with using air-rotary percussion methods to characterize subsurface conditions. While slower than the air rotary drill, conventional geotechnical drilling methods, especially rock coring, would more accurately characterize rock hardness, fracture spacing, and rock weathering. Due to the method of drilling used for this preliminary evaluation, the estimated thickness, degree of weathering, and hardness of the rock at each exploration should be considered approximate.

In the performance of subsurface investigations, specific information is obtained at specific times, and variations in subsurface conditions may exist across the site. This preliminary report does not reflect any variations that may occur between exploration locations. The nature and extent of variation may not become evident until site development is underway. Consequently, any material volume estimates developed using the information provided in this memorandum should be considered approximations intended for planning purposes only.

The information presented herein is preliminary and provides our general conclusions regarding the depth to rock and excavation methods with respect to the observed site conditions. This information is intended for preliminary planning purposes. Additional geotechnical investigation should be completed as specific projects are developed for specific locations on the property.

Please contact the undersigned if you have any questions.
Submitted for GRI,
Michael W. Reed, PE, GE
Principal

Brian J. Bayne, PE
Seth C. Reddy, PhD, EIT
Senior Engineer
Staff Engineer

5838-PRELIM EVAL MEMO

## Table 1: GUIDELINES FOR CLASSIFICATION OF SOIL

## Description of Relative Density for Granular Soil

| Relative Density | Standard Penetration Resistance <br> $(\mathbf{N}$-values) blows per foot |
| :---: | :---: |
| Very Loose | $0-4$ |
| Loose | $4-10$ |
| Medium Dense | $10-30$ |
| Dense | $30-50$ |
| Very Dense | over 50 |


| Description of Consistency for Fine-Grained (Cohesive) Soils |  |  |
| :---: | :---: | :---: |
|  | Standard Penetration <br> Resistance (N-values) <br> blows per foot | Torvane or <br> Undrained Shear <br> Strength, tsf |
| Consistency | $0-2$ | less than 0.125 |
| Very Soft | $2-4$ | $0.125-0.25$ |
| Soft | $4-8$ | $0.25-0.50$ |
| Medium Stiff | $8-15$ | $0.50-1.0$ |
| Stiff | $15-30$ | $1.0-2.0$ |
| Very Stiff | over 30 | over 2.0 |


| Grain-Size Classification | Modifier for Subclassification |  |  |
| :---: | :---: | :---: | :---: |
| Boulders: $>12 \text { in. }$ | Adjective | Primary Constituent SAND or GRAVEL | Primary Constituent SILT or CLAY |
| Cobbles: |  | Percentage of Other Material (by weight) |  |
| 3-12 in. | trace: | 5-15 (sand, gravel) | 5-15 (sand, gravel) |
| Gravel: | some: | 15-30 (sand, gravel) | 15-30 (sand, gravel) |
| 1/4-3/4 in. (fine) <br> 3/4-3 in. (coarse) | sandy, gravelly: | 30-50 (sand, gravel) | 30-50 (sand, gravel) |
|  | trace: <br> some: <br> silty, clayey: | <5 (silt, clay) | Relationship of clay and silt determined by plasticity index test |
| No. 200 - No. 40 sieve (fine) <br> No. 40 - No. 10 sieve (medium) |  | 5-12 (silt, clay) |  |
| No. 10 - No. 4 sieve (coarse) |  | 12-50 (silt, clay) |  |
| Silt/Clay: <br> pass No. 200 sieve |  |  |  |

## Table 2: GUIDELINES FOR CLASSIFICATION OF ROCK

## RELATIVE ROCK WEATHERING SCALE

| Term | Field Identification |
| :---: | :--- |
| Fresh | Crystals are bright. Discontinuities may show some minor surface staining. No discoloration in rock fabric. |
| Slightly | Rock mass is generally fresh. Discontinuities are stained and may contain clay. Some discoloration in rock <br> fabric. Decomposition extends up to 1 in. into rock. |
| Moderately | Rock mass is decomposed $50 \%$ or less. Significant portions of rock show discoloration and weathering <br> effects. Crystals are dull and show visible chemical alteration. Discontinuities are stained and may contain <br> secondary mineral deposits. |
| Weathered |  |

## RELATIVE ROCK HARDNESS SCALE

| Term | Hardness <br> Designation | Field Identification | Approximate Unconfined <br> Compressive Strength |
| :---: | :---: | :---: | :---: |
| Extremely <br> Soft | R0 | Can be indented with difficulty by thumbnail. May be <br> moldable or friable with finger pressure. | $<100 \mathrm{psi}$ |
| Sory | R1 | Crumbles under firm blows with point of a geology pick. <br> Can be peeled by a pocket knife and scratched with <br> fingernail. | $100-1,000 \mathrm{psi}$ |
| Soft | R2 | Can be peeled by a pocket knife with difficulty. Cannot be <br> scratched with fingernail. Shallow indentation made by firm <br> blow of geology pick. | $1,000-4,000 \mathrm{psi}$ |
| Medium <br> Hard | R3 | Can be scratched by knife or pick. Specimen can be <br> fractured with a single firm blow of hammer/geology pick. <br> Hard | R4 |
| Can be scratched with knife or pick only with difficulty. <br> Several hard hammer blows required to fracture specimen. <br> Cannot be scratched by knife or sharp pick. Specimen <br> Vequires many blows of hammer to fracture or chip. <br> Hammer rebounds after impact. | $8,000-8,000 \mathrm{psi}$ | $8,000-16,000 \mathrm{psi}$ |  |
| Hard | R5 |  | $>16,000 \mathrm{psi}$ |

## RQD AND ROCK QUALITY

| Relation of RQD and Rock Quality |  |
| :---: | :---: |
| RQD (Rock <br> Quality Designation), \% | Description of <br> Rock Quality |
| $0-25$ | Very Poor |
| $25-50$ | Poor |
| $50-75$ | Fair |
| $75-90$ | Good |
| $90-100$ | Excellent |

Terminology for Planar Surface

| Bedding | Joints and Fractures | Spacing |
| :---: | :---: | :---: |
| Laminated | Very Close | $<2 \mathrm{in}$. |
| Thin | Close | $2 \mathrm{in} .-12 \mathrm{in}$. |
| Medium | Moderately Close | $12 \mathrm{in} .-36 \mathrm{in}$. |
| Thick | Wide | $36 \mathrm{in} .-10 \mathrm{ft}$ |
| Massive | Very Wide | $>10 \mathrm{ft}$ |




ATTACHMENT D

## ATTACHMENT D

## NEARBY EXPLORATIONS

The site plan and exploration logs completed by GeoDesign in the site vicinity for the SW $124^{\text {th }}$ Avenue Extension Project (GeoDesign, 2014) are presented in this attachment.

B－1 BORING
INF－I INFILTRATION TEST
LEGEND： EXISTING TOPOGRAPHY EXTENTS
EXISTING TOPOGRAPHY
（2＇INTERVALS；10＇INDEX CONTOURS）
ALIGNMENT
（500＇MAJOR STATIONING；100＇MINOR STATIONING）
B－1 BORING



RELATIVE DENSITY - COARSE-GRAINED SOILS

| Relative Density | Standard Penetration <br> Resistance | Dames \& Moore Sampler <br> (140-pound hammer) | Dames \& Moore Sampler <br> (300-pound hammer) |
| :---: | :---: | :---: | :---: |
| Very Loose | $0-4$ | $0-11$ | $0-4$ |
| Loose | $4-10$ | $11-26$ | $4-10$ |
| Medium Dense | $10-30$ | $26-74$ | $10-30$ |
| Dense | $30-50$ | $74-120$ | $30-47$ |
| Very Dense | More than 50 | More than 120 | More than 47 |
| CONSISTENCY - FINE-GRAINED SOIIS |  |  |  |

CONSISTENCY - FINE-GRAINED SOILS


| HARDNESS | DESCRIPTION |  |
| :---: | :---: | :---: |
| Extremely Soft (RO) <br> Very Soft (R1) <br> Soft (R2) <br> Medium Hard (R3) <br> Hard (R4) <br> Very Hard (R5) | Indented by thumbnail <br> Can be peeled by pocket knife or scratched with finger nail Can be peeled by a pocket knife with difficulty <br> Can be scratched by knife or pick <br> Can be scratched with knife or pick only with difficulty <br> Cannot be scratched with knife or sharp pick |  |
| WEATHERING | DESCRIPTION |  |
| Decomposed <br> Predominantly Decomposed <br> Moderately Weathered Slightly Weathered Fresh | Rock mass is completely decomposed <br> Rock mass is more than $50 \%$ decomposed <br> Rock mass is decomposed locally <br> Rock mass is generally fresh <br> No discoloration in rock fabric |  |
| JOINT SPACING | DESCRIPTION |  |
| Very Close Close <br> Moderate Close Wide Very Wide | Less than 2 inches 2 inches to 1 foot 1 foot to 3 feet 3 feet to 10 feet Greater than 10 feet |  |
| FRACTURING | FRACTURE SPACING |  |
| Very Intensely Fractured Intensely Fractured Moderately Fractured Slightly Fractured Very Slightly Fractured Unfractured | Chips and fragments with a few scattered short core lengths <br> 0.1 foot to 0.3 foot with scattered fragments intervals <br> 0.3 foot to 1 foot with most lengths 0.6 foot <br> 1 foot to 3 feet <br> Greater than 3 feet <br> No fractures |  |
| HEALING | DESCRIPTION |  |
| Not Healed <br> Partly Healed Moderately Healed Totally Healed | Discontinuity surface, fractured zone, sheared material or filling not re-cemented Less than $50 \%$ of fractured or sheared material Greater than $50 \%$ of fractured or sheared material All fragments bonded |  |
| GEODESIGNミ <br> 15575 SW Sequoia Parkway - Suite 100 Portland OR 97224 Off 503.968.8787 Fax 503.968.3068 | ROCK CLASSIFICATION SYSTEM | TABLE A-3 |











## ATTACHMENT E

## ATTACHMENT E

EXCERPTS FROM HANDBOOK OF RIPPING.

## H A N D B OOK OF RIPPING



## Rippability Investigation \& Prediction Service

Although visible laminations, faults, and fractures may indicate rippability and are usually helpful, conditions which are not visible are also important. That's because surface features give only a clue as to what lies underneath. To determine rippability when a field trial is not feasible, a method of estimating underlying characteristics is required.

Caterpillar has developed a systematic analysis procedure to predict the rippability of a rock formation which combines new
technology with geological and ripping experience. Our process for gathering the information necessary to make a prediction is called the Rippability Investigation and Prediction (RIP) service and is available through Caterpillar research. (Contact your district office.) The service consists of three steps:

1. Rock analysis
2. Site inspection
3. Seismic analysis

## Rock Mechanics Analysis

A rock mechanics analysis is the first phase of the RIP service and requires that a fresh rock sample be submitted to our lab for analysis along with other pertinent information about the site. (Minimum sample size should be $10^{\prime \prime} \times 10^{\prime \prime} \times 10^{\prime \prime}$.)


## Geological Site Inspection

The second phase of the RIP service consists of a site visit by Caterpillar personnel which includes a geological inspection. During the site inspection, the rock formation in question is examined for in-place rock mass characteristics that may affect a ripping tractor's performance. These may include rock type, degree of weathering, bedding features,

joint characteristics, and many other pertinent geological features.

## Seismic Evaluation

The third phase of the RIP service includes a seismic evaluation. Caterpillar introduced the use of the refraction seismograph in 1958 as an aid to determine rippability of materials. The instrument functions by measuring seismic velocity, an indicator of the degree of consolidation of rock formations. Caterpillar continues to offer this service, along with many independent firms.

Rippers


## Rippability Investigation \& Prediction Service



## Gaterpillar Systematic Analysis

Not all material conditions are visible from the surface. To determine the rippability of below-the-surface material and formations, Caterpillar Inc. developed a systematic analysis procedure based on technology and field experience. The service consists of three steps:

1. Rock analysis
2. Site inspection
3. Seismic evaluation

Rippers



## Rippers

D11R Ripper Seismic Velocity
Performance

- Multi or Single Shank Ripper
- Estimated by Seismic Wave Velocities

RIPPABLE
MARGINAL
NON-RIPPABLE

Meters Per Second x 1000
Feet Per Second x 1000
GLACIAL TILL
IGNEOUS ROCKS
GRANITE
BASALT
TRAP ROCK
SEDIMENTARY ROCKS
SHALE
SANDSTONE
SILTSTONE
CLAYSTONE
CONGLOMERATE
BRECCIA
Caliche
LIMESTONE
METAMORPHIC ROCKS
SCHIST
SLATE
MINERALS \& ORES
COAL
IRON ORE


| To: | Kirk Olsen From: | Gregory J. Schaertl, P.E. George Saunders, P.E., G.E. |
| :---: | :---: | :---: |
| Company: | Trammell Crow Company Date: | December 23, 2019 |
| Address: | 1300 SW $5^{\text {th }}$ Avenue, Suite 3050 <br> Portland, OR 97201 |  |
| cc: | Tom Nieswander, Trammell Crow Company (via email only) Jeff Shoemaker, DOWL (via email only) |  |
| GDI Project: | TrammellCr-74-01 |  |
| RE: | Geotechnical Data Memorandum <br> Orr Property <br> SW $124^{\text {th }}$ Avenue and SW Tualatin-Sherwood <br> Washington County, Oregon |  |

## INTRODUCTION

This memorandum provides a summary of observations made during supplemental explorations at the Orr Property located in Washington County, Oregon. We prepared a preliminary geotechnical report for the site dated February 6, 2019.' The boundaries of the site, topography, and location of site explorations are shown on Figure 1.

## DISCUSSION

We observed subsurface explorations performed by Kerr Contractors on December 11, 2019, which consisted of 20 test pit excavations (TP-1 through TP-20) to depths between 2 and 19 feet BGS. Nine of the locations were chosen by Kerr Contractors and the remaining locations were chosen by a combination of DOWL and GeoDesign. The exploration locations are shown relative to site boundaries and topography on Figure 1. Associated exploration logs are presented in the Attachment.

Subsurface conditions were generally consistent with observations made previously at the site during geotechnical subsurface explorations performed by GeoDesign and GRI. These explorations are summarized in our preliminary geotechnical report. Further results from the recent work is in general agreement with the prior work in areas where the recent explorations are in close proximity to prior explorations or seismic refraction profiles.

[^29]
## Memorandum

## LIMITATIONS

We have prepared this memorandum report for use by Trammell Crow Company and members of their design and construction team. The data and report can be used for bidding or estimating purposes, but our conclusions and interpretations should not be construed as warranty of the subsurface conditions and are not applicable to other nearby sites.

Within the limitations of scope, schedule, and budget, our services have been executed in accordance with generally accepted practices in this area at the time this memorandum was prepared. No warranty, express or implied, should be understood.

GJS:GPS:kt
Attachment
One copy submitted (via email only)
Document ID: TrammellCr-74-01-122319-geom.docx
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EXPIRES: 06/30/22

FIGURES



RELATIVE DENSITY - COARSE-GRAINED SOIL

| Relative Density | Standard Penetration <br> Resistance | Dames \& Moore Sampler <br> (140-pound hammer) | Dames \& Moore Sampler <br> (300-pound hammer) |
| :---: | :---: | :---: | :---: |
| Very Loose | $0-4$ | $0-11$ | $0-4$ |
| Loose | $4-10$ | $11-26$ | $4-10$ |
| Medium Dense | $10-30$ | $26-74$ | $10-30$ |
| Dense | $30-50$ | $74-120$ | $30-47$ |
| Very Dense | More than 50 | More than 120 | More than 47 |
| CONSISTENCY - FINE-GRAINED SOIL |  |  |  |


| Consistency |  | Standard Penetration Resistance | Dames \& MooreSampler(140-pound hammer) |  |  | Dames \& MooreSampler(300-pound hammer) |  |  | Unconfined Compressive Strength (tsf) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Very Soft |  | Less than 2 | Less than 3 |  |  | Less than 2 |  |  | Less than 0.25 |  |
| Soft |  | 2-4 | 3-6 |  |  | 2-5 |  |  | 0.25-0.50 |  |
| Medium Stiff |  | 4-8 | 6-12 |  |  | 5-9 |  |  | 0.50-1.0 |  |
| Stiff |  | 8-15 | 12-25 |  |  | 9-19 |  |  | 1.0-2.0 |  |
| Very Stiff |  | 15-30 | 25-65 |  |  | 19-31 |  |  | 2.0-4.0 |  |
| Hard |  | More than 30 | More than 65 |  |  | More than 31 |  |  | More than 4.0 |  |
| PRIMARY SOIL DIVISIONS |  |  |  |  |  | GROUP SYMBOL |  | GROUP NAME |  |  |
| COARSE- <br> GRAINED SOIL <br> (more than 50\% retained on No. 200 sieve) |  | GRAVEL <br> (more than $50 \%$ of coarse fraction retained on No. 4 sieve) | CLEAN GRAVEL <br> (< 5\% fines) |  |  | GW or GP |  | GRAVEL |  |  |
|  |  | GRAVEL WITH FINES <br> ( $\geq 5 \%$ and $\leq 12 \%$ fines) |  |  | GW-GM or GP-GM |  | GRAVEL with silt |  |  |
|  |  | GW-GC or GP-GC | GRAVEL with clay |  |  |
|  |  | GRAVEL WITH FINES (> 12\% fines) |  |  | GM |  | silty GRAVEL |  |  |
|  |  | clayey GRAVEL |  |  |
|  |  | GC-GM | silty, clayey GRAVEL |  |  |
|  |  | SAND | CLEAN SAND (<5\% fines) |  |  | SW or SP |  | SAND |  |  |
|  |  | (50\% or more of coarse fraction passing No. 4 sieve) | SAND WITH FINES ( $\geq 5 \%$ and $\leq 12 \%$ fines) |  |  | SW-SM | r SP-SM | SAND with silt |  |  |
|  |  | SW-SC |  |  |  | r SP-SC | SAND with clay |  |  |
|  |  | SAND WITH FINES (> 12\% fines) |  |  | SM |  | silty SAND |  |  |
|  |  | SC | clayey SAND |  |  |
|  |  | SC-SM | silty, clayey SAND |  |  |
| $\begin{aligned} & \text { FINE-GRAINED } \\ & \text { SOIL } \end{aligned}$ |  |  |  |  | SILT AND CLAY | Liquid limit less than 50 |  |  |  |  | SILT |  |  |
|  |  | CL | CLAY |  |  |  |  |  |
|  |  | CL-ML | silty CLAY |  |  |  |  |  |
| (50\% or more passing No. 200 sieve) |  |  |  |  | OL |  |  |  | ORGANIC SILT or ORGANIC CLAY |  |  |
|  |  | Liquid limit 50 or greater |  |  |  | MH |  | SILT |  |  |
|  |  | CH | CLAY |  |  |  |  |  |
|  |  | OH | ORGANIC SILT or ORGANIC CLAY |  |  |  |  |  |
| HIGHLY ORGANIC SOIL |  |  |  |  |  | PT |  | PEAT |  |  |
| MOISTURE CLASSIFICATION |  |  | ADDITIONAL CONSTITUENTS |  |  |  |  |  |  |  |
| Term | Field Test |  | Secondary granular components or other materials such as organics, man-made debris, etc. |  |  |  |  |  |  |  |
|  |  |  |  |  |  | Percent | Silt and Clay In: |  |  | Percent | Sand and Gravel In: |  |  |
| dry | very dry to |  |  |  | moisture, ouch |  | Fine-Grained Soil |  | arsened Soil |  |  | rained oil | CoarseGrained So |
| moist | damp, without visible moisture |  | < 5 | trace |  | trace |  | $<5$ |  | ace | trace |
|  |  |  | 5-12 | minor |  | with |  | 5-15 |  | nor | minor |
| wet | visible free water, usually saturated |  | > 12 | some |  |  | /clayey | 15-30 |  | th | with |
|  |  |  |  |  |  |  |  | > 30 |  | gravelly | Indicate \% |






















December 27, 2019

Jennifer Scola

Mackenzie
1515 SE Water Ave \#100
Portland, OR 97214
Re: 12822 SW Tualatin Sherwood Rd Sherwood, OR 97140
We have reviewed the site plan for the above-mentioned project. The site plan shows 5 enclosures; 1 enclosure by building $\mathrm{A}, 1$ enclosure by building $\mathrm{B}, 1$ enclosure by building $\mathrm{C}, 1$ enclosure by building $D$ and 1 enclosure by building $E$ each measuring $10^{\prime}$ deep and $20^{\prime}$ wide, which allows for straight on access.

The other details on the site plan are not shown. These requirements will need to be met to ensure our access:

- The gates need to be hinged in front of the enclosure walls to allow for the full $20^{\prime}$ width. This will also allow for the 120 degree opening angle that is required.
- No center post at the gate access point.
- The gates need cane bolts and holes put in place for the gates to be locked in the open and closed position. The holes for the gates to be held open need to be at the full 120 degree opening angle.
- There must be $25^{\prime}$ of overhead clearance.

If you have any questions, feel free to contact me.
Sincerely,
Kristen Tabscott
Pride Disposal Co.
(503) 625-6177


## MEMORANDUM

DATE: January 15, 2020
TO: Kirk Olsen (Trammell Crow Company)
FROM: Todd Prager, AICP, RCA \#597, ISA Board Certified Master Arborist
RE: $\quad$ Tree Plan for T-S Corporate Park

## Summary

This report includes tree removal and protection recommendations to meet the requirements in section 16.142.070 (Trees on Property Subject to Certain Land Use Applications) of the City of Sherwood Code for the T-S Corporate Park project.

The total canopy provided through the preservation of onsite trees will be 60.8 percent. The minimum canopy requirement for the proposed development is 30 percent. Therefore, no additional trees are required to be planted to meet the minimum canopy requirement.

## Background

Trammell Crow Company is proposing to develop the T-S Corporate Park at SW Tualatin Sherwood Road and SW 124th Avenue in Sherwood. Existing trees are present on the property in the area of the proposed development. The proposed site plan with grading, streets, buildings, and parking in relation to the existing trees is provided in Attachment 1.

The assignment requested of our firm for this project was to:

- Assess the existing trees at the project site;
- Identify the trees to be removed and retained based on construction impacts;
- Provide tree protection recommendations for the trees to be retained; and
- Provide recommendations for meeting the tree canopy requirements in section 16.142.070 of the City of Sherwood Code.


## Tree Assessment

In December 2019 and January 2020 our firm completed the inventory of existing trees outside and at the edges of proposed construction impacts at the project site.

The complete inventory data for each tree is provided in Attachment 2 and includes the tree number, common name, scientific name, trunk diameter (DBH), crown radius, crown area (canopy), mature crown radius for the species, mature crown area (canopy) for the species, canopy credit for preserved trees ( 2 x crown area), health condition, structural condition, pertinent comments, and treatment recommendations (remove or protect).

The tree numbers in the inventory in Attachment 2 correspond to the tree numbers on the proposed site plan in Attachment 1.

## Tree Removal and Retention

The standard tree protection requirement in the City of Sherwood Code is to limit construction activities within the driplines of the trees to be retained unless otherwise approved by the project arborist. A typical alternative minimum tree protection zone allows encroachments no closer than a radius from a tree of .5 feet per inch of DBH as long as no more than 25 percent of the root protection zone area (estimated at one foot radius per inch of DBH ) is impacted. Figure 1 illustrates this concept. This standard may need to be adjusted on a case by case basis due to tree health, species, root distribution, whether the tree will be impacted on multiple sides, and other factors.

Using the criteria described above and the locations of the trees relative to grading, paving, construction,


Figure 1: Typical minimum protection zone and other site improvements, 508 of the assessed trees will be removed and 505 trees will be retained.

Tree protection recommendations for the trees to be retained are provided in the next section of this report.

## Tree Protection Recommendations

The critical root zone radii of .5 feet per inch of DBH are shown on the site plan in Attachment 1 for the trees directly adjacent to proposed construction. The trees to be retained can be adequately protected by placing tree protection fencing at or beyond their critical root zones wherever possible as shown in Attachment 1. No grading, stockpiling, storage, disposal, or any other construction related activity shall occur in the tree protection zones unless specifically reviewed and approved by the project arborist.

[^30]The following additional tree protection measures shall apply to the trees to be retained:

- Tree Protection Fencing: Tree protection fencing shall be placed in the locations shown in Attachment 1. Note that there are several locations on the site plan in Attachment 1 where grading is recommended to be shifted to outside the tree protection zones. If the grading cannot be shifted, it shall be completed under arborist supervision to minimize root impacts. Also, trees within the north wetland shown on sheet C7.2 in Attachment 1 may be protected with orange work limit fencing that is already required around the wetland. Note that it will need to be expanded in a few places to better protect the tree root zones.
- Directional Felling: Fell the trees to be removed away from the trees to be retained so they do not contact or otherwise damage the trunks or branches of the trees to be retained. No vehicles or heavy equipment shall be permitted within the tree protection zones during tree removal operations.
- Protect Tree Crowns: Care will need to be taken to not contact or otherwise damage the crowns of the trees that may extend into the construction area.
- Pruning: It may be necessary or desirable to prune trees at the site. All pruning should be completed by a qualified tree service with an ISA Certified Arborist on site. All pruning should be in accordance with ANSI A300 pruning standards and Z133.1 safety standards and approved in coordination with the project arborist.
- Sediment Fencing: Sediment fencing shall be installed outside the protection zones of the trees to be retained to minimize root disturbances. If erosion control is required inside the protection zones, straw wattles shall be used on the soil surface.

Additional tree protection recommendations that are consistent with section 16.142.070.G for the trees to be retained are provided in Attachment 3.

## Tree Canopy Requirements

Section 16.142.070.D of the City of Sherwood Code requires the proposed development type to achieve a minimum total tree canopy of 30 percent. Trees that are retained receive credit for double their mature canopy area, and trees that are planted receive credit for the expected mature canopy area as determined by a certified arborist. Street trees are eligible for full canopy credit even though they are planted in the public right of way.

## Retained Trees

The canopy area for each of the 505 retained trees is provided in the tree inventory in Attachment 2. Their total combined canopy area is $492,114.5$ square feet. Since retained trees receive double canopy credit, the credit from preservation of the trees is 984,229 square feet. The total net site area is $1,618,892$ square feet. Therefore, the canopy provided by the preserved trees represents 60.8 percent of the site area.

[^31]
## Planted Trees

The minimum canopy requirement for the development is 30 percent. Since the canopy provided through preservation of existing trees is 60.8 percent, no additional trees are required to be planted to meet the canopy requirement.

## Conclusion

The total canopy provided through the preservation of trees at the site will be 60.8 percent. The minimum canopy requirement for the proposed development is 30 percent. Therefore, no additional trees are required to be planted to meet the minimum canopy requirement.

The trees to be retained will be adequately protected by adhering to the recommendations in this report. Any change to the tree protection plan should be approved by the project arborist to ensure that the trees to be retained are adequately protected.

Please contact me if you have questions, concerns, or need any additional information.

Sincerely,


Todd Pager
ASCA Registered Consulting Arborist \#597
ISA Board Certified Master Arborist, WE-6723B
ISA Qualified Tree Risk Assessor
AICP, American Planning Association
Attachments: Attachment 1 - Site Plan with Trees
Attachment 2 - Tree Inventory
Attachment 3 - Tree Protection Recommendations
Attachment 4-Assumptions and Limiting Conditions




January 15, 2020
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Attachment 2

| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1000 | Willow | Salix sp. | 19 | 14 | 616 | 20 | 1257 | 2513 | Good | Good | 6 stems 6,6,7,8,9,10. | Protect |
| 1001 | Willow | Salix sp. | 13 | 10 | 314 | 20 | 1257 | 2513 | Good | Good | 3 stems 7,8,8. | Protect |
| 1002 | Willow | Salix sp. | 6 | 5 | 79 | 20 | 1257 | 2513 | Poor | Poor | Suppressed | Protect |
| 1003 | Willow | Salix sp. | 20 | 21 | 1385 | 21 | 1385 | 2771 | Fair | Fair | 6 stems 6,6,8,8,10,10. Severe decay in smaller stems. | Protect |
| 1004 | Willow | Salix sp. | 20 | 13 | 531 | 20 | 1257 | 2513 | Good | Good | 4 stems 6,6,7,10. | Protect |
| 1005 | Willow | Salix sp. | 18 | 24 | 1810 | 24 | 1810 | 3619 | Good | Good | 2 stems 10,15. | Protect |
| 1006 | Willow | Salix sp. | 7 | 14 | 616 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1007 | Oregon Ash | Fraxinus latifolia | 10 | 14 | 616 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1008 | Common Hawthorn | Crataegus monogyna | 6 | 7 | 154 | 7 | 154 | 308 | Good | Good |  | Protect |
| 1009 | Common Hawthorn | Crataegus monogyna | 10 | 10 | 314 | 10 | 314 | 628 | Poor | Poor | Cracks in trunk. Decline. 2 stems 7,7. | Protect |
| 1010 | Common Hawthorn | Crataegus monogyna | 14 | 20 | 1257 | 20 | 1257 | 2513 | Good | Good | 4 stems 8,7,7,7. | Protect |
| 1011 | Common Hawthorn | Crataegus monogyna | 12 | 15 | 707 | 15 | 707 | 1414 | Good | Good | 2 stems 8,9. | Protect |
| 1012 | Common Hawthorn | Crataegus monogyna | 6 | 8 | 201 | 8 | 201 | 402 | Good | Good |  | Protect |
| 1013 | Common Apple | Malus sp. | 16 | 9 | 254 | 12 | 452 | 905 | Poor | Poor | 2 stems 14,8. Partial uproot | Protect |
| 1014 | Sweet Cherry | Prunus avium | 9 | 10 | 314 | 15 | 707 | 1414 | Poor | Poor | 2 stems 6,7. Suppressed | Protect |
| 1015 | Common Hawthorn | Crataegus monogyna | 6 | 9 | 254 | 9 | 254 | 509 | Good | Good |  | Protect |
| 1016 | Common Hawthorn | Crataegus monogyna | 6 | 12 | 452 | 12 | 452 | 905 | Good | Good |  | Protect |
| 1017 | Common Apple | Malus sp. | 6 | 5 | 79 | 12 | 452 | 905 | Poor | Poor | Suppressed | Protect |
| 1018 | Oregon Ash | Fraxinus latifolia | 12 | 12 | 452 | 15 | 707 | 1414 | Good | Good | 2 stems 7,10. | Protect |
| 1019 | Sweet Cherry | Prunus avium | 6 | 9 | 254 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1020 | Common Hawthorn | Crataegus monogyna | 7 | 9 | 254 | 9 | 254 | 509 | Poor | Poor | Suppressed | Protect |
| 1021 | Sweet Cherry | Prunus avium | 7 | 6 | 113 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1022 | Common Hawthorn | Crataegus monogyna | 9 | 15 | 707 | 15 | 707 | 1414 | Good | Good | 2 stems 6,7. | Protect |
| 1023 | Willow | Salix sp. | 9 | 7 | 154 | 20 | 1257 | 2513 | Poor | Poor | Suppressed. | Protect |
| 1024 | Sweet Cherry | Prunus avium | 7 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1025 | Common Hawthorn | Crataegus monogyna | 6 |  | 0 |  | 0 | 0 | Good | Good |  | Protect |
| 1026 | Sweet Cherry | Prunus avium | 7 | 9 | 254 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1027 | Common Hawthorn | Crataegus monogyna | 14 | 17 | 908 | 17 | 908 | 1816 | Poor | Poor | Broken tops. 2 stems 7,12. | Protect |
| 1028 | Sweet Cherry | Prunus avium | 13 | 11 | 380 | 15 | 707 | 1414 | Poor | Poor | Broken top | Protect |
| 1029 | Common Hawthorn | Crataegus monogyna |  |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1030 | Oregon Ash | Fraxinus latifolia | 7 | 7 | 154 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1031 | Common Hawthorn | Crataegus monogyna | 10 | 7 | 154 | 7 | 154 | 308 | Poor | Poor | Suppressed. 2 stems 6,8. | Protect |
| 1032 | Common Hawthorn | Crataegus monogyna | 8 | 5 | 79 | 7 | 154 | 308 | Poor | Poor | 2 stems 6,6. Severe lean east. | Protect |
| 1033 | Common Hawthorn | Crataegus monogyna | 9 | 7 | 154 | 7 | 154 | 308 | Poor | Poor | 2 stems 6,7. Suppressed | Protect |
| 1034 | Common Hawthorn | Crataegus monogyna | 7 | 8 | 201 | 8 | 201 | 402 | Poor | Poor | Suppressed | Protect |
| 1035 | Common Hawthorn | Crataegus monogyna | 9 | 8 | 201 | 8 | 201 | 402 | Poor | Poor | 2 stems 6,7. Suppressed | Protect |
| 1036 | Common Hawthorn | Crataegus monogyna | 6 | 4 | 50 | 7 | 154 | 308 | Poor | Poor | Suppressed. | Protect |
| 1037 | Common Hawthorn | Crataegus monogyna | 6 | 4 | 50 | 7 | 154 | 308 | Poor | Poor | Suppressed. | Protect |
| 1038 | Oregon Ash | Fraxinus latifolia | 20 | 18 | 1018 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1039 | Oregon Ash | Fraxinus latifolia | 19 | 19 | 1134 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1040 | Common Hawthorn | Crataegus monogyna | 6 | 5 | 79 | 7 | 154 | 308 | Fair | Fair |  | Protect |
| 1041 | Common Hawthorn | Crataegus monogyna | 6 | 5 | 79 | 7 | 154 | 308 | Fair | Fair |  | Protect |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1042 | Oregon Ash | Fraxinus Iatifolia | 35 | 28 | 2463 | 15 | 707 | 1414 | Fair | Fair | 9 stems $7,12,7,13,11,14,11,12,15$. History of larger limb failure. | Protect |
| 1043 | Sweet Cherry | Prunus avium | 6 | 4 | 50 | 15 | 707 | 1414 | Poor | Poor | Suppressed. | Protect |
| 1044 | Sweet Cherry | Prunus avium | 9 | 4 | 50 | 15 | 707 | 1414 | Good | Good | 2 stems 6,7. | Protect |
| 1045 | Sweet Cherry | Prunus avium | 6 | 3 | 28 | 15 | 707 | 1414 | Fair | Poor | Suppressed. | Protect |
| 1050 | Sweet Cherry | Prunus avium | 8 | 14 | 616 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1051 | Sweet Cherry | Prunus avium | 8 | 6 | 113 | 15 | 707 | 1414 | Good | Good | 2 stems 6,6. | Protect |
| 1052 | Sweet Cherry | Prunus avium | 6 | 4 | 50 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1053 | Cascara | Rhamnus purshiana | 8 | 9 | 254 | 12 | 452 | 905 | Good | Good | 2 stems 6,6. | Protect |
| 1151 | Pacific Madrone | Arbutus menziesii | 7 | 7 | 154 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1152 | Pacific Madrone | Arbutus menziesii | 11 | 13 | 531 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1153 | Pacific Madrone | Arbutus menziesii | 7 | 5 | 79 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1156 | Oregon White Oak | Quercus garrayana | 7 | 9 | 254 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1157 | Douglas Fir | Pseudotsuga menziesii | 8 | 4 | 50 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1158 | Douglas Fir | Pseudotsuga menziesii | 11 | 7 | 154 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1159 | Douglas Fir | Pseudotsuga menziesii | 14 | 12 | 452 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1160 | Douglas Fir | Pseudotsuga menziesii | 18 | 12 | 452 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1161 | Douglas Fir | Pseudotsuga menziesii | 18 | 8 | 201 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1168 | Pacific Madrone | Arbutus menziesii | 10 | 14 | 616 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1170 | Oregon White Oak | Quercus garrayana | 22 | 15 | 707 | 25 | 1963 | 3927 | Good | Good | 2 stems 13,18. | Protect |
| 1171 | Douglas Fir | Pseudotsuga menziesii | 21 | 20 | 1257 | 20 | 1257 | 2513 | Poor | Poor | Thin crown. | Protect |
| 1172 | Douglas Fir | Pseudotsuga menziesii | 16 | 14 | 616 | 20 | 1257 | 2513 | Very Poor | Very Poor | Broken top. Severe crown die back. | Protect |
| 1173 | Douglas Fir | Pseudotsuga menziesii | 23 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1174 | Douglas Fir | Pseudotsuga menziesii | 23 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1175 | Sweet Cherry | Prunus avium | 6 | 5 | 79 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1176 | Sweet Cherry | Prunus avium | 14 | 13 | 531 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1177 | Sweet Cherry | Prunus avium | 7 | 5 | 79 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1178 | Sweet Cherry | Prunus avium | 6 | 5 | 79 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1179 | Sweet Cherry | Prunus avium | 6 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1180 | Pacific Madrone | Arbutus menziesii | 7 | 11 | 380 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1181 | Sweet Cherry | Prunus avium | 7 | 10 | 314 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1182 | Sweet Cherry | Prunus avium | 8 | 10 | 314 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1183 | Oregon White Oak | Quercus garrayana | 8 | 9 | 254 | 25 | 1963 | 3927 | Good | Good | 2 stems 6,6. | Protect |
| 1184 | Sweet Cherry | Prunus avium | 8 | 13 | 531 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1185 | Pacific Madrone | Arbutus menziesii | 14 | 15 | 707 | 15 | 707 | 1414 | Good | Good | 3 stems 7,8,9. | Protect |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1188 | Douglas Fir | Pseudotsuga menziesii | 21 | 18 | 1018 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1189 | Pacific Madrone | Arbutus menziesii | 9 | 13 | 531 | 15 | 707 | 1414 | Good | Good | 2 stems 8,5. | Protect |
| 1190 | Cascara | Rhamnus purshiana | 7 | 5 | 79 | 12 | 452 | 905 | Good | Good |  | Protect |
| 1191 | Oregon Ash | Fraxinus latifolia | 9 | 12 | 452 | 15 | 707 | 1414 | Good | Good | 2 stems 7,8. | Protect |
| 1192 | Oregon Ash | Fraxinus latifolia | 25 | 18 | 1018 | 18 | 1018 | 2036 | Good | Good |  | Protect |
| 1193 | Oregon Ash | Fraxinus latifolia | 18 | 22 | 1521 | 22 | 1521 | 3041 | Good | Good |  | Protect |
| 1194 | Oregon Ash | Fraxinus latifolia | 24 | 16 | 804 | 16 | 804 | 1608 | Good | Good |  | Protect |
| 1195 | Oregon Ash | Fraxinus latifolia | 15 | 12 | 452 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1196 | Douglas Fir | Pseudotsuga menziesii | 21 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1197 | Oregon Ash | Fraxinus latifolia | 9 | 10 | 314 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1198 | Douglas Fir | Pseudotsuga menziesii | 25 | 23 | 1662 | 23 | 1662 | 3324 | Good | Good |  | Protect |
| 1199 | Oregon Ash | Fraxinus latifolia | 23 | 18 | 1018 | 18 | 1018 | 2036 | Good | Good | 2 stems 6,22. | Protect |
| 1200 | Cascara | Rhamnus purshiana | 6 | 10 | 314 | 12 | 452 | 905 | Poor | Poor | Trunk decay. Epicormic sprouts. | Protect |
| 1201 | Cascara | Rhamnus purshiana | 6 | 10 | 314 | 12 | 452 | 0 | Poor | Poor | Trunk decay. Epicormic sprouts. Offsite. | Protect |
| 1202 | Oregon Ash | Fraxinus latifolia | 12 | 10 | 314 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1203 | Oregon Ash | Fraxinus latifolia | 16 | 10 | 314 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1204 | Oregon Ash | Fraxinus latifolia | 12 | 8 | 201 | 15 | 707 | 1414 | Fair | Fair | Thin crown. | Protect |
| 1205 | Oregon Ash | Fraxinus latifolia | 15 | 11 | 380 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1206 | Oregon Ash | Fraxinus latifolia | 25 | 15 | 707 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1207 | Oregon Ash | Fraxinus latifolia | 12 | 9 | 254 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1208 | Bigleaf Maple | Acer macrophyllum | 7 | 8 | 201 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1209 | Douglas Fir | Pseudotsuga menziesii | 13 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1210 | Douglas Fir | Pseudotsuga menziesii | 19 | 13 | 531 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1211 | Douglas Fir | Pseudotsuga menziesii | 27 | 24 | 1810 | 24 | 1810 | 3619 | Good | Good |  | Protect |
| 1212 | Douglas Fir | Pseudotsuga menziesii | 17 | 20 | 1257 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1213 | Sweet Cherry | Prunus avium | 6 | 4 | 50 | 15 | 707 | 1414 | Poor | Poor | Partial uproot. | Protect |
| 1214 | Sweet Cherry | Prunus avium | 6 | 9 | 254 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1215 | Bigleaf Maple | Acer macrophyllum | 12 | 18 | 1018 | 25 | 1963 | 3927 | Good | Good | 2 stems 8,9. | Protect |
| 1216 | Oregon Ash | Fraxinus latifolia | 10 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1217 | Oregon Ash | Fraxinus latifolia | 6 | 7 | 154 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1218 | Oregon Ash | Fraxinus Iatifolia | 9 | 10 | 314 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1219 | Oregon Ash | Fraxinus latifolia | 9 | 10 | 314 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1220 | Oregon Ash | Fraxinus latifolia | 25 | 16 | 804 | 15 | 707 | 1414 | Poor | Poor | Trunk decay. | Protect |
| 1221 | Oregon Ash | Fraxinus latifolia | 48 | 20 | 1257 | 15 | 707 | 1414 | Poor | Poor | Trunk decay Broken top. | Protect |
| 1222 | Oregon Ash | Fraxinus latifolia | 29 | 22 | 1521 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1223 | Oregon Ash | Fraxinus latifolia | 19 | 12 | 452 | 15 | 707 | 1414 | Good | Good | 2 stems 9,17. | Protect |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1224 | Oregon Ash | Fraxinus latifolia | 12 |  | 0 | 15 | 707 | 1414 | Dead |  |  | Protect |
| 1225 | Oregon Ash | Fraxinus latifolia | 10 | 13 | 531 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1226 | Oregon Ash | Fraxinus latifolia | 8 | 9 | 254 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1227 | Oregon Ash | Fraxinus latifolia | 14 | 8 | 201 | 15 | 707 | 1414 | Fair | Fair | Trunk decay. | Protect |
| 1228 | Oregon Ash | Fraxinus latifolia | 12 | 12 | 452 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1229 | Oregon Ash | Fraxinus latifolia | 23 | 15 | 707 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1230 | Oregon Ash | Fraxinus latifolia | 25 | 20 | 1257 | 15 | 707 | 1414 | Poor | Poor | Trunk decay | Protect |
| 1231 | Oregon Ash | Fraxinus latifolia | 17 | 13 | 531 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1232 | Oregon Ash | Fraxinus latifolia | 11 | 5 | 79 | 15 | 707 | 1414 | Poor | Poor | Trunk decay. Suppressed | Protect |
| 1233 | Oregon Ash | Fraxinus latifolia | 9 |  | 0 | 15 | 707 | 1414 | Dead |  |  | Protect |
| 1234 | Oregon Ash | Fraxinus latifolia | 11 | 11 | 380 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1235 | Willow | Salix sp. | 8 | 9 | 254 | 20 | 1257 | 2513 | Poor | Poor | Trunk decay. | Protect |
| 1236 | Sweet Cherry | Prunus avium | 6 | 7 | 154 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1237 | Willow | Salix sp. | 6 | 5 | 79 | 20 | 1257 | 2513 | Poor | Poor | Trunk decay. | Protect |
| 1238 | Sweet Cherry | Prunus avium | 10 | 14 | 616 | 15 | 707 | 1414 | Good | Good | 2 stems 7,7. | Protect |
| 1239 | Cascara | Rhamnus purshiana | 6 | 8 | 201 | 12 | 452 | 905 | Good | Good |  | Protect |
| 1240 | Willow | Salix sp. | 7 | 8 | 201 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1241 | Sweet Cherry | Prunus avium | 13 | 19 | 1134 | 19 | 1134 | 2268 | Good | Good | 3 stems 6,8,8. | Protect |
| 1242 | Bigleaf Maple | Acer macrophyllum | 17 | 20 | 1257 | 25 | 1963 | 3927 | Good | Good | 5 stems 6,7,8,8,8. | Protect |
| 1243 | Bigleaf Maple | Acer macrophyllum | 17 | 12 | 452 | 25 | 1963 | 3927 | Good | Good | 4 stems $7,8,9,9$. | Protect |
| 1244 | Bigleaf Maple | Acer macrophyllum | 16 | 20 | 1257 | 25 | 1963 | 3927 | Good | Good | 6 stems 6,6,7,7,7,7. | Protect |
| 1245 | Sweet Cherry | Prunus avium | 10 | 10 | 314 | 15 | 707 | 1414 | Good | Good | 2 stems 7,7. | Protect |
| 1246 | Sweet Cherry | Prunus avium | 15 | 10 | 314 | 15 | 707 | 1414 | Good | Good | 4 stems 6,7,8,8. | Protect |
| 1247 | Douglas Fir | Pseudotsuga menziesii | 13 | 12 | 452 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1248 | Willow | Salix sp. | 12 | 12 | 452 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1249 | Douglas Fir | Pseudotsuga menziesii | 10 | 11 | 380 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1250 | Douglas Fir | Pseudotsuga menziesii | 19 | 15 | 707 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1251 | Douglas Fir | Pseudotsuga menziesii | 7 | 9 | 254 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1252 | Willow | Salix sp. | 7 | 8 | 201 | 20 | 1257 | 2513 | Poor | Poor | Trunk decay. | Protect |
| 1253 | Douglas Fir | Pseudotsuga menziesii | 6 | 8 | 201 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1254 | Willow | Salix sp. | 6 | 4 | 50 | 20 | 1257 | 2513 | Poor | Poor | Suppressed | Protect |
| 1255 | Willow | Salix sp. | 10 | 6 | 113 | 20 | 1257 | 2513 | Poor | Poor | Partial uproot. | Protect |
| 1256 | Willow | Salix sp. | 7 | 4 | 50 | 20 | 1257 | 2513 | Poor | Poor | Suppressed | Protect |
| 1257 | Douglas Fir | Pseudotsuga menziesii | 10 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1258 | Douglas Fir | Pseudotsuga menziesii | 17 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1259 | Willow | Salix sp. | 8 | 10 | 314 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1260 | Willow | Salix sp. | 7 | 11 | 380 | 20 | 1257 | 2513 | Good | Good |  | Protect |

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Attachment 2

| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1261 | Willow | Salix sp. | 6 | 8 | 201 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1262 | Sweet Cherry | Prunus avium | 7 | 7 | 154 | 15 | 707 | 1414 | Good | Good | 2 stems 4,6. | Protect |
| 1263 | Willow | Salix sp. | 7 | 8 | 201 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1264 | Douglas Fir | Pseudotsuga menziesii | 8 | 8 | 201 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1265 | Willow | Salix sp. | 6 | 5 | 79 | 20 | 1257 | 2513 | Fair | Fair | Thin crown. | Protect |
| 1266 | Willow | Salix sp. | 6 | 8 | 201 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1267 | Willow | Salix sp. | 6 | 8 | 201 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1268 | Sweet Cherry | Prunus avium | 6 | 7 | 154 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1269 | Willow | Salix sp. | 8 | 3 | 28 | 20 | 1257 | 2513 | Poor | Poor | Crown die back. | Protect |
| 1270 | Douglas Fir | Pseudotsuga menziesii | 15 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1271 | Sweet Cherry | Prunus avium | 6 | 5 | 79 | 15 | 707 | 1414 | Poor | Poor | Crown die back. | Protect |
| 1272 | Douglas Fir | Pseudotsuga menziesii | 15 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1273 | Sweet Cherry | Prunus avium |  | 13 | 531 | 15 | 707 | 1414 | Good | Good | 2 stems 6,8. | Protect |
| 1274 | Oregon Ash | Fraxinus latifolia | 16 | 8 | 201 | 15 | 707 | 1414 | Poor | Poor | Lost top. | Protect |
| 1275 | Oregon Ash | Fraxinus latifolia |  | 11 | 380 | 15 | 707 | 1414 | Poor | Poor | Thin crown. 2 stems 13,17. | Protect |
| 1276 | Oregon Ash | Fraxinus latifolia | 8 | 6 | 113 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1277 | Oregon Ash | Fraxinus latifolia | 8 | 10 | 314 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1278 | Oregon Ash | Fraxinus latifolia | 13 | 7 | 154 | 15 | 707 | 1414 | Poor | Poor | Partial uproot. | Protect |
| 1279 | Oregon Ash | Fraxinus latifolia | 8 | 6 | 113 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1280 | Oregon Ash | Fraxinus latifolia | 6 | 3 | 28 | 15 | 707 | 1414 | Very Poor | Very Poor | Severe trunk decay. | Protect |
| 1281 | Oregon Ash | Fraxinus latifolia | 10 |  | 0 | 15 | 707 | 1414 | Dead |  |  | Protect |
| 1282 | Oregon Ash | Fraxinus latifolia | 12 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1283 | Oregon Ash | Fraxinus latifolia | 9 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1284 | Oregon Ash | Fraxinus latifolia | 6 | 4 | 50 | 15 | 707 | 1414 | Poor | Poor | Suppressed. | Protect |
| 1285 | Oregon Ash | Fraxinus latifolia | 13 | 10 | 314 | 15 | 707 | 1414 | Fair | Fair | History of limb failure. | Protect |
| 1286 | Oregon Ash | Fraxinus latifolia | 10 | 5 | 79 | 15 | 707 | 1414 | Poor | Poor | Broken top. | Protect |
| 1287 | Oregon Ash | Fraxinus latifolia | 11 | 12 | 452 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1288 | Oregon Ash | Fraxinus latifolia | 7 | 4 | 50 | 15 | 707 | 1414 | Poor | Poor | Suppressed | Protect |
| 1289 | Oregon Ash | Fraxinus latifolia | 22 | 15 | 707 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1290 | Douglas Fir | Pseudotsuga menziesii | 10 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1291 | Douglas Fir | Pseudotsuga menziesii | 15 | 12 | 452 | 20 | 1257 | 2513 | Fair | Fair | Thinning crown. | Protect |
| 1292 | Douglas Fir | Pseudotsuga menziesii | 8 | 9 | 254 | 20 | 1257 | 2513 | Fair | Fair | Thinning crown. | Protect |
| 1293 | Bigleaf Maple | Acer macrophyllum | 6 | 9 | 254 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1294 | Willow | Salix sp. | 6 | 6 | 113 | 20 | 1257 | 2513 | Poor | Poor | Trunk decay | Protect |
| 1295 | Bigleaf Maple | Acer macrophyllum |  | 19 | 1134 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1296 | Bigleaf Maple | Acer macrophyllum | 6 | 10 | 314 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1297 | Bigleaf Maple | Acer macrophyllum | 7 | 10 | 314 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1298 | Willow | Salix sp. | 8 | 9 | 254 | 20 | 1257 | 2513 | Good | Good |  | Protect |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1299 | Willow | Salix sp. | 9 | 10 | 314 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1300 | Oregon Ash | Fraxinus latifolia | 7 | 5 | 79 | 15 | 707 | 1414 | Poor | Poor | Suppressed. | Protect |
| 1301 | Oregon Ash | Fraxinus latifolia | 10 | 10 | 314 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1302 | Oregon Ash | Fraxinus latifolia | 11 | 9 | 254 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1303 | Oregon Ash | Fraxinus Iatifolia | 13 | 8 | 201 | 15 | 707 | 1414 | Fair | Fair | Suppressed | Protect |
| 1304 | Oregon Ash | Fraxinus Iatifolia | 6 | 6 | 113 | 15 | 707 | 1414 | Poor | Poor | Suppressed | Protect |
| 1305 | Oregon Ash | Fraxinus latifolia | 8 |  | 0 | 15 | 707 | 1414 | Dead |  |  | Protect |
| 1306 | Oregon Ash | Fraxinus latifolia | 11 | 7 | 154 | 15 | 707 | 1414 | Poor | Poor | Partial uproot. | Protect |
| 1307 | Oregon Ash | Fraxinus latifolia | 14 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1308 | Oregon Ash | Fraxinus latifolia | 8 | 8 | 201 | 15 | 707 | 1414 | Fair | Fair | Suppressed | Protect |
| 1309 | Oregon Ash | Fraxinus Iatifolia | 18 | 12 | 452 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1310 | Oregon Ash | Fraxinus latifolia | 9 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1311 | Bigleaf Maple | Acer macrophyllum | 9 | 11 | 380 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1311.1 | Oregon Ash | Oregon Ash | 10 | 7 | 154 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1312 | Douglas Fir | Pseudotsuga menziesii | 8 | 9 | 254 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1313 | Bigleaf Maple | Acer macrophyllum | 7 | 9 | 254 | 25 | 1963 | 3927 | Poor | Fair | Thin crown | Protect |
| 1314 | Bigleaf Maple | Acer macrophyllum | 7 | 12 | 452 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1315 | Bigleaf Maple | Acer macrophyllum | 6 | 12 | 452 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1316 | Bigleaf Maple | Acer macrophyllum | 8 | 14 | 616 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1317 | Bigleaf Maple | Acer macrophyllum | 12 | 16 | 804 | 25 | 1963 | 3927 | Good | Good | 3 stems 6,6,8. | Protect |
| 1318 | Bigleaf Maple | Acer macrophyllum | 7 | 8 | 201 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1319 | Douglas Fir | Pseudotsuga menziesii | 10 | 12 | 452 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1320 | Douglas Fir | Pseudotsuga menziesii | 9 | 9 | 254 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1321 | Douglas Fir | Pseudotsuga menziesii | 20 | 17 | 908 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1322 | Bigleaf Maple | Acer macrophyllum | 7 | 9 | 254 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1323 | Willow | Salix sp. | 8 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1324 | Willow | Salix sp. | 7 | 9 | 254 | 20 | 1257 | 2513 | Poor | Poor | Trunk decay. | Protect |
| 1325 | Douglas Fir | Pseudotsuga menziesii | 6 | 8 | 201 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1326 | Oregon Ash | Oregon Ash | 10 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1327 | Douglas Fir | Pseudotsuga menziesii | 6 | 5 | 79 | 20 | 1257 | 2513 | Poor | Poor | Suppressed | Protect |
| 1328 | Douglas Fir | Pseudotsuga menziesii | 6 | 5 | 79 | 20 | 1257 | 2513 | Poor | Poor | Suppressed | Protect |
| 1329 | Willow | Salix sp. | 8 | 9 | 254 | 20 | 1257 | 2513 | Poor | Poor | Trunk decay. | Protect |
| 1330 | Willow | Salix sp. | 6 | 7 | 154 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1331 | Oregon Ash | Fraxinus latifolia | 9 | 7 | 154 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1332 | Oregon Ash | Fraxinus latifolia | 18 | 13 | 531 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1333 | Oregon Ash | Fraxinus latifolia | 8 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1334 | Oregon Ash | Fraxinus latifolia | 12 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1335 | Oregon Ash | Fraxinus latifolia | 18 | 13 | 531 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1336 | Oregon Ash | Fraxinus latifolia | 26 | 19 | 1134 | 19 | 1134 | 2268 | Good | Good | 2 stems 6,25. | Protect |
| 1337 | Oregon Ash | Fraxinus latifolia | 7 | 5 | 79 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1338 | Oregon Ash | Fraxinus latifolia | 7 | 5 | 79 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1339 | Oregon Ash | Fraxinus latifolia | 12 | 9 | 254 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1340 | Oregon Ash | Fraxinus latifolia | 8 | 9 | 254 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1341 | Oregon Ash | Fraxinus latifolia | 30 | 25 | 1963 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1342 | Oregon Ash | Fraxinus latifolia | 14 | 7 | 154 | 15 | 707 | 1414 | Poor | Poor | Thinning crown Broken top. | Protect |
| 1343 | Oregon Ash | Fraxinus latifolia | 27 |  | 0 |  | 0 | 0 | Poor | Poor | History of large limb failure | Protect |
| 1344 | Oregon Ash | Fraxinus latifolia | 8 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1345 | Oregon Ash | Fraxinus latifolia | 9 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1346 | Oregon Ash | Fraxinus latifolia | 13 | 9 | 254 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1347 | Oregon Ash | Fraxinus latifolia | 12 | 10 | 314 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1348 | Oregon Ash | Fraxinus latifolia | 17 | 14 | 616 | 15 | 707 | 1414 | Fair | Fair | History of large limb failure. | Protect |
| 1349 | Oregon Ash | Fraxinus Iatifolia | 13 | 9 | 254 | 15 | 707 | 1414 | Poor | Poor | 2 stems 6,12. History of large limb failure. | Protect |
| 1350 | Oregon Ash | Fraxinus latifolia | 10 | 5 | 79 | 15 | 707 | 1414 | Poor | Poor | Thinning crown. Limb loss. | Protect |
| 1351 | Oregon Ash | Fraxinus latifolia | 22 | 18 | 1018 | 18 | 1018 | 2036 | Good | Good |  | Protect |
| 1352 | Oregon Ash | Fraxinus latifolia | 11 | 7 | 154 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1353 | Oregon Ash | Fraxinus latifolia | 10 | 11 | 380 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1354 | Willow | Salix sp. | 6 | 6 | 113 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1355 | Oregon Ash | Fraxinus latifolia | 10 | 7 | 154 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1356 | Douglas Fir | Pseudotsuga menziesii | 8 | 10 | 314 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1357 | Willow | Salixsp. | 6 | 6 | 113 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1358 | Bigleaf Maple | Acer macrophyllum | 17 | 14 | 616 | 25 | 1963 | 3927 | Good | Good | 6 stems 6,6,6,7,8,9. | Protect |
| 1359 | Pacific Madrone | Arbutus menziesii | 12 | 11 | 380 | 15 | 707 | 1414 | Good | Good | 4 stems 7,7,5,5. | Protect |
| 1360 | Douglas Fir | Pseudotsuga menziesii | 9 | 6 | 113 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1361 | Douglas Fir | Pseudotsuga menziesii | 6 | 6 | 113 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1362 | Willow | Salix sp. | 8 | 12 | 452 | 20 | 1257 | 2513 | Fair | Fair | Trunk cavity. | Protect |
| 1363 | Douglas Fir | Pseudotsuga menziesii | 6 | 6 | 113 | 20 | 1257 | 2513 | Good | Good |  | Protect |

[^32]| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1364 | Douglas Fir | Pseudotsuga menziesii | 17 | 14 | 616 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1365 | Willow | Salixsp. | 8 | 5 | 79 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1366 | Oregon Ash | Fraxinus latifolia | 13 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1367 | Oregon Ash | Fraxinus latifolia | 26 | 10 | 314 | 15 | 707 | 1414 | Poor | Poor | 2 stems 16,20. Broken tops. | Protect |
| 1368 | Oregon Ash | Fraxinus latifolia | 10 | 8 | 201 | 15 | 707 | 1414 | Poor | Poor | Thin crown | Protect |
| 1369 | Oregon Ash | Fraxinus latifolia | 12 | 8 | 201 | 15 | 707 | 1414 | Poor | Poor | Broken top | Protect |
| 1370 | Oregon Ash | Fraxinus latifolia | 12 | 7 | 154 | 15 | 707 | 1414 | Poor | Poor | Thin crown. Crown die back. | Protect |
| 1371 | Oregon Ash | Fraxinus latifolia | 12 | 4 | 50 | 15 | 707 | 1414 | Poor | Poor | Dead top. | Protect |
| 1372 | Oregon Ash | Fraxinus latifolia | 13 | 10 | 314 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1373 | Oregon Ash | Fraxinus latifolia | 16 | 15 | 707 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1374 | Oregon Ash | Fraxinus latifolia | 8 | 4 | 50 | 15 | 707 | 1414 | Poor | Poor | Suppressed. | Protect |
| 1375 | Oregon Ash | Fraxinus latifolia | 17 | 10 | 314 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1376 | Oregon Ash | Fraxinus latifolia | 17 | 15 | 707 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1377 | Oregon Ash | Fraxinus latifolia | 6 | 4 | 50 | 15 | 707 | 1414 | Poor | Poor | Broken top. | Protect |
| 1378 | Oregon Ash | Fraxinus latifolia | 12 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1379 | Oregon Ash | Fraxinus latifolia | 6 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1380 | Oregon Ash | Fraxinus latifolia | 17 | 20 | 1257 | 20 | 1257 | 0 | Good | Good | Offsite. | Protect |
| 1381 | Oregon Ash | Fraxinus latifolia | 9 | 6 | 113 | 15 | 707 | 1414 | Poor | Poor | Broken top. | Protect |
| 1382 | Oregon Ash | Fraxinus latifolia | 6 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1383 | Oregon Ash | Fraxinus latifolia | 8 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1384 | Oregon Ash | Fraxinus Iatifolia | 15 | 13 | 531 | 15 | 707 | 1414 | Fair | Fair | Crown die back. History of limb loss. | Protect |
| 1385 | Oregon Ash | Fraxinus latifolia | 8 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1386 | Oregon Ash | Fraxinus latifolia | 8 | 7 | 154 | 15 | 707 | 1414 | Poor | Poor | Suppressed. | Protect |
| 1387 | Oregon Ash | Fraxinus latifolia | 17 | 12 | 452 | 15 | 707 | 1414 | Good | Good | 2 stems 6,16. | Protect |
| 1387.1 | Oregon Ash | Fraxinus latifolia | 7 | 7 | 154 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1388 | Oregon Ash | Fraxinus latifolia | 17 | 18 | 1018 | 18 | 1018 | 2036 | Good | Good |  | Protect |
| 1389 | Oregon Ash | Fraxinus latifolia | 14 | 15 | 707 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1390 | Oregon Ash | Fraxinus latifolia | 10 | 12 | 452 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1391 | Oregon Ash | Fraxinus latifolia | 15 | 14 | 616 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1392 | Oregon Ash | Fraxinus latifolia | 7 | 9 | 254 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1393 | Oregon Ash | Fraxinus latifolia | 13 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1394 | Oregon Ash | Fraxinus latifolia | 10 | 10 | 314 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1395 | Oregon Ash | Fraxinus latifolia | 14 | 7 | 154 | 15 | 707 | 1414 | Poor | Poor | Broken top. | Protect |
| 1396 | Oregon Ash | Fraxinus latifolia | 15 | 13 | 531 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1397 | Oregon Ash | Fraxinus latifolia | 10 | 7 | 154 | 15 | 707 | 1414 | Poor | Poor | Broken top. | Protect |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1398 | Oregon Ash | Fraxinus latifolia | 14 | 16 | 804 | 16 | 804 | 1608 | Good | Good |  | Protect |
| 1399 | Oregon Ash | Fraxinus Iatifolia | 9 | 10 | 314 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1400 | Oregon Ash | Fraxinus latifolia | 9 | 10 | 314 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1401 | Oregon Ash | Fraxinus latifolia | 7 | 10 | 314 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1402 | Oregon Ash | Fraxinus Iatifolia | 10 | 9 | 254 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1403 | Oregon White Oak | Quercus garrayana | 29 | 42 | 5542 | 42 | 5542 | 11084 | Good | Good | 2 stems 17,23. | Protect |
| 1404 | Douglas Fir | Pseudotsuga menziesii | 17 | 15 | 707 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1405 | Bigleaf Maple | Acer macrophyllum | 6 | 12 | 452 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1406 | Oregon Ash | Fraxinus latifolia | 8 | 9 | 254 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1407 | Bigleaf Maple | Acer macrophyllum | 15 | 13 | 531 | 25 | 1963 | 3927 | Fair | Fair | Broken top. Thin crown. 2 stems 7,13. | Protect |
| 1408 | Oregon White Oak | Quercus garrayana | 9 | 10 | 314 | 25 | 1963 | 3927 | Good | Good | 3 stems 6,5,5. | Protect |
| 1409 | Oregon Ash | Fraxinus latifolia | 6 | 9 | 254 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1410 | Douglas Fir | Pseudotsuga menziesii | 20 | 20 | 1257 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1411 | Sweet Cherry | Prunus avium | 7 | 9 | 254 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1412 | Oregon White Oak | Quercus garrayana | 10 | 12 | 452 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1413 | Sweet Cherry | Prunus avium | 6 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1414 | Willow | Salix sp. | 8 | 12 | 452 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1418 | Oregon Ash | Fraxinus latifolia | 23 | 18 | 1018 | 18 | 1018 | 2036 | Good | Good | 2 stems 8,22 | Protect |
| 1419 | Oregon Ash | Fraxinus latifolia | 13 | 17 | 908 | 17 | 908 | 1816 | Good | Good | 2 stems 6,12. | Protect |
| 1420 | Sweet Cherry | Prunus avium | 8 | 6 | 113 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1421 | Sweet Cherry | Prunus avium | 7 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1422 | Sweet Cherry | Prunus avium | 6 | 6 | 113 | 15 | 707 | 1414 | Poor | Poor | Broken top. | Protect |
| 1423 | Sweet Cherry | Prunus avium | 7 | 8 | 201 | 15 | 707 | 1414 | Poor | Poor | Broken top | Protect |
| 1424 | Sweet Cherry | Prunus avium | 6 | 6 | 113 | 15 | 707 | 1414 | Poor | Poor | Broken top. | Protect |
| 1425 | Willow | Salix sp. | 18 | 21 | 1385 | 21 | 1385 | 2771 | Good | Good |  | Protect |
| 1426 | Willow | Salix sp. | 12 | 15 | 707 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1427 | Willow | Salix sp. | 6 | 6 | 113 | 20 | 1257 | 2513 | Poor | Poor | Suppressed. | Protect |
| 1428 | Willow | Salix sp. | 9 | 8 | 201 | 20 | 1257 | 2513 | Good | Good |  | Protect |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1429 | Willow | Salix sp. | 8 | 12 | 452 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1430 | Willow | Salix sp. | 7 | 12 | 452 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1441 | Willow | Salix sp. | 16 | 11 | 380 | 20 | 1257 | 2513 | Poor | Poor | 5 stems 4,4,4,6,13. Trunk decay. | Protect |
| 1442 | Bigleaf Maple | Acer macrophyllum | 14 | 10 | 314 | 25 | 1963 | 3927 | Poor | Poor | Dead top. 2 stems 8,12. | Protect |
| 1443 | Douglas Fir | Pseudotsuga menziesii | 14 | 9 | 254 | 20 | 1257 | 2513 | Poor | Poor | Thin crown. | Protect |
| 1444 | Douglas Fir | Pseudotsuga menziesii | 20 | 16 | 804 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1445 | Willow | Salix sp. | 6 | 7 | 154 | 20 | 1257 | 2513 | Fair | Good | Trunk cavity. | Protect |
| 1446 | Bigleaf Maple | Acer macrophyllum | 6 | 11 | 380 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1447 | Douglas Fir | Pseudotsuga menziesii | 8 | 12 | 452 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1448 | Douglas Fir | Pseudotsuga menziesii | 13 | 14 | 616 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1449 | Douglas Fir | Pseudotsuga menziesii | 7 | 8 | 201 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1450 | Willow | Salix sp. | 6 | 4 | 50 | 20 | 1257 | 2513 | Poor | Poor | Suppressed. | Protect |
| 1451 | Douglas Fir | Pseudotsuga menziesii | 11 | 10 | 314 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1452 | Douglas Fir | Pseudotsuga menziesii | 11 | 12 | 452 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1453 | Willow | Salix sp. | 6 | 4 | 50 | 20 | 1257 | 2513 | Poor | Poor | Broken top. | Protect |
| 1454 | Sweet Cherry | Prunus avium | 7 | 4 | 50 | 15 | 707 | 1414 | Poor | Poor | Broken top. | Protect |
| 1455 | Sweet Cherry | Prunus avium | 6 | 5 | 79 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1456 | Douglas Fir | Pseudotsuga menziesii | 12 | 15 | 707 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1457 | Douglas Fir | Pseudotsuga menziesii | 17 | 16 | 804 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1458 | Willow | Salix sp. | 6 | 8 | 201 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1459 | Willow | Salix sp. | 6 | 9 | 254 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1460 | Willow | Salix sp. | 6 | 6 | 113 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1461 | Willow | Salix sp. | 6 | 6 | 113 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1462 | Oregon Ash | Fraxinus latifolia | 11 | 12 | 452 | 15 | 707 | 1414 | Good | Good | 2 stems 7,8. | Protect |
| 1463 | Oregon Ash | Fraxinus latifolia | 7 | 9 | 254 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1464 | Oregon Ash | Fraxinus latifolia | 8 | 14 | 616 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1465 | Willow | Salix sp. | 6 | 7 | 154 | 20 | 1257 | 2513 | Poor | Poor | Broken top. | Protect |
| 1466 | Willow | Salix sp. | 6 | 10 | 314 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1467 | Bigleaf Maple | Acer macrophyllum | 7 | 6 | 113 | 25 | 1963 | 3927 | Poor | Poor | Broken top. | Protect |
| 1468 | Bigleaf Maple | Acer macrophyllum | 6 | 4 | 50 | 25 | 1963 | 3927 | Poor | Poor | Broken top. | Protect |
| 1469 | Bigleaf Maple | Acer macrophyllum | 8 | 5 | 79 | 25 | 1963 | 3927 | Poor | Poor | Dead top. | Protect |
| 1470 | Bigleaf Maple | Acer macrophyllum | 13 | 16 | 804 | 25 | 1963 | 3927 | Poor | Poor | Thin crown. | Protect |
| 1471 | Bigleaf Maple | Acer macrophyllum | 10 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1472 | Bigleaf Maple | Acer macrophyllum | 15 | 11 | 380 | 25 | 1963 | 3927 | Poor | Poor | Thin crown. | Protect |
| 1473 | Bigleaf Maple | Ace | 7 | 10 | 314 | 25 | 1963 | 927 | Poor | Poor | Dead top. | Protect |

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| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1474 | Bigleaf Maple | Acer macrophyllum | 7 | 7 | 154 | 25 | 1963 | 3927 | Poor | Poor | Thin crown. | Protect |
| 1475 | Oregon White Oak | Quercus garrayana | 12 | 9 | 254 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1476 | Douglas Fir | Pseudotsuga menziesii | 16 | 9 | 254 | 20 | 1257 | 2513 | Poor | Poor | Broken top | Protect |
| 1477 | Pacific Madrone | Arbutus menziesii | 8 | 13 | 531 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1478 | Pacific Madrone | Arbutus menziesii | 9 | 12 | 452 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1479 | Bigleaf Maple | Acer macrophyllum | 9 | 14 | 616 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1480 | Oregon Ash | Fraxinus latifolia | 12 | 12 | 452 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1481 | Sweet Cherry | Prunus avium | 8 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1481.1 | Sweet Cherry | Prunus avium | 10 | 10 | 314 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1482 | Sweet Cherry | Prunus avium | 11 | 12 | 452 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1483 | Sweet Cherry | Prunus avium | 9 | 10 | 314 | 15 | 707 | 1414 | Good | Good | 2 stems 6,7. | Protect |
| 1484 | Sweet Cherry | Prunus avium | 6 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1485 | Sweet Cherry | Prunus avium | 6 | 4 | 50 | 15 | 707 | 1414 | Poor | Poor | Suppressed. | Protect |
| 1486 | Douglas Fir | Pseudotsuga menziesii | 9 |  | 0 |  | 0 | 0 |  |  |  | Protect |
| 1487 | Douglas Fir | Pseudotsuga menziesii | 11 | 8 | 201 | 20 | 1257 | 2513 | Poor | Poor | Suppressed. | Protect |
| 1488 | Oregon White Oak | Quercus garrayana | 22 | 25 | 1963 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1489 | Sweet Cherry | Prunus avium | 16 | 25 | 1963 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1490 | Oregon White Oak | Quercus garrayana | 10 | 8 | 201 | 25 | 1963 | 3927 | Good | Good | 2 stems 6,8. | Protect |
| 1491 | Douglas Fir | Pseudotsuga menziesii | 8 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1492 | Willow | Salix sp. | 6 | 5 | 79 | 20 | 1257 | 2513 | Poor | Poor | Trunk decay. | Protect |
| 1493 | Oregon White Oak | Quercus garrayana | 14 | 14 | 616 | 25 | 1963 | 3927 | Good | Good | 5 stems 6,6,6,7,7. | Protect |
| 1494 | Oregon White Oak | Quercus garrayana | 26 | 35 | 3848 | 35 | 3848 | 7697 | Good | Good | 2 stems 13,23. | Protect |
| 1495 | Oregon White Oak | Quercus garrayana | 13 | 4 | 50 | 25 | 1963 | 3927 | Poor | Poor | 3 stems 7,8,8. Broken tops | Protect |
| 1496 | Sweet Cherry | Prunus avium | 6 | 5 | 79 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1497 | Oregon White Oak | Quercus garrayana | 14 | 12 | 452 | 25 | 1963 | 3927 | Fair | Fair | Epicormic sprouts. | Protect |
| 1498 | Sweet Cherry | Prunus avium | 11 | 13 | 531 | 15 | 707 | 1414 | Good | Good | 2 stems 8,8. | Protect |
| 1499 | Sweet Cherry | Prunus avium | 11 | 13 | 531 | 15 | 707 | 1414 | Good | Good | 2 stems 6,7. | Protect |
| 1500 | Douglas Fir | Pseudotsuga menziesii | 12 | 15 | 707 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1501 | Oregon White Oak | Quercus garrayana | 13 | 12 | 452 | 25 | 1963 | 3927 | Good | Good | 4 stems 5,6,7,7. | Protect |
| 1502 | Oregon White Oak | Quercus garrayana | 8 | 6 | 113 | 25 | 1963 | 3927 | Good | Good | 2 stems 6,6. | Protect |
| 1503 | Sweet Cherry | Prunus avium | 6 | 9 | 254 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1504 | Sweet Cherry | Prunus avium | 7 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1505 | Oregon White Oak | Quercus garrayana | 32 | 23 | 1662 | 25 | 1963 | 3927 | Good | Good | 2 stems 16,28. | Protect |
| 1506 | Oregon Ash | Fraxinus latifolia | 18 | 17 | 908 | 17 | 908 | 1816 | Good | Good |  | Protect |
| 1507 | Oregon Ash | Fraxinus latifolia | 18 | 12 | 452 | 15 | 707 | 1414 | Poor | Poor | 7 stems 5,5,6,6,7,7,11. Broken top | Protect |
| 1508 | Oregon Ash | Fraxinus latifolia | 7 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1509 | Oregon Ash | Fraxinus latifolia | 12 | 9 | 254 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1510 | Oregon Ash | Fraxinus latifolia | 17 | 10 | 314 | 15 | 707 | 1414 | Poor | Poor | Broken top. | Protect |
| 1511 | Oregon Ash | Fraxinus latifolia | 19 | 13 | 531 | 15 | 707 | 1414 | Good | Good |  | Protect |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1512 | Oregon Ash | Fraxinus latifolia | 13 | 9 | 254 | 15 | 707 | 1414 | Good | Good | 2 stems 6,12. | Protect |
| 1513 | Oregon Ash | Fraxinus latifolia | 15 | 12 | 452 | 15 | 707 | 1414 | Fair | Fair | Broken stem. | Protect |
| 1514 | Oregon Ash | Fraxinus latifolia | 18 | 13 | 531 | 15 | 707 | 1414 | Good | Good | 2 stems 7,17 | Protect |
| 1515 | Oregon Ash | Fraxinus latifolia | 16 | 14 | 616 | 15 | 707 | 1414 | Fair | Fair | Trunk decay. | Protect |
| 1516 | Oregon Ash | Fraxinus latifolia | 16 | 18 | 1018 | 18 | 1018 | 2036 | Good | Good |  | Protect |
| 1517 | Oregon Ash | Fraxinus latifolia | 17 | 12 | 452 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1518 | Oregon Ash | Fraxinus latifolia | 16 | 9 | 254 | 15 | 707 | 1414 | Poor | Poor | Broken top. | Protect |
| 1519 | Oregon Ash | Fraxinus latifolia | 9 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1520 | Oregon Ash | Fraxinus Iatifolia | 28 | 19 | 1134 | 19 | 1134 | 2268 | Poor | Poor | 2 stems 11,26. Broken decay. Broken tops | Protect |
| 1521 | Oregon Ash | Fraxinus latifolia | 22 | 15 | 707 | 15 | 707 | 1414 | Poor | Poor | Broken top. | Protect |
| 1522 | Douglas Fir | Pseudotsuga menziesii | 22 | 13 | 531 | 20 | 1257 | 2513 | Poor | Poor | Dead top. | Protect |
| 1523 | Oregon White Oak | Quercus garrayana | 25 | 23 | 1662 | 25 | 1963 | 3927 | Good | Good | 3 stems 6,15,19. | Protect |
| 1524 | Douglas Fir | Pseudotsuga menziesii | 15 | 14 | 616 | 20 | 1257 | 2513 | Fair | Fair | Thin crown. | Protect |
| 1525 | Pacific Madrone | Arbutus menziesii | 7 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1526 | Pacific Madrone | Arbutus menziesii | 10 | 8 | 201 | 15 | 707 | 1414 | Fair | Fair | Trunk decay. | Protect |
| 1527 | Oregon Ash | Fraxinus latifolia | 7 | 9 | 254 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1542 | Willow | Salix sp. | 7 | 8 | 201 | 20 | 1257 | 2513 | Poor | Poor | Trunk decay | Protect |
| 1543 | Oregon White Oak | Quercus garrayana | 8 | 8 | 201 | 25 | 1963 | 3927 | Good | Good | 2 stems 6,6. | Protect |
| 1544 | Pacific Madrone | Arbutus menziesii | 15 | 14 | 616 | 15 | 707 | 1414 | Good | Good | 3 stems 9,10,11. | Protect |
| 1545 | Pacific Madrone | Arbutus menziesii | 17 | 14 | 616 | 15 | 707 | 1414 | Good | Good | 8 stems 5,5,5,5,6,6,8,I. | Protect |
| 1548 | Pacific Madrone | Arbutus menziesii | 10 | 12 | 452 | 15 | 707 | 1414 | Good | Good | 3 stems 5,6,7. | Protect |
| 1558 | Sweet Cherry | Prunus avium | 11 | 10 | 314 | 15 | 707 | 1414 | Good | Good | 3 stems 4,7,8, | Protect |
| 1559 | Sweet Cherry | Prunus avium | 7 | 4 | 50 | 15 | 707 | 1414 | Poor | Poor | Partial uproot | Protect |
| 1560 | Sweet Cherry | Prunus avium | 7 | 6 | 113 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1561 | Douglas Fir | Pseudotsuga menziesii | 17 | 16 | 804 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1562 | Douglas Fir | Pseudotsuga menziesii | 16 | 16 | 804 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1563 | Willow | Salix sp. | 6 | 5 | 79 | 20 | 1257 | 2513 | Poor | Poor | Suppressed. | Protect |
| 1564 | Willow | Salix sp. | 8 | 12 | 452 | 20 | 1257 | 2513 | Good | Good | 2 stems 6,6. | Protect |
| 1565 | Willow | Salix sp. | 8 | 8 | 201 | 20 | 1257 | 2513 | Poor | Poor | Crown die back. Trunk cavity | Protect |
| 1566 | Willow | Salix sp. | 6 | 6 | 113 | 20 | 1257 | 2513 | Poor | Poor | Crown die back | Protect |
| 1567 | Willow | Salix sp. | 17 | 18 | 1018 | 20 | 1257 | 2513 | Good | Good | 2 stems 12,12. | Protect |
| 1568 | Douglas Fir | Pseudotsuga menziesii | 13 | 14 | 616 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1569 | Willow | Salix sp. | 11 | 9 | 254 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1570 | Sweet Cherry | Prunus avium | 9 | 12 | 452 | 15 | 707 | 1414 | Good | Good | 2 stems 6,8. | Protect |
| 1571 | Willow | Salix sp. | 8 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1572 | Willow | Salix sp. | 10 | 7 | 154 | 20 | 1257 | 2513 | Poor | Poor | 2 stems 6,7. Crown die back | Protect |
| 1573 | Willow | Salix sp. | 12 | 14 | 616 | 20 | 1257 | 2513 | Good | Good | 2 stems 8,9. | Protect |
| 1574 | Willow | Salix sp. | 8 | 11 | 380 | 20 | 1257 | 2513 | Good | Good | 2 stems 6,6. | Protect |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1575 | Bigleaf Maple | Acer macrophyllum | 18 | 20 | 1257 | 25 | 1963 | 3927 | Good | Good | 4 stems 7,8,10,11. | Protect |
| 1576 | Oregon White Oak | Quercus garrayana | 6 | 9 | 254 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1577 | Oregon White Oak | Quercus garrayana | 6 | 8 | 201 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1578 | Pacific Madrone | Arbutus menziesii | 15 | 15 | 707 | 15 | 707 | 1414 | Good | Good | 4 stems 5,5,9,9. | Protect |
| 1579 | Pacific Madrone | Arbutus menziesii | 12 | 19 | 1134 | 15 | 707 | 1414 | Good | Good | 4 stems 4,6,6,7. | Protect |
| 1580 | Oregon White Oak | Quercus garrayana | 8 | 11 | 380 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1581 | Pacific Madrone | Arbutus menziesii | 7 | 11 | 380 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1582 | Willow | Salix sp. | 13 | 13 | 531 | 20 | 1257 | 2513 | Poor | Poor | 2 stems 9,9. Trunk decay. History of limb failure | Protect |
| 1583 | Oregon White Oak | Quercus garrayana | 6 | 11 | 380 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1584 | Oregon White Oak | Quercus garrayana | 10 | 13 | 531 | 25 | 1963 | 3927 | Good | Good | 4 stems 6,5,4,4. | Protect |
| 1586 | Oregon White Oak | Quercus garrayana | 13 | 14 | 616 | 25 | 1963 | 3927 | Good | Good | 3 stems 6,7,8. | Protect |
| 1587 | Pacific Madrone | Arbutus menziesii | 7 | 14 | 616 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1588 | Pacific Madrone | Arbutus menziesii | 7 | 11 | 380 | 15 | 707 | 1414 | Good | Fair | Severe lean. | Protect |
| 1589 | Douglas Fir | Pseudotsuga menziesii | 13 | 13 | 531 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1590 | Douglas Fir | Pseudotsuga menziesii | 15 | 16 | 804 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1591 | Sweet Cherry | Prunus avium | 7 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1592 | Sweet Cherry | Prunus avium | 7 | 9 | 254 | 15 | 707 | 1414 | Good | Good | 2 stems 6,4. | Protect |
| 1593 | Willow | Salix sp. | 11 | 14 | 616 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1594 | Willow | Salix sp. | 10 | 9 | 254 | 20 | 1257 | 2513 | Good | Good | 3 stems 4,6,7. | Protect |
| 1595 | Bigleaf Maple | Acer macrophyllum | 8 | 6 | 113 | 25 | 1963 | 3927 | Good | Good | 2 stems 6,6. | Protect |
| 1596 | Willow | Salix sp. | 6 | 5 | 79 | 20 | 1257 | 2513 | Poor | Poor | 2 stems 6,5. Trunk decay | Protect |
| 1597 | Douglas Fir | Pseudotsuga menziesii | 13 | 12 | 452 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1598 | Willow | Salix sp. | 11 | 9 | 254 | 20 | 1257 | 2513 | Poor | Poor | Trunk decay. 2 stems 7,8. | Protect |
| 1599 | Willow | Salix sp. | 6 | 8 | 201 | 20 | 1257 | 2513 | Poor | Poor | Trunk decay | Protect |
| 1600 | Sweet Cherry | Prunus avium | 6 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1601 | Sweet Cherry | Prunus avium | 8 | 12 | 452 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1602 | Willow | Salix sp. | 6 | 9 | 254 | 20 | 1257 | 2513 | Fair | Fair | Trunk lesions | Protect |
| 1603 | Douglas Fir | Pseudotsuga menziesii | 11 | 8 | 201 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1604 | Douglas Fir | Pseudotsuga menziesii | 6 | 5 | 79 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1605 | Sweet Cherry | Prunus avium | 9 | 12 | 452 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1606 | Willow | Salix sp. | 6 | 11 | 380 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1606.1 | Willow | Salix sp. | 9 | 9 | 254 | 20 | 1257 | 2513 | Good | Good | Stems 5,5,5. | Protect |
| 1607 | Douglas Fir | Pseudotsuga menziesii | 10 | 10 | 314 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1608 | Douglas Fir | Pseudotsuga menziesii | 9 | 8 | 201 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1609 | Douglas Fir | Pseudotsuga menziesii | 11 | 14 | 616 | 20 | 1257 | 2513 | Good | Good |  | Protect |

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arboricultural Consultants
Attachment 2

| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1610 | Willow | Salix sp. | 7 | 6 | 113 | 20 | 1257 | 2513 | Poor | Poor | Suppressed | Protect |
| 1611 | Douglas Fir | Pseudotsuga menziesii | 14 | 14 | 616 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1612 | Willow | Salix sp. | 6 | 7 | 154 | 20 | 1257 | 2513 | Poor | Poor | Dead top | Protect |
| 1613 | Pacific Madrone | Arbutus menziesii | 13 | 12 | 452 | 15 | 707 | 1414 | Good | Good | 3 stems 7,8,8. | Protect |
| 1614 | Oregon White Oak | Quercus garrayana | 12 | 18 | 1018 | 25 | 1963 | 3927 | Good | Good |  | Protect |
| 1615 | Pacific Madrone | Arbutus menziesii | 18 | 17 | 908 | 15 | 707 | 1414 | Good | Good | 3 stems 10,10,11. | Protect |
| 1616 | Sweet Cherry | Prunus avium | 6 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1617 | Sweet Cherry | Prunus avium | 7 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1618 | Sweet Cherry | Prunus avium | 8 | 9 | 254 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1619 | Willow | Salix sp. | 7 | 10 | 314 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1620 | Sweet Cherry | Prunus avium | 13 | 18 | 1018 | 18 | 1018 | 2036 | Good | Good |  | Protect |
| 1621 | Willow | Salix sp. | 11 | 9 | 254 | 20 | 1257 | 2513 | Good | Good | 2 stems 8,8. | Protect |
| 1622 | Willow | Salix sp. | 6 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 1623 | Willow | Salix sp. | 8 | 9 | 254 | 20 | 1257 | 2513 | Good | Good | 2 stems 4,7. | Protect |
| 1624 | Oregon Ash | Fraxinus latifolia | 6 | 6 | 113 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1625 | Sweet Cherry | Prunus avium | 6 | 8 | 201 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1626 | Sweet Cherry | Prunus avium | 6 | 7 | 154 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1627 | Willow | Salix sp. | 6 | 4 | 50 | 20 | 1257 | 2513 | Poor | Poor | Trunk decay | Protect |
| 1628 | Willow | Salix sp. | 8 | 11 | 380 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1629 | Willow | Salix sp. | 12 | 12 | 452 | 20 | 1257 | 2513 | Good | Good | 3 stems 6,7,7. | Protect |
| 1630 | Willow | Salix sp. | 9 | 10 | 314 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1631 | Sweet Cherry | Prunus avium | 8 | 10 | 314 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1632 | Willow | Salix sp. | 8 | 9 | 254 | 20 | 1257 | 2513 | Good | Good | 2 stems 6,6. | Protect |
| 1633 | Willow | Salix sp. | 12 | 14 | 616 | 20 | 1257 | 2513 | Good | Good | 2 stems 7,10. | Protect |
| 1634 | Willow | Salix sp. | 9 | 12 | 452 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1635 | Willow | Salix sp. | 8 | 10 | 314 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 1636 | Douglas Fir | Pseudotsuga menziesii | 12 | 9 | 254 | 20 | 1257 | 2513 | Poor | Poor | Thin crown | Protect |
| 1637 | Willow | Salix sp. | 6 | 6 | 113 | 20 | 1257 | 2513 | Fair | Fair |  | Protect |
| 15229 | Pacific Madrone | Arbutus menziesii | 6 | 6 | 113 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 15391 | Oregon Ash | Oregon Ash | 21 | 15 | 707 | 15 | 707 | 0 | Good | Good | 2 stems 13,16. Offsite. | Protect |
| 15397 | Oregon Ash | Fraxinus latifolia | 22 | 19 | 1134 | 15 | 707 | 0 | Good | Good | 2 stems 6,20. Offsite. | Protect |
| 15398 | Oregon Ash | Fraxinus latifolia | 21 | 20 | 1257 | 15 | 707 | 0 | Good | Good | Offsite. | Protect |
| 15403 | Oregon Ash | Fraxinus latifolia | 14 | 10 | 314 | 15 | 707 | 0 | Poor | Poor | Severe trunk decay. Offsite. | Protect |
| 15452 | Oregon Ash | Fraxinus latifolia | 18 | 14 | 616 | 15 | 707 | 0 | Good | Good | Offsite. | Protect |
| 15455 | Oregon Ash | Fraxinus latifolia | 7 | 3 | 28 | 15 | 707 | 1414 | Poor | Poor | Suppressed. | Protect |
| 15498 | Oregon Ash | Fraxinus latifolia | 12 |  | 0 |  | 0 | 0 | Dead |  |  | Protect |
| 15499 | Oregon Ash | Fraxinus latifolia | 20 | 15 | 707 | 15 | 707 | 0 | Good | Good | Offsite. | Protect |
| 15500 | Oregon Ash | Fraxinus latifolia | 16 | 12 | 452 | 15 | 707 | 0 | Fair | Fair | History of large limb failure. Offsite. | Protect |
| 15603 | Douglas Fir | Pseudotsuga menziesii | 7 | 7 | 154 | 20 | 1257 | 2513 | Good | Good |  | Protect |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 15603 | Douglas Fir | Pseudotsuga menziesii | 7 | 7 | 154 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 15639 | Douglas Fir | Pseudotsuga menziesii | 17 | 13 | 531 | 20 | 1257 | 2513 | Good | Good |  | Protect |
| 15697 | Pacific Madrone | Arbutus menziesii | 7 | 9 | 254 | 15 | 707 | 1414 | Good | Good |  | Protect |
| 1046 | Oregon Ash | Fraxinus latifolia | 35 | 38 | 4536 | 15 | 707 | 0 | Good | Good | 6 stems 12,13,13,15,15,16. | Remove |
| 1047 | Sweet Cherry | Prunus avium | 11 | 5 | 79 | 15 | 707 | 0 | Good | Good | 2 stems 6,9. | Remove |
| 1048 | Oregon Ash | Fraxinus latifolia | 8 | 13 | 531 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1049 | Oregon Ash | Fraxinus latifolia | 10 | 13 | 531 | 15 | 707 | 0 | Good | Poor | 2 stems 7,7. Topped. | Remove |
| 1054 | Oregon Ash | Fraxinus latifolia | 7 | 10 | 314 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1055 | Oregon Ash | Fraxinus latifolia | 6 | 10 | 314 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1056 | Oregon Ash | Fraxinus latifolia | 7 | 11 | 380 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1057 | Oregon Ash | Fraxinus latifolia | 6 | 13 | 531 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1058 | European White Birch | Betula pendula | 11 | 10 | 314 | 12 | 452 | 0 | Good | Good |  | Remove |
| 1059 | European White Birch | Betula pendula | 20 | 12 | 452 | 12 | 452 | 0 | Good | Good | 4 stems 6,8,11,13 | Remove |
| 1060 | Common Hawthorn | Crataegus monogyna | 8 | 9 | 254 | 9 | 254 | 0 | Good | Good |  | Remove |
| 1061 | Pacific Madrone | Arbutus menziesii | 21 | 22 | 1521 | 22 | 1521 | 0 | Good | Good |  | Remove |
| 1062 | Pacific Madrone | Arbutus menziesii | 8 | 10 | 314 | 15 | 707 | 0 | Good | Fair | Severe lean north | Remove |
| 1063 | Douglas Fir | Pseudotsuga menziesii | 32 | 24 | 1810 | 24 | 1810 | 0 | Good | Good |  | Remove |
| 1064 | Pacific Madrone | Arbutus menziesii | 11 | 10 | 314 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1065 | Pacific Madrone | Arbutus menziesii | 14 | 13 | 531 | 15 | 707 | 0 | Good | Good | 2 stems 8,13. | Remove |
| 1066 | Pacific Madrone | Arbutus menziesii | 18 | 16 | 804 | 16 | 804 | 0 | Good | Good | 2 stems 13,13. | Remove |
| 1067 | Douglas Fir | Pseudotsuga menziesii | 18 | 16 | 804 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1068 | Pacific Madrone | Arbutus menziesii | 7 | 6 | 113 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1069 | Sweet Cherry | Prunus avium | 6 | 9 | 254 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1070 | Oregon White Oak | Quercus garrayana | 19 | 15 | 707 | 25 | 1963 | 0 | Good | Good |  | Remove |
| 1071 | Pacific Madrone | Arbutus menziesii | 14 | 14 | 616 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1072 | Douglas Fir | Pseudotsuga menziesii | 11 | 10 | 314 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1073 | Douglas Fir | Pseudotsuga menziesii | 8 | 9 | 254 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1074 | Douglas Fir | Pseudotsuga menziesii | 10 | 12 | 452 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1075 | Douglas Fir | Pseudotsuga menziesii | 12 | 0 | 0 | 20 | 1257 | 0 | Dead |  |  | Remove |
| 1076 | Pacific Madrone | Arbutus menziesii | 11 | 15 | 707 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1077 | Cascara | Rhamnus purshiana | 6 | 4 | 50 | 12 | 452 | 0 | Good | Good |  | Remove |
| 1078 | Douglas Fir | Pseudotsuga menziesii | 21 | 14 | 616 | 20 | 1257 | 0 | Good | Good |  | Remove |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1079 | Douglas Fir | Pseudotsuga menziesii | 38 | 20 | 1257 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1080 | Sweet Cherry | Prunus avium | 6 | 8 | 201 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1081 | Douglas Fir | Pseudotsuga menziesii | 24 | 15 | 707 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1082 | Douglas Fir | Pseudotsuga menziesii | 11 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1083 | Douglas Fir | Pseudotsuga menziesii | 32 | 18 | 1018 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1084 | Douglas Fir | Pseudotsuga menziesii | 31 | 15 | 707 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1085 | Sweet Cherry | Prunus avium | 6 | 6 | 113 | 15 | 707 | 0 | Poor | Poor | Broken top. | Remove |
| 1086 | Oregon White Oak | Quercus garrayana | 15 | 15 | 707 | 25 | 1963 | 0 | Good | Good |  | Remove |
| 1087 | Douglas Fir | Pseudotsuga menziesii | 28 | 26 | 2124 | 26 | 2124 | 0 | Good | Good |  | Remove |
| 1088 | Sweet Cherry | Prunus avium | 6 | 5 | 79 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1089 | Sweet Cherry | Prunus avium | 10 | 8 | 201 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1090 | Sweet Cherry | Prunus avium | 13 | 10 | 314 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1091 | Douglas Fir | Pseudotsuga menziesii | 15 | 15 | 707 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1092 | Pacific Madrone | Arbutus menziesii | 11 | 14 | 616 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1093 | Sweet Cherry | Prunus avium | 11 | 9 | 254 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1094 | Sweet Cherry | Prunus avium | 22 | 12 | 452 | 15 | 707 | 0 | Poor | Poor | Lost tops. History of large limb failure. | Remove |
| 1095 | Sweet Cherry | Prunus avium | 13 | 10 | 314 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1096 | Sweet Cherry | Prunus avium | 7 | 5 | 79 | 15 | 707 | 0 | Poor | Poor | Suppressed | Remove |
| 1097 | Douglas Fir | Pseudotsuga menziesii | 37 | 17 | 908 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1098 | Pacific Madrone | Arbutus menziesii | 13 | 12 | 452 | 15 | 707 | 0 | Good | Good | 2 stems 6,12. | Remove |
| 1099 | Pacific Madrone | Arbutus menziesii | 8 | 7 | 154 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1100 | Douglas Fir | Pseudotsuga menziesii | 24 | 10 | 314 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1101 | Douglas Fir | Pseudotsuga menziesii | 20 | 18 | 1018 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1101.1 | Sweet Cherry | Prunus avium | 7 | 6 | 113 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1102 | Douglas Fir | Pseudotsuga menziesii | 17 | 15 | 707 | 20 | 1257 | 0 | Poor | Good | Red Ring Rot conk. | Remove |

[^33]| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1103 | Douglas Fir | Pseudotsuga menziesii | 24 | 18 | 1018 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1104 | Douglas Fir | Pseudotsuga menziesii | 9 | 8 | 201 | 20 | 1257 | 0 | Fair | Fair | Thin crown. | Remove |
| 1105 | Elderberry | Sambucus sp. | 12 | 11 | 380 | 11 | 380 | 0 | Good | Good |  | Remove |
| 1106 | Sweet Cherry | Prunus avium | 8 | 14 | 616 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1107 | Sweet Cherry | Prunus avium | 8 | 10 | 314 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1108 | Oregon White Oak | Quercus garrayana | 14 | 10 | 314 | 25 | 1963 | 0 | Good | Good |  | Remove |
| 1109 | Douglas Fir | Pseudotsuga menziesii | 28 | 11 | 380 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1110 | Pacific Madrone | Arbutus menziesii | 13 | 5 | 79 | 15 | 707 | 0 | Good | Good | 2 stems 9,9. | Remove |
| 1111 | Pacific Madrone | Arbutus menziesii | 9 | 7 | 154 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1112 | Douglas Fir | Pseudotsuga menziesii | 28 | 15 | 707 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1113 | Douglas Fir | Pseudotsuga menziesii | 28 | 20 | 1257 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1114 | Douglas Fir | Pseudotsuga menziesii | 11 | 6 | 113 | 20 | 1257 | 0 | Poor | Poor | Suppressed | Remove |
| 1115 | Pacific Madrone | Arbutus menziesii | 8 | 5 | 79 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1116 | Douglas Fir | Pseudotsuga menziesii | 20 | 20 | 1257 | 20 | 1257 | 0 | Good | Good | 2 stems 14,15. | Remove |
| 1117 | Pacific Madrone | Arbutus menziesii | 6 | 5 | 79 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1118 | Douglas Fir | Pseudotsuga menziesii | 20 | 15 | 707 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1119 | Pacific Madrone | Arbutus menziesii | 15 | 12 | 452 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1120 | Sweet Cherry | Prunus avium | 9 | 7 | 154 | 15 | 707 | 0 | Poor | Poor | Suppressed | Remove |
| 1121 | Sweet Cherry | Prunus avium | 12 | 10 | 314 | 15 | 707 | 0 | Fair | Fair |  | Remove |
| 1122 | Sweet Cherry | Prunus avium | 6 | 6 | 113 | 15 | 707 | 0 | Poor | Poor | Suppressed | Remove |
| 1123 | Sweet Cherry | Prunus avium | 9 | 8 | 201 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1124 | Douglas Fir | Pseudotsuga menziesii | 26 | 23 | 1662 | 23 | 1662 | 0 | Good | Good |  | Remove |
| 1125 | Pacific Madrone | Arbutus menziesii | 6 | 3 | 28 | 15 | 707 | 0 | Poor | Poor | Broken top. | Remove |
| 1126 | Sweet Cherry | Prunus avium | 6 | 6 | 113 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1127 | Sweet Cherry | Prunus avium | 7 | 8 | 201 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1128 | Sweet Cherry | Prunus avium | 7 | 10 | 314 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1129 | Douglas Fir | Pseudotsuga menziesii | 26 | 24 | 1810 | 24 | 1810 | 0 | Good | Good |  | Remove |
| 1130 | Sweet Cherry | Prunus avium | 6 | 8 | 201 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1131 | Pacific Madrone | Arbutus menziesii | 11 | 11 | 380 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1132 | Pacific Madrone | Arbutus menziesii | 12 | 12 | 452 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1133 | Pacific Madrone | Arbutus menziesii | 10 | 15 | 707 | 15 | 707 | 0 | Good | Good | 2 stems 6,8. | Remove |
| 1134 | Pacific Madrone | Arbutus menziesii | 7 | 6 | 113 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1135 | Douglas Fir | Pseudotsuga menziesii | 25 | 16 | 804 | 20 | 1257 | 0 | Good | Good |  | Remove |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1136 | Douglas Fir | Pseudotsuga menziesii | 15 | 17 | 908 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1137 | Douglas Fir | Pseudotsuga menziesii | 13 | 10 | 314 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1138 | Douglas Fir | Pseudotsuga menziesii | 9 | 4 | 50 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1139 | Douglas Fir | Pseudotsuga menziesii | 28 | 24 | 1810 | 24 | 1810 | 0 | Good | Good |  | Remove |
| 1140 | Pacific Madrone | Arbutus menziesii | 9 | 9 | 254 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1141 | Douglas Fir | Pseudotsuga menziesii | 23 | 18 | 1018 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1142 | Sweet Cherry | Prunus avium | 10 | 10 | 314 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1143 | Common Hawthorn | Crataegus monogyna | 10 | 9 | 254 | 9 | 254 | 0 | Good | Good |  | Remove |
| 1144 | Douglas Fir | Pseudotsuga menziesii | 15 |  | 0 |  | 0 | 0 | Dead |  |  | Remove |
| 1145 | Oregon White Oak | Quercus garrayana | 11 | 10 | 314 | 25 | 1963 | 0 | Good | Good | 2 stems 7,8. | Remove |
| 1146 | Douglas Fir | Pseudotsuga menziesii | 16 |  | 0 |  | 0 | 0 | Dead |  |  | Remove |
| 1147 | Pacific Madrone | Arbutus menziesii | 14 | 16 | 804 | 16 | 804 | 0 | Good | Good |  | Remove |
| 1148 | Douglas Fir | Pseudotsuga menziesii |  |  | 0 |  | 0 | 0 | Dead |  |  | Remove |
| 1149 | Douglas Fir | Pseudotsuga menziesii | 29 | 24 | 1810 | 24 | 1810 | 0 | Good | Good |  | Remove |
| 1150 | Pacific Madrone | Arbutus menziesii | 10 | 5 | 79 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1154 | Pacific Madrone | Arbutus menziesii | 13 | 14 | 616 | 15 | 707 | 0 | Good | Good | 2 stems 6,11. | Remove |
| 1155 | Pacific Madrone | Arbutus menziesii | 6 | 8 | 201 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1162 | Pacific Madrone | Arbutus menziesii | 7 | 6 | 113 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1163 | Pacific Madrone | Arbutus menziesii | 6 | 6 | 113 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1164 | Pacific Madrone | Arbutus menziesii | 6 | 9 | 254 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1165 | Pacific Madrone | Arbutus menziesii | 10 | 12 | 452 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1166 | Pacific Madrone | Arbutus menziesii | 6 | 6 | 113 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1167 | Douglas Fir | Pseudotsuga menziesii | 12 | 8 | 201 | 20 | 1257 | 0 | Poor | Poor | Dead top. | Remove |
| 1169 | Oregon White Oak | Quercus garrayana | 7 | 7 | 154 | 25 | 1963 | 0 | Good | Good |  | Remove |
| 1186 | Pacific Madrone | Arbutus menziesii | 13 | 15 | 707 | 15 | 707 | 0 | Good | Good | 4 stems 6,6,6,8. | Remove |
| 1187 | Pacific Madrone | Arbutus menziesii | 11 | 14 | 616 | 15 | 707 | 0 | Good | Good | 3 stems 6,6,7. | Remove |
| 1415 | Douglas Fir | Pseudotsuga menziesii | 10 | 13 | 531 | 20 | 1257 | 0 | Poor | Poor | Thin crown. | Remove |
| 1416 | Douglas Fir | Pseudotsuga menziesii | 10 | 12 | 452 | 20 | 1257 | 0 | Poor | Poor | Thin crown. | Remove |
| 1417 | Willow | Salix sp. | 9 | 12 | 452 | 20 | 1257 | 0 | Good | Good | 2 stems and | Remove |
| 1431 | Willow | Salix sp. | 6 | 5 | 79 | 20 | 1257 | 0 | Poor | Poor | Thin crown. | Remove |
| 1432 | Willow | Salix sp. | 7 | 5 | 79 | 20 | 1257 | 0 | Poor | Poor | History of limb loss. | Remove |
| 1433 | Willow | Salix sp. | 6 | 5 | 79 | 20 | 1257 | 0 | Poor | Poor | Thin crown. | Remove |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1434 | Willow | Salix sp. | 6 | 15 | 707 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1435 | Willow | Salix sp. | 8 | 12 | 452 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1436 | Willow | Salix sp. | 6 |  | 0 |  | 0 | 0 | Dead |  |  | Remove |
| 1437 | Douglas Fir | Pseudotsuga menziesii | 13 | 15 | 707 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1438 | Douglas Fir | Pseudotsuga menziesii | 9 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1439 | Sweet Cherry | Prunus avium | 7 | 9 | 254 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1440 | Sweet Cherry | Prunus avium | 10 | 12 | 452 | 15 | 707 | 0 | Good | Good | 2 stems 6,8. | Remove |
| 1528 | Oregon Ash | Fraxinus latifolia | 8 | 10 | 314 | 15 | 707 | 0 | Good | Good | 2 stems 6,6. | Remove |
| 1529 | Oregon Ash | Fraxinus latifolia | 8 | 12 | 452 | 15 | 707 | 0 | Good | Good | 2 stems 6,6. | Remove |
| 1530 | Oregon White Oak | Quercus garrayana | 11 | 14 | 616 | 25 | 1963 | 0 | Good | Good | 4 stems 6,6,5,5. | Remove |
| 1531 | Sweet Cherry | Prunus avium | 6 | 8 | 201 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1532 | Douglas Fir | Pseudotsuga menziesii | 12 | 14 | 616 | 20 | 1257 | 0 | Fair | Fair | Thin crown | Remove |
| 1533 | Douglas Fir | Pseudotsuga menziesii | 6 |  | 0 |  | 0 | 0 | Dead |  |  | Remove |
| 1534 | Willow | Salix sp. | 10 | 13 | 531 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1535 | Pacific Madrone | Arbutus menziesii | 10 | 11 | 380 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1536 | Pacific Madrone | Arbutus menziesii | 13 | 12 | 452 | 15 | 707 | 0 | Good | Good | 2 stems 9,11. | Remove |
| 1537 | Douglas Fir | Pseudotsuga menziesii | 18 | 17 | 908 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1538 | Oregon White Oak | Quercus garrayana | 8 | 9 | 254 | 25 | 1963 | 0 | Good | Good | 2 stems 6,6. | Remove |
| 1539 | Oregon White Oak | Quercus garrayana | 8 | 9 | 254 | 25 | 1963 | 0 | Good | Good | 2 stems 6,6. | Remove |
| 1540 | Pacific Madrone | Arbutus menziesii | 13 | 12 | 452 | 15 | 707 | 0 | Good | Good | 5 stems 5,5,6,6,7. | Remove |
| 1541 | Oregon White Oak | Quercus garrayana | 7 | 8 | 201 | 25 | 1963 | 0 | Good | Good |  | Remove |
| 1546 | Pacific Madrone | Arbutus menziesii | 11 | 13 | 531 | 15 | 707 | 0 | Good | Good | 3 stems 5,6,8. | Remove |
| 1547 | Pacific Madrone | Arbutus menziesii | 14 | 16 | 804 | 16 | 804 | 0 | Good | Good | 3 stems 6,9,9. | Remove |

[^34]| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1549 | Sweet Cherry | Prunus avium | 14 | 18 | 1018 | 18 | 1018 | 0 | Good | Good | 6 stems 4,4,5,6,7,7. | Remove |
| 1550 | Willow | Salix sp. | 7 |  | 0 |  | 0 | 0 | Dead |  |  | Remove |
| 1551 | Pacific Madrone | Arbutus menziesii | 10 | 10 | 314 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1552 | Pacific Madrone | Arbutus menziesii | 6 | 4 | 50 | 15 | 707 | 0 | Poor | Poor | Suppressed | Remove |
| 1553 | Pacific Madrone | Arbutus menziesii | 7 | 8 | 201 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1554 | Pacific Madrone | Arbutus menziesii | 13 | 14 | 616 | 15 | 707 | 0 | Good | Good | 4 stems 6,6,7,7. | Remove |
| 1555 | Willow | Salix sp. | 8 | 7 | 154 | 20 | 1257 | 0 | Poor | Poor |  | Remove |
| 1556 | Willow | Salix sp. | 6 | 7 | 154 | 20 | 1257 | 0 | Good | Good | Crown die back | Remove |
| 1557 | Willow | Salix sp. | 11 | 10 | 314 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1585 | Pacific Madrone | Arbutus menziesii | 14 | 15 | 707 | 15 | 707 | 0 | Good | Good | 4 stems 5,6,8,8. | Remove |
| 1638 | Douglas Fir | Pseudotsuga menziesii | 10 | 13 | 531 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1639 | Douglas Fir | Pseudotsuga menziesii | 6 | 15 | 707 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1640 | Douglas Fir | Pseudotsuga menziesii | 12 | 15 | 707 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1641 | Douglas Fir | Pseudotsuga menziesii | 12 | 12 | 452 | 20 | 1257 | 0 | Good | Good | 2 stems 7,10. | Remove |
| 1642 | Douglas Fir | Pseudotsuga menziesii | 10 | 15 | 707 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1643 | Douglas Fir | Pseudotsuga menziesii | 14 | 12 | 452 | 20 | 1257 | 0 | Good | Good | 2 stems 9,10. | Remove |
| 1644 | Douglas Fir | Pseudotsuga menziesii | 7 | 6 | 113 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1645 | Douglas Fir | Pseudotsuga menziesii | 13 | 14 | 616 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1646 | Douglas Fir | Pseudotsuga menziesii | 9 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1647 | Douglas Fir | Pseudotsuga menziesii | 18 | 13 | 531 | 20 | 1257 | 0 | Good | Good | 3 stems 8,11,12. | Remove |
| 1648 | Douglas Fir | Pseudotsuga menziesii | 13 | 13 | 531 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1649 | Douglas Fir | Pseudotsuga menziesii | 11 | 12 | 452 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1650 | Douglas Fir | Pseudotsuga menziesii | 9 | 6 | 113 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1651 | Douglas Fir | Pseudotsuga menziesii | 10 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1652 | Douglas Fir | Pseudotsuga menziesii | 7 | 5 | 79 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1653 | Douglas Fir | Pseudotsuga menziesii | 8 | 4 | 50 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1654 | Douglas Fir | Pseudotsuga menziesii | 11 | 12 | 452 | 20 | 1257 | 0 | Good | Good |  | Remove |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1655 | Douglas Fir | Pseudotsuga menziesii | 15 | 15 | 707 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1656 | Douglas Fir | Pseudotsuga menziesii | 9 | 6 | 113 | 20 | 1257 | 0 | Fair | Fair | Suppressed. | Remove |
| 1657 | Douglas Fir | Pseudotsuga menziesii | 11 | 9 | 254 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1658 | Douglas Fir | Pseudotsuga menziesii | 6 | 3 | 28 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1659 | Douglas Fir | Pseudotsuga menziesii | 11 | 10 | 314 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1660 | Douglas Fir | Pseudotsuga menziesii | 9 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1661 | Douglas Fir | Pseudotsuga menziesii | 6 | 3 | 28 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1662 | Douglas Fir | Pseudotsuga menziesii | 12 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1663 | Douglas Fir | Pseudotsuga menziesii | 10 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1664 | Douglas Fir | Pseudotsuga menziesii | 8 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1665 | Douglas Fir | Pseudotsuga menziesii | 8 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1666 | Douglas Fir | Pseudotsuga menziesii | 14 | 15 | 707 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1667 | Douglas Fir | Pseudotsuga menziesii | 8 | 4 | 50 | 20 | 1257 | 0 | Fair | Fair | Suppressed. | Remove |
| 1668 | Douglas Fir | Pseudotsuga menziesii | 8 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1669 | Douglas Fir | Pseudotsuga menziesii | 8 | 4 | 50 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1670 | Douglas Fir | Pseudotsuga menziesii | 8 | 6 | 113 | 20 | 1257 | 0 | Fair | Fair | Suppressed. | Remove |
| 1671 | Douglas Fir | Pseudotsuga menziesii | 9 | 9 | 254 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1672 | Douglas Fir | Pseudotsuga menziesii | 7 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1673 | Douglas Fir | Pseudotsuga menziesii | 6 | 6 | 113 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1674 | Douglas Fir | Pseudotsuga menziesii | 11 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1675 | Douglas Fir | Pseudotsuga menziesii | 9 | 7 | 154 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1676 | Douglas Fir | Pseudotsuga menziesii | 10 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1677 | Douglas Fir | Pseudotsuga menziesii | 7 | 3 | 28 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1678 | Douglas Fir | Pseudotsuga menziesii | 9 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1679 | Douglas Fir | Pseudotsuga menziesii | 12 | 10 | 314 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1680 | Douglas Fir | Pseudotsuga menziesii | 12 | 10 | 314 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1681 | Douglas Fir | Pseudotsuga menziesii | 8 | 5 | 79 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1682 | Douglas Fir | Pseudotsuga menziesii | 9 | 7 | 154 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1683 | Douglas Fir | Pseudotsuga menziesii | 16 | 13 | 531 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1684 | Douglas Fir | Pseudotsuga menziesii | 13 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1685 | Douglas Fir | Pseudotsuga menziesii | 6 | 3 | 28 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1686 | Douglas Fir | Pseudotsuga menziesii | 12 | 10 | 314 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1687 | Douglas Fir | Pseudotsuga menziesii | 7 | 3 | 28 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1688 | Douglas Fir | Pseudotsuga menziesii | 11 | 7 | 154 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1689 | Douglas Fir | Pseudotsuga menziesii | 9 | 6 | 113 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1690 | Douglas Fir | Pseudotsuga menziesii | 7 | 3 | 28 | 20 | 1257 | 0 | Fair | Fair | Suppressed. | Remove |
| 1691 | Douglas Fir | Pseudotsuga menziesii | 8 | 4 | 50 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1692 | Douglas Fir | Pseudotsuga menziesii | 6 | 3 | 28 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1693 | Douglas Fir | Pseudotsuga menziesii | 14 | 10 | 314 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1694 | Douglas Fir | Pseudotsuga menziesii | 6 | 3 | 28 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1695 | Black Cottonwood | Populus trichocarpa | 23 | 20 | 1257 | 20 | 1257 | 0 | good | Good | 2 stems 16,17. | Remove |
| 1696 | Douglas Fir | Pseudotsuga menziesii | 6 | 2 | 13 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1697 | Douglas Fir | Pseudotsuga menziesii | 6 | 2 | 13 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1698 | Douglas Fir | Pseudotsuga menziesii | 12 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1712 | Sweet Cherry | Prunus avium | 8 |  | 0 |  | 0 | 0 | Dead |  |  | Remove |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1713 | Douglas Fir | Pseudotsuga menziesii | 6 | 2 | 13 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1714 | Douglas Fir | Pseudotsuga menziesii | 11 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1715 | Douglas Fir | Pseudotsuga menziesii | 11 | 9 | 254 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1716 | Douglas Fir | Pseudotsuga menziesii | 10 | 4 | 50 | 20 | 1257 | 0 | Fair | Fair | Suppressed. | Remove |
| 1717 | Douglas Fir | Pseudotsuga menziesii | 6 |  | 0 |  | 0 | 0 | Dead |  |  | Remove |
| 1718 | Douglas Fir | Pseudotsuga menziesii | 8 | 6 | 113 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1719 | Douglas Fir | Pseudotsuga menziesii | 16 | 9 | 254 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1720 | Douglas Fir | Pseudotsuga menziesii | 10 | 9 | 254 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1721 | Douglas Fir | Pseudotsuga menziesii | 7 | 6 | 113 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1722 | Douglas Fir | Pseudotsuga menziesii | 6 | 3 | 28 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1723 | Black Cottonwood | Populus trichocarpa | 21 | 20 | 1257 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1724 | Pacific Madrone | Arbutus menziesii | 6 | 3 | 28 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1725 | Douglas Fir | Pseudotsuga menziesii | 12 | 9 | 254 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1726 | Black Cottonwood | Populus trichocarpa | 16 | 15 | 707 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1727 | Douglas Fir | Pseudotsuga menziesii | 11 | 10 | 314 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1728 | Douglas Fir | Pseudotsuga menziesii | 6 | 6 | 113 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1729 | Douglas Fir | Pseudotsuga menziesii | 6 | 5 | 79 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1730 | Black Cottonwood | Populus trichocarpa | 14 | 12 | 452 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1731 | Douglas Fir | Pseudotsuga menziesii | 6 | 3 | 28 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1732 | Black Cottonwood | Populus trichocarpa | 13 | 12 | 452 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1733 | Pacific Madrone | Arbutus menziesii | 6 | 5 | 79 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1733.1 | Pacific Madrone | Arbutus menziesii | 7 | 10 | 314 | 15 | 707 | 0 | Good | Good | 2 stems 5,5. | Remove |
| 1734 | Pacific Madrone | Arbutus menziesii | 8 | 8 | 201 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1735 | Pacific Madrone | Arbutus menziesii | 7 | 8 | 201 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1736 | Pacific Madrone | Arbutus menziesii | 6 | 5 | 79 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1737 | Douglas Fir | Pseudotsuga menziesii | 6 | 3 | 28 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1738 | Sweet Cherry | Prunus avium | 9 | 9 | 254 | 15 | 707 | 0 | Good | Good |  | Remove |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1739 | Douglas Fir | Pseudotsuga menziesii | 6 | 2 | 13 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1740 | Douglas Fir | Pseudotsuga menziesii | 7 | 6 | 113 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1741 | Douglas Fir | Pseudotsuga menziesii | 7 | 6 | 113 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1742 | Black Cottonwood | Populus trichocarpa | 8 | 6 | 113 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1743 | Pacific Madrone | Arbutus menziesii | 6 |  | 0 |  | 0 | 0 | Good | Good | 2 stems 6,0. | Remove |
| 1744 | Douglas Fir | Pseudotsuga menziesii | 6 | 3 | 28 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1745 | Douglas Fir | Pseudotsuga menziesii | 7 | 3 | 28 | 20 | 1257 | 0 | Poor | Poor | Thin crown. | Remove |
| 1746 | Black Cottonwood | Populus trichocarpa | 6 | 7 | 154 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1747 | Douglas Fir | Pseudotsuga menziesii | 9 | 9 | 254 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1748 | Black Cottonwood | Populus trichocarpa | 6 | 4 | 50 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1749 | Douglas Fir | Pseudotsuga menziesii | 11 | 9 | 254 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1750 | Black Cottonwood | Populus trichocarpa | 9 | 4 | 50 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1751 | Black Cottonwood | Populus trichocarpa | 8 | 7 | 154 | 20 | 1257 | 0 | Good | Good | 2 stems,6. | Remove |
| 1752 | Douglas Fir | Pseudotsuga menziesii | 8 | 4 | 50 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1753 | Douglas Fir | Pseudotsuga menziesii | 13 | 12 | 452 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1754 | Douglas Fir | Pseudotsuga menziesii | 7 | 4 | 50 | 20 | 1257 | 0 | Poor | Poor | Broken top. | Remove |
| 1755 | Douglas Fir | Pseudotsuga menziesii | 9 | 6 | 113 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1756 | Douglas Fir | Pseudotsuga menziesii | 11 | 10 | 314 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1757 | Douglas Fir | Pseudotsuga menziesii | 12 | 12 | 452 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1758 | Douglas Fir | Pseudotsuga menziesii | 11 | 12 | 452 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1759 | Douglas Fir | Pseudotsuga menziesii | 10 | 12 | 452 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1760 | Douglas Fir | Pseudotsuga menziesii | 14 | 12 | 452 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1761 | Douglas Fir | Pseudotsuga menziesii | 11 | 10 | 314 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1762 | Black Cottonwood | Populus trichocarpa | 10 | 10 | 314 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1762 | Black Cottonwood | Populus trichocarpa | 10 | 10 | 314 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1763 | Sweet Cherry | Prunus avium | 6 | 5 | 79 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1763 | Sweet Cherry | Prunus avium | 6 | 5 | 79 | 15 | 707 | 0 | Good | Good |  | Remove |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1764 | Douglas Fir | Pseudotsuga menziesii | 13 | 14 | 616 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1764 | Douglas Fir | Pseudotsuga menziesii | 13 | 14 | 616 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1765 | Douglas Fir | Pseudotsuga menziesii | 12 | 12 | 452 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1765 | Douglas Fir | Pseudotsuga menziesii | 12 | 12 | 452 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1766 | Douglas Fir | Pseudotsuga menziesii | 12 | 12 | 452 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1766 | Douglas Fir | Pseudotsuga menziesii | 12 | 12 | 452 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1767 | Douglas Fir | Pseudotsuga menziesii | 14 | 13 | 531 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1767 | Douglas Fir | Pseudotsuga menziesii | 14 | 13 | 531 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1768 | Pacific Madrone | Arbutus menziesii | 6 | 6 | 113 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1768 | Pacific Madrone | Arbutus menziesii | 6 | 6 | 113 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1769 | Douglas Fir | Pseudotsuga menziesii | 14 | 14 | 616 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1769 | Douglas Fir | Pseudotsuga menziesii | 14 | 14 | 616 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1770 | Douglas Fir | Pseudotsuga menziesii | 7 | 9 | 254 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1770 | Douglas Fir | Pseudotsuga menziesii | 7 | 9 | 254 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1771 | Douglas Fir | Pseudotsuga menziesii | 11 | 12 | 452 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1771 | Douglas Fir | Pseudotsuga menziesii | 11 | 12 | 452 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1772 | Douglas Fir | Pseudotsuga menziesii | 11 | 10 | 314 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1772 | Douglas Fir | Pseudotsuga menziesii | 11 | 10 | 314 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1773 | Douglas Fir | Pseudotsuga menziesii | 10 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1773 | Douglas Fir | Pseudotsuga menziesii | 10 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1774 | Douglas Fir | Pseudotsuga menziesii | 12 | 12 | 452 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1774 | Douglas Fir | Pseudotsuga menziesii | 12 | 12 | 452 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1775 | Douglas Fir | Pseudotsuga menziesii | 21 | 15 | 707 | 20 | 1257 | 0 | Poor | Poor | Dead top. | Remove |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1776 | Western Red Cedar | Thuja plicata | 22 |  | 0 |  | 0 | 0 | Dead |  | 2 stems ,12,18. | Remove |
| 1777 | Douglas Fir | Pseudotsuga menziesii | 12 |  | 0 |  | 0 | 0 | Dead |  |  | Remove |
| 1778 | Sweet Cherry | Prunus avium | 6 | 5 | 79 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1779 | Sweet Cherry | Prunus avium | 8 | 11 | 380 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1780 | Sweet Cherry | Prunus avium | 7 | 11 | 380 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1781 | Sweet Cherry | Prunus avium | 6 | 6 | 113 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1782 | Sweet Cherry | Prunus avium | 6 | 6 | 113 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1783 | Douglas Fir | Pseudotsuga menziesii | 7 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1784 | Douglas Fir | Pseudotsuga menziesii | 10 | 8 | 201 | 20 | 1257 | 0 | Poor | Poor | Thin crown | Remove |
| 1785 | Sweet Cherry | Prunus avium | 6 | 6 | 113 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1786 | Willow | Salix sp. | 6 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1787 | Douglas Fir | Pseudotsuga menziesii | 16 | 18 | 1018 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1788 | Douglas Fir | Pseudotsuga menziesii | 22 | 16 | 804 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1789 | Douglas Fir | Pseudotsuga menziesii | 22 | 13 | 531 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1790 | Sweet Cherry | Prunus avium | 6 | 5 | 79 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1791 | Douglas Fir | Pseudotsuga menziesii | 6 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1792 | Douglas Fir | Pseudotsuga menziesii | 9 | 10 | 314 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1793 | Douglas Fir | Pseudotsuga menziesii | 10 | 12 | 452 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1794 | Douglas Fir | Pseudotsuga menziesii | 12 | 14 | 616 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1795 | Douglas Fir | Pseudotsuga menziesii | 9 | 10 | 314 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1796 | Sweet Cherry | Prunus avium | 9 | 11 | 380 | 15 | 707 | 0 | Good | Good | 2 stems 6,7. | Remove |
| 1797 | Douglas Fir | Pseudotsuga menziesii | 15 | 14 | 616 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1798 | Douglas Fir | Pseudotsuga menziesii | 9 | 9 | 254 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1799 | Willow | Salix sp. | 11 | 12 | 452 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1800 | Sweet Cherry | Prunus avium | 6 | 6 | 113 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1801 | Douglas Fir | Pseudotsuga menziesii | 10 | 11 | 380 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1802 | Douglas Fir | Pseudotsuga menziesii | 6 |  | 0 |  | 0 | 0 | Dead |  |  | Remove |
| 1803 | Willow | Salix sp. | 10 | 10 | 314 | 20 | 1257 | 0 | Poor | Poor | 3 stems 6,6,6. Trunk decay | Remove |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1804 | Douglas Fir | Pseudotsuga menziesii | 8 | 7 | 154 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1805 | Douglas Fir | Pseudotsuga menziesii | 9 | 7 | 154 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1806 | Douglas Fir | Pseudotsuga menziesii | 6 | 4 | 50 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1807 | Douglas Fir | Pseudotsuga menziesii | 11 | 10 | 314 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1808 | Douglas Fir | Pseudotsuga menziesii | 11 | 11 | 380 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1809 | Douglas Fir | Pseudotsuga menziesii | 7 |  | 0 |  | 0 | 0 | Dead |  |  | Remove |
| 1810 | Douglas Fir | Pseudotsuga menziesii | 7 | 7 | 154 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1811 | Sweet Cherry | Prunus avium | 8 | 8 | 201 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1812 | Douglas Fir | Pseudotsuga menziesii | 8 |  | 0 |  | 0 | 0 | Dead |  |  | Remove |
| 1813 | Douglas Fir | Pseudotsuga menziesii | 18 | 16 | 804 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1814 | Douglas Fir | Pseudotsuga menziesii | 11 | 7 | 154 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1815 | Douglas Fir | Pseudotsuga menziesii | 15 | 8 | 201 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1816 | Douglas Fir | Pseudotsuga menziesii | 21 | 13 | 531 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1817 | Sweet Cherry | Prunus avium | 7 | 14 | 616 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1818 | Douglas Fir | Pseudotsuga menziesii | 14 | 15 | 707 | 20 | 1257 | 0 | Good | Good | 2 stems 10,10. | Remove |
| 1819 | Douglas Fir | Pseudotsuga menziesii | 21 | 10 | 314 | 20 | 1257 | 0 | Good | Good | stems 7,20. | Remove |
| 1820 | Douglas Fir | Pseudotsuga menziesii | 21 | 14 | 616 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1821 | Douglas Fir | Pseudotsuga menziesii | 7 | 3 | 28 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1822 | Willow | Salix sp. | 11 | 6 | 113 | 20 | 1257 | 0 | Poor | Poor | Trunk decay. stems 6,7. | Remove |
| 1823 | Douglas Fir | Pseudotsuga menziesii | 24 | 18 | 1018 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1824 | Pacific Madrone | Arbutus menziesii | 8 | 11 | 380 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1825 | Douglas Fir | Pseudotsuga menziesii | 17 | 14 | 616 | 20 | 1257 | 0 | Poor | Poor | Thin crown. | Remove |
| 1826 | Douglas Fir | Pseudotsuga menziesii | 15 | 12 | 452 | 20 | 1257 | 0 | Poor | Poor | Thin crown. | Remove |
| 1827 | Bigleaf Maple | Acer macrophyllum | 7 | 8 | 201 | 25 | 1963 | 0 | Good | Good |  | Remove |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1828 | Douglas Fir | Pseudotsuga menziesii | 19 | 20 | 1257 | 20 | 1257 | 0 | Poor | Poor | Thin crown. | Remove |
| 1829 | Douglas Fir | Pseudotsuga menziesii | 17 | 16 | 804 | 20 | 1257 | 0 | Poor | Poor | Broken top. | Remove |
| 1830 | Oregon White Oak | Quercus garrayana | 11 | 6 | 113 | 25 | 1963 | 0 | Good | Good | 2 stems 6,7. | Remove |
| 1831 | Pacific Madrone | Arbutus menziesii | 11 | 14 | 616 | 15 | 707 | 0 | Good | Good | 2 stems 7,9. | Remove |
| 1832 | Oregon White Oak | Quercus garrayana | 18 | 23 | 1662 | 25 | 1963 | 0 | Good | Good |  | Remove |
| 1833 | Pacific Madrone | Arbutus menziesii | 8 | 10 | 314 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1834 | Pacific Madrone | Arbutus menziesii | 7 | 8 | 201 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1835 | Pacific Madrone | Arbutus menziesii | 7 | 8 | 201 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1836 | Pacific Madrone | Arbutus menziesii | 6 | 8 | 201 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1837 | Pacific Madrone | Arbutus menziesii | 16 | 18 | 1018 | 15 | 707 | 0 | Poor | Poor | Trunk cavity. | Remove |
| 1838 | Pacific Madrone | Arbutus menziesii | 13 | 15 | 707 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1839 | Pacific Madrone | Arbutus menziesii | 6 | 8 | 201 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1840 | Pacific Madrone | Arbutus menziesii | 6 | 8 | 201 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1841 | Oregon White Oak | Quercus garrayana | 6 | 5 | 79 | 25 | 1963 | 0 | Good | Good |  | Remove |
| 1842 | Oregon White Oak | Quercus garrayana | 11 | 10 | 314 | 25 | 1963 | 0 | Good | Good | 2 stems 7,8. | Remove |
| 1843 | Oregon White Oak | Quercus garrayana | 21 | 23 | 1662 | 25 | 1963 | 0 | Good | Good | 2 stems 15,15. | Remove |
| 1844 | Pacific Madrone | Arbutus menziesii | 11 | 13 | 531 | 15 | 707 | 0 | Good | Good | 2 stems 7,9. | Remove |
| 1845 | Oregon White Oak | Quercus garrayana | 14 | 12 | 452 | 25 | 1963 | 0 | Good | Good | 5 stems 6,6,6,7,7. | Remove |
| 1846 | Oregon White Oak | Quercus garrayana | 8 | 9 | 254 | 25 | 1963 | 0 | Good | Good | 2 stems 6,6. | Remove |
| 1847 | Oregon White Oak | Quercus garrayana | 20 | 14 | 616 | 25 | 1963 | 0 | Good | Good | 5 stems 6,7,9,10,11. | Remove |
| 1848 | Pacific Madrone | Arbutus menziesii | 13 | 10 | 314 | 15 | 707 | 0 | Poor | Poor | Partial uproot | Remove |
| 1849 | Oregon White Oak | Quercus garrayana | 10 | 13 | 531 | 25 | 1963 | 0 | Good | Good |  | Remove |
| 1850 | Sweet Cherry | Prunus avium | 7 | 12 | 452 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1851 | Willow | Salix sp. | 6 | 9 | 254 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1852 | Willow | Salix sp. | 11 | 16 | 804 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1853 | Douglas Fir | Pseudotsuga menziesii | 14 | 13 | 531 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1854 | Oregon White Oak | Quercus garrayana | 10 | 12 | 452 | 25 | 1963 | 0 | Good | Good |  | Remove |
| 1855 | Douglas Fir | Pseudotsuga menziesii | 19 | 10 | 314 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1856 | Douglas Fir | Pseudotsuga menziesii | 30 | 16 | 804 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1857 | Douglas Fir | Pseudotsuga menziesii | 21 |  | 0 | 20 | 1257 | 0 | Dead |  |  | Remove |
| 1858 | Douglas Fir | Pseudotsuga menziesii | 21 |  | 0 | 20 | 1257 | 0 | Dead |  |  | Remove |
| 1859 | Oregon White Oak | Quercus garrayana | 10 | 12 | 452 | 25 | 1963 | 0 | Good | Good | 2 stems 7,7. | Remove |
| 1860 | Oregon White Oak | Quercus garrayana | 8 | 7 | 154 | 25 | 1963 | 0 | Good | Good | 2 stems 5,6. | Remove |
| 1861 | Pacific Madrone | Arbutus menziesii | 8 | 9 | 254 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1862 | Pacific Madrone | Arbutus menziesii | 12 | 8 | 201 | 15 | 707 | 0 | Poor | Poor | Broken top | Remove |
| 1863 | Pacific Madrone | Arbutus menziesii | 10 | 8 | 201 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1864 | Pacific Madrone | Arbutus menziesii | 15 | 13 | 531 | 15 | 707 | 0 | Poor | Poor | Crown die back. | Remove |

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arboricultural Consultants
Attachment 2

| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1865 | Pacific Madrone | Arbutus menziesii | 13 |  | 0 | 15 | 707 | 0 | Dead |  |  | Remove |
| 1866 | Oregon White Oak | Quercus garrayana | 9 | 10 | 314 | 25 | 1963 | 0 | Good | Good |  | Remove |
| 1867 | Oregon White Oak | Quercus garrayana |  | 17 | 908 | 25 | 1963 | 0 | Good | Good | 3 stems 7,10,14. | Remove |
| 1868 | Oregon White Oak | Quercus garrayana | 17 | 11 | 380 | 25 | 1963 | 0 | Good | Good |  | Remove |
| 1869 | Douglas Fir | Pseudotsuga menziesii | 13 |  | 0 | 20 | 1257 | 0 | Dead |  |  | Remove |
| 1870 | Oregon White Oak | Quercus garrayana | 13 | 15 | 707 | 25 | 1963 | 0 | Poor | Poor | Thin crown. | Remove |
| 1871 | Douglas Fir | Pseudotsuga menziesii | 23 |  | 0 | 20 | 1257 | 0 | Dead |  |  | Remove |
| 1872 | Pacific Madrone | Arbutus menziesii | 21 | 6 | 113 | 15 | 707 | 0 | Poor | Poor | 4 stems 7,10,12,12. die back. | Remove |
| 1873 | Pacific Madrone | Arbutus menziesii |  |  | 0 | 15 | 707 | 0 | Dead |  |  | Remove |
| 1874 | Oregon White Oak | Quercus garrayana | 14 | 14 | 616 | 25 | 1963 | 0 | Good | Good |  | Remove |
| 1875 | Pacific Madrone | Arbutus menziesii | 6 | 5 | 79 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1876 | Pacific Madrone | Arbutus menziesii | 11 |  | 0 | 15 | 707 | 0 | Dead |  |  | Remove |
| 1877 | Oregon White Oak | Quercus garrayana | 23 | 24 | 1810 | 25 | 1963 | 0 | Good | Good | 3 stems 12,14,14. | Remove |
| 1878 | Pacific Madrone | Arbutus menziesii | 6 | 3 | 28 | 15 | 707 | 0 | Poor | Poor | Crown die back. | Remove |
| 1879 | Pacific Madrone | Arbutus menziesii | 7 | 8 | 201 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1880 | Pacific Madrone | Arbutus menziesii | 11 | 13 | 531 | 15 | 707 | 0 | Poor | Poor | Crown die back. | Remove |
| 1881 | Pacific Madrone | Arbutus menziesii | 8 | 6 | 113 | 15 | 707 | 0 | Poor | Poor | Crown die back. | Remove |
| 1882 | Pacific Madrone | Arbutus menziesii | 8 |  | 0 | 15 | 707 | 0 | Dead |  |  | Remove |
| 1883 | Pacific Madrone | Arbutus menziesii | 8 | 7 | 154 | 15 | 707 | 0 | Poor | Poor | Crown die back. | Remove |
| 1884 | Pacific Madrone | Arbutus menziesii | 9 | 12 | 452 | 15 | 707 | 0 | Poor | Poor | Crown die back. | Remove |
| 1885 | Oregon White Oak | Quercus garrayana | 13 | 18 | 1018 | 25 | 1963 | 0 | Good | Good | 3 stems 7,8,8. | Remove |
| 1886 | Pacific Madrone | Arbutus menziesii | 8 |  | 0 | 15 | 707 | 0 | Dead |  |  | Remove |
| 1887 | Pacific Madrone | Arbutus menziesii | 9 | 7 | 154 | 15 | 707 | 0 | Poor | Poor | Crown die back. | Remove |
| 1888 | Pacific Madrone | Arbutus menziesii | 7 | 3 | 28 | 15 | 707 | 0 | Poor | Poor | Dead top. | Remove |
| 1889 | Pacific Madrone | Arbutus menziesii | 7 | 9 | 254 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1890 | Pacific Madrone | Arbutus menziesii | 7 | 7 | 154 | 15 | 707 | 0 | Poor | Poor | Crown die back. | Remove |
| 1891 | Pacific Madrone | Arbutus menziesii | 17 |  | 0 | 15 | 707 | 0 | Dead |  | 4 stems 8,8,9,9. | Remove |
| 1892 | Douglas Fir | Pseudotsuga menziesii | 13 |  | 0 | 20 | 1257 | 0 | Dead |  |  | Remove |
| 1893 | Oregon White Oak | Quercus garrayana | 14 | 26 | 2124 | 25 | 1963 | 0 | Good | Good | 2 stems 10,10. | Remove |
| 1894 | Pacific Madrone | Arbutus menziesii | 7 | 15 | 707 | 15 | 707 | 0 | Poor | Poor | Thin crown. | Remove |
| 1895 | Pacific Madrone | Arbutus menziesii | 8 | 10 | 314 | 15 | 707 | 0 | Poor | Poor | Crown die back. | Remove |
| 1896 | Douglas Fir | Pseudotsuga menziesii | 16 |  | 0 | 20 | 1257 | 0 | Dead |  |  | Remove |
| 1897 | Pacific Madrone | Arbutus menziesii | 6 |  | 0 | 15 | 707 | 0 | Dead |  |  | Remove |
| 1898 | Pacific Madrone | Arbutus menziesii | 6 |  | 0 | 15 | 707 | 0 | Dead |  |  | Remove |
| 1899 | Douglas Fir | Pseudotsuga menziesii | 11 |  | 0 | 20 | 1257 | 0 | Dead |  |  | Remove |
| 1900 | Douglas Fir | Pseudotsuga menziesii | 17 |  | 0 | 20 | 1257 | 0 | Dead |  |  | Remove |
| 1901 | Pacific Madrone | Arbutus menziesii | 8 | 12 | 452 | 15 | 707 | 0 | Poor | Poor | Crown die back. | Remove |
| 1902 | Pacific Madrone | Arbutus menziesii | 6 | 11 | 380 | 15 | 707 | 0 | Poor | Poor | Crown die back. | Remove |

[^35]| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1903 | Pacific Madrone | Arbutus menziesii | 12 | 18 | 1018 | 15 | 707 | 0 | Poor | Poor | Thin crown | Remove |
| 1904 | Pacific Madrone | Arbutus menziesii | 9 | 16 | 804 | 15 | 707 | 0 | Poor | Poor | Crown die back. | Remove |
| 1905 | Pacific Madrone | Arbutus menziesii | 10 |  | 0 | 15 | 707 | 0 | Dead |  |  | Remove |
| 1906 | Douglas Fir | Pseudotsuga menziesii | 18 | 14 | 616 | 20 | 1257 | 0 | Fair | Fair | Thin crown. | Remove |
| 1907 | Pacific Madrone | Arbutus menziesii | 7 |  | 0 | 15 | 707 | 0 | Dead |  |  | Remove |
| 1908 | Douglas Fir | Pseudotsuga menziesii | 19 | 14 | 616 | 20 | 1257 | 0 | Poor | Poor | Thin crown. | Remove |
| 1909 | Pacific Madrone | Arbutus menziesii | 8 |  | 0 | 15 | 707 | 0 | Dead |  |  | Remove |
| 1910 | Pacific Madrone | Arbutus menziesii | 7 | 11 | 380 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1911 | Pacific Madrone | Arbutus menziesii | 8 | 6 | 113 | 15 | 707 | 0 | Poor | Poor | Dead top. | Remove |
| 1912 | Douglas Fir | Pseudotsuga menziesii | 21 |  | 0 | 20 | 1257 | 0 | Dead |  |  | Remove |
| 1913 | Douglas Fir | Pseudotsuga menziesii | 12 | 16 | 804 | 20 | 1257 | 0 | Poor | Poor | Thin crown. | Remove |
| 1914 | Pacific Madrone | Arbutus menziesii | 9 | 14 | 616 | 15 | 707 | 0 | Poor | Poor | Thin crown. | Remove |
| 1915 | Pacific Madrone | Arbutus menziesii | 6 | 9 | 254 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1916 | Oregon White Oak | Quercus garrayana | 9 | 10 | 314 | 25 | 1963 | 0 | Good | Good |  | Remove |
| 1917 | Pacific Madrone | Arbutus menziesii | 6 | 8 | 201 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1918 | Pacific Madrone | Arbutus menziesii | 10 | 12 | 452 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1919 | Pacific Madrone | Arbutus menziesii | 9 | 9 | 254 | 15 | 707 | 0 | Poor | Poor | Dead top. | Remove |
| 1920 | Oregon White Oak | Quercus garrayana | 15 | 15 | 707 | 25 | 1963 | 0 | Good | Good | 3 stems 6,9,10. | Remove |
| 1921 | Pacific Madrone | Arbutus menziesii | 8 | 7 | 154 | 15 | 707 | 0 | Poor | Poor | Crown die back. | Remove |
| 1922 | Pacific Madrone | Arbutus menziesii | 8 | 11 | 380 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1923 | Pacific Madrone | Arbutus menziesii | 8 | 8 | 201 | 15 | 707 | 0 | Poor | Poor | Crown die back. | Remove |
| 1924 | Pacific Madrone | Arbutus menziesii | 10 | 8 | 201 | 15 | 707 | 0 | Poor | Poor | Crown die back. | Remove |
| 1925 | Pacific Madrone | Arbutus menziesii | 10 | 10 | 314 | 15 | 707 | 0 | Poor | Poor | Thin crown. | Remove |
| 1926 | Douglas Fir | Pseudotsuga menziesii | 14 |  | 0 | 20 | 1257 | 0 | Dead |  |  | Remove |
| 1927 | Pacific Madrone | Arbutus menziesii | 7 | 8 | 201 | 15 | 707 | 0 | Poor | Poor | Thin crown. | Remove |
| 1928 | Oregon White Oak | Quercus garrayana | 9 | 12 | 452 | 25 | 1963 | 0 | Good | Good |  | Remove |
| 1929 | Pacific Madrone | Arbutus menziesii | 6 | 8 | 201 | 15 | 707 | 0 | Poor | Poor | Crown die back. | Remove |
| 1930 | Douglas Fir | Pseudotsuga menziesii | 13 | 7 | 154 | 20 | 1257 | 0 | Poor | Poor | 2 stem 7,11. Dead top. | Remove |
| 1931 | Douglas Fir | Pseudotsuga menziesii | 23 | 13 | 531 | 20 | 1257 | 0 | Poor | Poor | Dead top. | Remove |
| 1932 | Douglas Fir | Pseudotsuga menziesii | 7 | 7 | 154 | 20 | 1257 | 0 | Poor | Poor | Dead top. | Remove |
| 1933 | Pacific Madrone | Arbutus menziesii | 6 | 8 | 201 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1934 | Pacific Madrone | Arbutus menziesii | 6 | 10 | 314 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1935 | Douglas Fir | Pseudotsuga menziesii | 17 | 13 | 531 | 20 | 1257 | 0 | Poor | Poor | Thin crown. | Remove |
| 1936 | Douglas Fir | Pseudotsuga menziesii | 18 | 17 | 908 | 20 | 1257 | 0 | Poor | Poor | Thin crown. | Remove |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1937 | Pacific Madrone | Arbutus menziesii | 11 | 16 | 804 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1938 | Douglas Fir | Pseudotsuga menziesii | 20 |  | 0 | 20 | 1257 | 0 | Dead |  |  | Remove |
| 1939 | Douglas Fir | Pseudotsuga menziesii | 15 |  | 0 | 20 | 1257 | 0 | Dead |  |  | Remove |
| 1940 | Douglas Fir | Pseudotsuga menziesii | 20 |  | 0 | 20 | 1257 | 0 | Dead |  |  | Remove |
| 1941 | Oregon White Oak | Quercus garrayana | 19 | 19 | 1134 | 25 | 1963 | 0 | Good | Good |  | Remove |
| 1942 | Oregon White Oak | Quercus garrayana | 23 | 25 | 1963 | 25 | 1963 | 0 | Good | Good | 2 stems 12,20. | Remove |
| 1943 | Pacific Madrone | Arbutus menziesii | 13 | 17 | 908 | 15 | 707 | 0 | Good | Good | 2 stems 6,12. | Remove |
| 1944 | Douglas Fir | Pseudotsuga menziesii | 13 |  | 0 | 20 | 1257 | 0 | Dead |  |  | Remove |
| 1945 | Douglas Fir | Pseudotsuga menziesii | 22 | 16 | 804 | 20 | 1257 | 0 | Poor | Poor | Thin crown. | Remove |
| 1946 | Pacific Madrone | Arbutus menziesii | 10 | 12 | 452 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1947 | Douglas Fir | Pseudotsuga menziesii | 24 | 18 | 1018 | 20 | 1257 | 0 | Poor | Poor | Thin crown. | Remove |
| 1948 | Pacific Madrone | Arbutus menziesii | 10 | 12 | 452 | 15 | 707 | 0 | Good | Good |  | Remove |
| 1949 | Douglas Fir | Pseudotsuga menziesii | 13 | 14 | 616 | 20 | 1257 | 0 | Poor | Poor | Thin crown. | Remove |
| 1950 | Douglas Fir | Pseudotsuga menziesii | 14 | 10 | 314 | 20 | 1257 | 0 | Poor | Poor | Suppressed. | Remove |
| 1951 | Douglas Fir | Pseudotsuga menziesii | 24 | 12 | 452 | 20 | 1257 | 0 | Good | Good | 2 stems 10,22. | Remove |
| 1952 | Douglas Fir | Pseudotsuga menziesii | 20 | 18 | 1018 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 1953 | Douglas Fir | Pseudotsuga menziesii | 16 | 12 | 452 | 20 | 1257 | 0 | Poor | Poor | Broken top. | Remove |
| 1954 | Pacific Madrone | Arbutus menziesii | 8 | 8 | 201 | 15 | 707 | 0 | Poor | Poor | Thin crown. | Remove |
| 1955 | Douglas Fir | Pseudotsuga menziesii | 10 | 10 | 314 | 20 | 1257 | 0 | Poor | Poor | Dead top. | Remove |
| 1956 | Pacific Madrone | Arbutus menziesii | 6 |  | 0 | 15 | 707 | 0 | Dead |  |  | Remove |
| 1957 | Pacific Madrone | Arbutus menziesii | 13 |  | 0 | 15 | 707 | 0 | Dead |  |  | Remove |
| 11003 |  |  |  |  | 0 |  | 0 | 0 |  |  | Not found | Remove |
| 11004 |  |  |  |  | 0 |  | 0 | 0 |  |  | Not found | Remove |
| 11011 |  |  |  |  | 0 |  | 0 | 0 |  |  | Not found | Remove |
| 11016 |  |  |  |  | 0 |  | 0 | 0 |  |  | Not found | Remove |
| 11030 |  |  |  |  | 0 |  | 0 | 0 |  |  | Not found | Remove |
| 11519 |  |  |  |  | 0 |  | 0 | 0 |  |  | Not found | Remove |
| 11529 |  |  |  |  | 0 |  | 0 | 0 |  |  | Not found | Remove |
| 15032 | Pacific Madrone | Arbutus menziesii | 9 | 10 | 314 | 15 | 707 | 0 | Good | Good |  | Remove |
| 15033 | Oregon White Oak | Quercus garrayana | 16 | 15 | 707 | 25 | 1963 | 0 | Good | Good | 2 stems 10,13. | Remove |
| 15035 | Pacific Madrone | Arbutus menziesii | 7 | 8 | 201 | 15 | 707 | 0 | Good | Good | 2 stems 5,5. | Remove |
| 15036 | Pacific Madrone | Arbutus menziesii | 13 | 13 | 531 | 15 | 707 | 0 | Good | Good |  | Remove |


| Tree No. | Common Name | Scientific Name | DBH ${ }^{1}$ | C-Rad ${ }^{2}$ | Canopy Area (sq. ft.) | Mature <br> C-Rad ${ }^{3}$ | Mature Canopy Area (sq. ft.) | Canopy Credit ${ }^{4}$ | Condition ${ }^{5}$ | Structure ${ }^{5}$ | Comments | Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 15039 | Pacific Madrone | Arbutus menziesii | 8 | 17 | 908 | 17 | 908 | 0 | Good | Good |  | Remove |
| 15043 | Pacific Madrone | Arbutus menziesii | 10 | 15 | 707 | 15 | 707 | 0 | Good | Good | 2 stems 7,7. | Remove |
| 15135 | Douglas Fir | Pseudotsuga menziesii | 25 | 18 | 1018 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 15135.1 | Douglas Fir | Pseudotsuga menziesii | 23 | 16 | 804 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 15135.2 | Douglas Fir | Pseudotsuga menziesii | 19 | 16 | 804 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 15135.3 | Pacific Madrone | Arbutus menziesii | 9 | 5 | 79 | 15 | 707 | 0 | Good | Good |  | Remove |
| 15136 | Pacific Madrone | Arbutus menziesii | 10 | 15 | 707 | 15 | 707 | 0 | Good | Good |  | Remove |
| 15137 | Douglas Fir | Pseudotsuga menziesii | 6 | 4 | 50 | 20 | 1257 | 0 | Fair | Fair | Suppressed | Remove |
| 15138 | Pacific Madrone | Arbutus menziesii | 5 | 7 | 154 | 15 | 707 | 0 | Good | Good |  | Remove |
| 15139 | Sweet Cherry | Prunus avium | 5 | 4 | 50 | 15 | 707 | 0 | Poor | Poor | Suppressed | Remove |
| 15140 | Douglas Fir | Pseudotsuga menziesii | 17 | 15 | 707 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 15202 | Pacific Madrone | Arbutus menziesii | 6 | 4 | 50 | 15 | 707 | 0 | Good | Good |  | Remove |
| 15205 | Pacific Madrone | Arbutus menziesii | 9 | 12 | 452 | 15 | 707 | 0 | Good | Good |  | Remove |
| 15206 | Pacific Madrone | Arbutus menziesii | 9 | 13 | 531 | 15 | 707 | 0 | Good | Good |  | Remove |
| 15207 | Pacific Madrone | Arbutus menziesii | 13 | 12 | 452 | 15 | 707 | 0 | Good | Good |  | Remove |
| 15210 | Oregon White Oak | Quercus garrayana | 9 | 13 | 531 | 25 | 1963 | 0 | Good | Good |  | Remove |
| 15288 | Pacific Madrone | Arbutus menziesii | 8 | 12 | 452 | 15 | 707 | 0 | Good | Good |  | Remove |
| 15289 | Oregon White Oak | Quercus garrayana | 13 | 10 | 314 | 25 | 1963 | 0 | Good | Good | 2 stems 7,8. | Remove |
| 15718 | Oregon White Oak | Quercus garrayana | 18 | 18 | 1018 | 25 | 1963 | 0 | Good | Good |  | Remove |
| 15719 | Douglas Fir | Pseudotsuga menziesii | 27 | 17 | 908 | 20 | 1257 | 0 | Poor | Poor | Thin crown. | Remove |
| 15723 | Douglas Fir | Pseudotsuga menziesii | 16 | 11 | 380 | 20 | 1257 | 0 | Good | Good |  | Remove |
| 15724 | Douglas Fir | Pseudotsuga menziesii | 18 | 12 | 452 | 20 | 1257 | 0 | Fair | Fair | Thin crown. | Remove |
| 15728 | Douglas Fir | Pseudotsuga menziesii | 21 |  | 0 | 20 | 1257 | 0 | Dead |  |  | Remove |
| 15735 | Pacific Madrone | Arbutus menziesii | 18 | 9 | 254 | 15 | 707 | 0 | Poor | Poor | 2 stems 12,14. crown die back. | Remove |
|  |  |  |  |  |  |  |  |  |  |  |  |  |

${ }^{1}$ DBH is the trunk diameter measured according to the International Society of Arboriculture standards in inches. In cases where the tree splits into multiple trunks at ground level, DBH is the square root of the sum of the squared DBH of each stem.
${ }^{2} \mathrm{C}$-rad is the approximate crown radius in feet.
${ }^{3}$ Mature $\mathbf{C}$-rad is the mature crown radius in feet based on scientific literature. If the existing crown is larger than the mature crown listed in the scientific literature, the existing crown size is used.
${ }^{4}$ Canopy Credit is $2 x$ the mature canopy for onsite trees that are preserved.
${ }^{5}$ Condition and Structure ratings range from very poor, poor, fair, to good.

## Attachment 3

## Additional Tree Protection Recommendations

The following recommendations meet City of Sherwood Code requirements:

## Before Construction Begins

1. Notify all contractors of tree protection procedures. For successful tree protection on a construction site, all contractors must know and understand the goals of tree protection.
a. Hold a tree protection meeting with all contractors to explain the goals of tree protection.
c. Have all contractors sign memoranda of understanding regarding the goals of tree protection. The memoranda should include a penalty for violating the tree protection plan. The penalty should equal the resulting fines issued by the local jurisdiction plus the appraised value of the tree(s) within the violated tree protection zone per the current Trunk Formula Method as outline in the current edition of the Guide for Plant Appraisal by the Council of Tree \& Landscape Appraisers. The penalty should be paid to the owner of the property.
2. Fencing
a. Trees to remain on site should be protected by installation of tree protection fencing at the dripline. Alternatively, tree protection fencing may be set as shown in Attachment 1.
b. The fencing should be put in place before the ground is cleared in order to protect the trees and the soil around the trees from disturbances.
c. Fencing should be established by the project arborist based on the needs of the trees to be protected and to facilitate construction.
d. Fencing should consist of 6 -foot high steel fencing on concrete blocks or 6foot metal fencing secured to the ground with 8 -foot metal posts to prevent it from being moved by contractors, sagging, or falling down. Trees within the north wetland may be protected with orange work limit fencing.
e. Fencing should remain in the position that is established by the project arborist and not be moved without approval from the project arborist until final project approval.
3. Signage
a. All tree protection fencing should have signage as follows so that all contractors understand the purpose of the fencing:

## TREE PROTECTION ZONE

## DO NOT REMOVE OR ADJUST THE APPROVED LOCATION OF THIS TREE PROTECTION FENCING.

Please contact the project arborist if alterations to the approved location of the tree protection fencing are necessary.

Todd Prager, Project Arborist - 971-295-4835
b. Signage should be placed every 75 -feet or less.

[^36]
## During Construction

1. Protection Guidelines Within the Tree Protection Zones:
a. No new buildings; grade change or cut and fill, during or after construction; new impervious surfaces; or utility or drainage field placement should be allowed within the tree protection zones.
b. No traffic should be allowed within the tree protection zones. This includes but is not limited to vehicle, heavy equipment, or even repeated foot traffic.
c. No storage of materials including but not limiting to soil, construction material, or waste from the site should be permitted within the tree protection zones. Waste includes but is not limited to concrete wash out, gasoline, diesel, paint, cleaner, thinners, etc.
d. Construction trailers should not to be parked/placed within the tree protection zones.
e. No vehicles should be allowed to park within the tree protection zones.
f. No other activities should be allowed that will cause soil compaction within the tree protection zones.
2. The trees should be protected from any cutting, skinning or breaking of branches, trunks or woody roots.
3. The project arborist should be notified prior to the cutting of woody roots from trees that are to be retained to evaluate and oversee the proper cutting of roots with sharp cutting tools. Cut roots should be immediately covered with soil or mulch to prevent them from drying out.
4. Trees that have roots cut should be provided supplemental water during the summer months.
5. Any necessary passage of utilities through the tree protection zones should be by means of tunneling under woody roots by hand digging or boring with oversight by the project arborist.
6. Any deviation from the recommendations in this section should receive prior approval from the project arborist.

## After Construction

1. Carefully landscape the areas within the tree protection zones. Do not allow trenching for irrigation or other utilities within the tree protection zones.
2. Carefully plant new plants within the tree protection zones. Avoid cutting the woody roots of trees that are retained.
3. Do not install permanent irrigation within the tree protection zones unless it is drip irrigation to support a specific planting or the irrigation is approved by the project arborist.
4. Provide adequate drainage within the tree protection zones and do not alter soil hydrology significantly from existing conditions for the trees to be retained.
5. Provide for the ongoing inspection and treatment of insect and disease populations that are capable of damaging the retained trees and plants.
6. The retained trees may need to be fertilized if recommended by the project arborist.
7. Any deviation from the recommendations in this section should receive prior approval from the project arborist.
[^37]
## Attachment 4 <br> Assumptions and Limiting Conditions

1. Any legal description provided to the consultant is assumed to be correct. The site plans and other information provided by Trammell Crow Company and their consultants was the basis of the information provided in this report.
2. It is assumed that this property is not in violation of any codes, statutes, ordinances, or other governmental regulations.
3. The consultant is not responsible for information gathered from others involved in various activities pertaining to this project. Care has been taken to obtain information from reliable sources.
4. Loss or alteration of any part of this delivered report invalidates the entire report.
5. Drawings and information contained in this report may not be to scale and are intended to be used as display points of reference only.
6. The consultant's role is only to make recommendations. Inaction on the part of those receiving the report is not the responsibility of the consultant.
7. The purpose of this report is to:

- Assess the existing trees at the project site;
- Identify the trees to be removed and retained based on construction impacts;
- Provide tree protection recommendations for the trees to be retained; and
- Provide recommendations for meeting the tree canopy requirements in section 16.142.070 of the City of Sherwood Code.

[^38]TO: Bob Galati, P.E. - City Engineer<br>City of Sherwood<br>FROM: Ryan Halvorson, P.E. - Design Engineer<br>DATE: $\quad$ March $4^{\text {th }}, 2020$<br>SUBJECT: Design Modification Request for Cul-de-sac 200 Foot Maximum Length

## Location of Requested Design Modification

The new SW Cipole Place street between Tualatin-Sherwood Rd and future Blake Rd.

## Current Standards

Section 210.7.C of the City of Sherwood Engineering Manual states "Cul-de-sacs shall not be more than 200 feet in length, except for the modified infill design cul-de-sac, which shall not be more than 150 feet in length. The length of a cul-de-sac shall be measured along the center line of the cul-de-sac from the near side right-of-way of the nearest through traffic intersecting street to the farthest point of the cul-de-sac right-of-way. See standard details for cul-de-sac right-of-way and pavement requirements."

## Design Modification Being Requested

We are requesting approval for a 619-foot long cul-de-sac as measured from the southern right-of-way line of Tualatin-Sherwood Road to the most southernly point the cul-de-sac right-of-way. This 619-foot length would exceed the City standard 200 -foot maximum length.

## Existing Conditions

The existing site is an undeveloped site, consisting of forested sections and grass land sloping from the southwest corner to the north east corner. In addition to the site being undeveloped, there are existing sensitive area consisting of wetlands and vegetated corridor buffers are located on the site limiting the development of the site.

Where Cipole Place cul-de-sac is proposed, the existing topography of the site is approximately 3 percent. As a result, the proposed Cipole Place is proposed at 3 percent grade and has minimal impact to sensitive areas, only impacting a small section of vegetated corridor.

## Result of Meeting Standards

To meet the City's standard cul-de-sac length, SW Cipole Place would be limited to 200 feet from the southern right-of-way line of Tualatin Sherwood Road. At 200 feet long, the cul-de-sac central access to the subdivision lots would be located significantly further north of the proposed location, limiting the access to the lots and limiting the overall development of the site. As a result, the cul-de-sac full access would be in the middle of one of the existing wetlands and require additional impact to the
wetlands for development. To avoid impacting the wetlands would require nearly eliminating access to subdivision lots, making the site undevelopable.

In addition, the Traffic Impact Analysis completed by Kittelson \& Associates, dated January 15, 2020, found the development needs to have a minimum queue length of 150 feet. This required queue length and the minimum radius for the WB-67 truck movements (approximately 130 feet), the total length of the cul-de-sac needs to be a minimum of 300 feet to serve the development.

## Proposed Design Modification

The proposed cul-de-sac street length of 619 feet is to provide access to the subdivision layout and to accommodate the required traffic queue length for the industrial development.

## Reason Why Design Request Should be Approved

The design exception meets the criteria of Section 145.1.5.A. 2 because the existing vegetated corridor and wetland create a hardship on the site to meet the City standard. To avoid impacting the existing wetland, and minimize the impact of the vegetated corridor, the cul-de-sac needs to be extended beyond the City 200 -foot standard to provide access to the subdivision lots. The proposed 619 -foot cul-de-sac will provide full access to the subdivision lots and avoid impacting the existing sensitive areas, while minimizing the length of the cul-de-sac to greatest extent possible.

Additionally, the 619-foot cul-de-sac will provide the required queue storage for the development traffic per the Kittelson \& Associates TIA and allow for proper truck movements with the actual cul-de-sac outside the queue storage.

The proposed design will meet the City cul-des-sac design intent and improve safety for the public beyond the City standard cul-de-sac.


TO: Bob Galati, P.E. - City Engineer<br>City of Sherwood<br>FROM: Ryan Halvorson, P.E. - Design Engineer<br>DATE: $\quad$ March $4^{\text {th }}, 2020$<br>SUBJECT: Design Modification Request for Cul-de-sac Geometry

## Location of Requested Design Modification

The new SW Cipole Place street between Tualatin-Sherwood Rd and future Blake Rd.

## Current Standards

Section 210.7.A of the City of Sherwood Engineering Manual states "The following specifies the minimum requirements for cul-de-sac, eyebrows, and turnaround areas. Other turnaround geometrics may be used when conditions warrant and the City Engineer and TVF\&R Fire Marshal approve the design and application of its use."

## Design Modification Being Requested

We are requesting a revision to the geometry of City's standard cul-de-sac design because SW Cipole Place is industrial street and the City's standard is for a residential local road.

## Existing Conditions

The existing site is an undeveloped site, consisting of forested sections and grass land sloping from the southwest corner to the north east corner. In addition to the site being undeveloped, there are existing sensitive area consisting of wetlands and vegetated corridor buffers are located on the site limiting the development of the site.

Where Cipole Place cul-de-sac is proposed, the existing topography of the site is approximately 3 percent. As a result, the proposed Cipole Place is proposed at 3 percent grade and has minimal impact to sensitive areas, only impacting a small section of vegetated corridor.

## Result of Meeting Standards

The City's standards cul-de-sac dimensions are designed for a residential street and a fire truck. The site is zoned industrial, and SW Cipole Place is designated as a commercial/industrial street. The City's standard cul-de-sac is adequate for passenger cars and fire truck movements but is too small for WB-67 tuck to turn around.

As a result, a WB-67 would be required to make multiple truck movements, including reversing, to complete a full turn around movement with the City's standard cul-de-sac.

## Proposed Design Modification

The proposed cul-de-sac radius will be modified from 48 -foot radius to the face of curb City standard to 54 -foot radius to the face of curb to accommodate WB-67 truck movements. Attached are Autoturn truck turning movements showing a fire truck and a WB-67 truck can maneuver the enlarged 54-foot radius without having to make multiple movements.

The proposed landscape strip, sidewalk, and monument strip width are proposed at the City standards. As a result, the proposed 54 -foot radius will result in 66 -foot radius right-of-way.

## Reason Why Design Request Should be Approved

The proposed industrial park will see vehicular traffic, including WB-67 traffic. The design exception meets the criteria of Section 145.1.5.A. 1 because the City standard cul-de-sac of a 48 -foot radius is too small to accommodate truck maneuvering and the standard is not applicable in this situation. A WB-67 truck movement will require multiple movements, including reversing, which is a safety concern to the general public. The City standard is inadequate for this project and will cause potential harm to the public.

By increasing the cul-de-sac radius from 48 -foot City standard to 54 -foot radius will allow for WB-67 trucks to fully maneuver within the cul-de-sac and reduce the safety risk of associated with extra truck maneuvers. The proposed cul-de-sac will meet all other City and TVF\&R dimensions.




TO: Bob Galati, P.E. - City Engineer
City of Sherwood
FROM: Ryan Halvorson, P.E. - Design Engineer
DATE: $\quad$ March $4^{\text {th }}, 2020$
SUBJECT: Design Modification Request for SW Cipole Block Length on Blake Road

## Location of Requested Design Modification

The new SW Cipole Place street connecting to future Blake Road.

## Current Standards

Section 210.6.E of the City of Sherwood Engineering Manual states the minimum and maximum distance between streets.

| Street <br> Classification |  <br> Driveways <br> Spacing (max) | Full Access <br> Intersections <br> Spacing (min) | Limited Access* <br> Intersections <br> Spacing (min) | Driveway <br> Spacing (min) |
| :--- | :---: | :---: | :---: | :---: |
| Major Arterial | N/A | 1,000 feet | 500 feet | 500 feet |
| Minor Arterial | N/A | 600 feet | 300 feet | 300 feet |
| Collector | 530 feet | 400 feet | 400 feet | 200 feet |
| Neighborhood | 530 feet | 200 feet | N/A | N/A |
| Local | 530 feet | 200 feet | N/A | N/A |

Note: Street Classifications are identified in the City TSP
*Limited Access - Vehicles are restricted to right-in/right-out turn movements. In some cases, left-in turn movements may be permitted.

1. Distance between streets is measures form the centerline of the subject street to the centerline of the adjacent street.
2. Local street connections are based on the Metro RTP requirements for new residential or mixed used developments.
3. Provide full street connections with spacing of no more than 530 feet between connections except where prohibited by barriers.
4. Provide bike and pedestrian access ways in-lieu-of streets with spacing of no more than 330 feet except where prevented by barriers.

## Design Modification Being Requested

The block length being 800 feet from the intersection of future Blake Road and SW $124^{\text {th }}$ Ave.
We are requesting approval for SW Cipole Place not to connect to future Blake Road (collector) and exceeding the 530 -foot maximum roadway spacing.

## Existing Conditions

The existing site is an undeveloped site, consisting of forested sections and grass land sloping from the southwest corner to the north east corner. In addition to the site being undeveloped, there are existing sensitive area consisting of wetlands and vegetated corridor buffers are located on the site limiting the development of the site.

Where Cipole Place cul-de-sac is proposed, the existing topography of the site is approximately 3 percent. As a result, the proposed Cipole Place is proposed at 3 percent grade and has minimal impact to sensitive areas, only impacting a small section of vegetated corridor. The topography from the end of the cul-de-sac to the future Blake Road is approximately 37 feet. This results in an average slope of 9 percent and would require significant earthworks.

## Result of Meeting Standards

To meet the City's standard block length of 530 feet on future Blake Rd, SW Cipole Place's alignment would need to be extended southward to Blake and its alignment would need to be revised east ward, impacting the existing wetlands and vegetated corridor located on the site. SW Cipole Road would need to be revised approximately 300 feet east encroaching further into the existing vegetated corridor and impacting the existing wetlands (see attached EX-1 Cipole Extension Site Impact Exhibit - Option A).

The road grade from Tualatin-Sherwood Road to the main vehicular entries at the cul-de-sac bulb is set at 3 percent to facilitate truck maneuvering and allow reasonable access to the lots without having to impact the wetlands. From the main vehicular entries to connect to Blake Rd, the road profile would need to be at 14.9 percent slope, thus requiring City Engineer approval (City standard dictate slope cannot exceed 15 percent, and any slopes above 12 percent require special approval from the City Engineer). Vertical curves required for the road profile would be designed at minimum K values (meeting AASHTO and City of Sherwood standards) and street lighting would be required (see attached EX-2 Cipole Extension Site Impact Exhibit - Profile View Option A).

In addition to the general non-desire for a 14.9 percent road, a secondary issue is safety. The 14.9 percent road presents a steep grade for vehicular traffic navigating the road, especially any potential truck traffic. Truck traffic navigating from Blake Rd on to Cipole and being able to stop on the steep grade for any vehicles leaving the main access driveway points is a safety concern, as increased stopping sight distance is needed to make the stop. In conjunction, standard vehicles traveling from Blake to Cipole will not be able to see the full access driveway due to the vertical curve, only being able to see the driveways once the vehicle has crested the vertical curve. Both situations create a hazardous traffic condition for the connection to Blake.

## Proposed Design Modification

The proposed SW Cipole Place will not connect to the future Blake Road at the maximum 530-foot block length to avoid impacting the existing wetlands and vegetated corridor.

## Reason Why Design Request Should be Approved

The design exception meets the criteria of Section 145.1.5.A. 2 because the existing topography and sensitive areas within the site cause a hardship of connecting SW Cipole Place to Future Blake Road. The existing topography from the end of the cul-de-sac to the future Blake Road will require a road profile of the connection is beyond the City's and AASHTO normal design standards. Whereas the 14.9 percent road grade and comfort vertical curves could be approved under a special exemption, the local industrial traffic makes this an unsafe roadway to exempt out of the standard. In addition, this extension of Cipole Place would impact additional vegetated corridor for the additional grading. This allow for the maximum developable area, minimize impacts to the existing sensitive areas (vegetated corridor and wetland) on the site, and maintain a safe road for the public, we propose not connecting SW Cipole Place to Blake Road.

Additionally, the design exception meets the criteria of Section 145.1.5.A. 3 because the connection of SW Cipole Place to future Blake Road will impose undue hardship of the project and have no material benefit to the public. The Kittelson \& Associates Traffic Impact Analysis, dated January 15, 2020, found if Cipole Place were to connect to Blake Rd, there would no material change in the traffic volumes going to all of the intersections evaluated. The TIS shows the connection does not provide significant volumes going to Blake Rd and the traffic volumes will utilize the intersection at TualatinSherwood Rd.

In addition, TVF\&R Fire Marshal has indicated a secondary connection to a public street is not required to provide coverage to the site, and the cul-de-sac is allowed if the buildings are fully sprinklered (all the proposed buildings are to be fully sprinklered).


3/4/2020
Date

Craig Christensen, P.E. - City Project Manager
Date

- Approved
- Approved with Conditions (conditions below or on attached sheet)
$\square$ Denied


$\begin{array}{ll}\text { TO: } & \begin{array}{l}\text { Bob Galati, P.E. - City Engineer } \\ \text { City of Sherwood }\end{array} \\ \text { FROM: } & \text { Ryan Halvorson, P.E. - Design Engineer } \\ \text { DATE: } & \text { March 4 }{ }^{\text {th }}, 2020 \\ \text { SUBJECT: } & \begin{array}{l}\text { Design Modification Request to Exclude Sidewalk and PUE on East Side of } \\ \text { Street }\end{array}\end{array}$


## Location of Requested Design Modification

The new SW Cipole Place street between Tualatin-Sherwood Rd and future Blake Rd.

## Current Standards

Standard Drawing RD-1 of the City of Sherwood Engineering Manual states for Standard Commercial/Industrial Streets Not Exceeding 3,000 Vehicles Per Day are to have 6-foot sidewalks on both sides of the street.

Under Section 16.118.020.B of the Sherwood Zoning and Community Development Code states public utility easements shall be a minimum of eight (8) feet in width unless a reduced with is specifically exempted by the City Engineer. An eight-foot-wide public utility easement (PUE) shall be provided on private property along all public street frontages. This standard does not apply to developments within the Old Town Overlay.

## Design Modification Being Requested

We are requesting approval to exclude the 6 -foot sidewalk and 8 -foot PUE on the east side of SW Cipole Place.

## Existing Conditions

The existing site is an undeveloped site, consisting of forested sections and grass land sloping from the southwest corner to the north east corner. In addition to the site being undeveloped, there are existing sensitive area consisting of wetlands and vegetated corridor buffers are located on the site limiting the development.

## Result of Meeting Standards

To meet the City's standard with having an 6 -foot sidewalk on the east side of SW Cipole Place would place the sidewalk over existing vegetated corridor. As a result, the 6 -foot sidewalk would impact additional vegetated corridor.

To meet the City's standard with having an 8-foot PUE on the east side of SW Cipole Place would place the PUE over existing vegetated corridor. As a result, the 8 -foot PUE would impact additional
vegetated corridor to install private franchise utilities, and the vegetated corridor would be impacted in the future by the private franchise utility providers installing additional lines within the PUE.

## Proposed Design Modification

We propose to alter the standard section, removing the 6 -foot sidewalk and 8 -foot PUE long the east side of the Cipole Place north of the cul-de-sac. The 6 -foot sidewalk along the west side of Cipole will provide pedestrian access to Lots 2 through 5 . Since the private franchise utilities which would be installed on this side of the street within the PUE would only serve the TSCP development.

## Reason Why Design Request Should be Approved

The design exception meets the criteria of Section 145.1.5.A. 2 because the existing wetlands and vegetated corridor will create a short and long term maintenance cost over the life of the project. The 6 -foot sidewalk and 8 -foot PUE on the east side of Cipole Place would be located over the existing vegetated corridor, impacting additional vegetated corridor for the project. In the future, the private franchise utility providers would impact the vegetated corridor to as potential additional utilities would need to be installed with the PUE.

The 6-foot sidewalk on the west side of the Cipole Place will remain to provide connectivity to all the lots fronting Cipole Place, providing pedestrian access within the right-of-way. The 8 -foot PUE on the west side of Cipole Place will adequately provide a backbone between Tualatin-Sherwood Road and Blake Road, as well as serve the TSCP development. The private franchise utilities installed on the east side of Cipole Place would serve only the TSCP development. As a result, we propose to eliminate the 6 -foot sidewalk and the 8 -foot PUE on the east side of Cipole Place north of the cul-desac.


$$
3 / 4 / 2020
$$

Date

Craig Christensen, P.E. - City Project Manager
Date

- Approved
- Approved with Conditions (conditions below or on attached sheet)
- Denied

Oregon
Kate Brown, Governor

Re: WD \# 2020-0015 Approved
Wetland Delineation Report for the T-S Corporate Park Washington County; T2S R1W S28D TL1100 (Portion)

Bev Clarno Secretary of State

Tobias Read
Dear Mr. Olsen:
The Department of State Lands has reviewed the wetland delineation report prepared by Pacific Habitat Services for the site referenced above. Please note that the study area includes only a portion of the tax lot described above (see the attached maps). Based upon the information presented in the report, and additional information submitted upon request, we concur with the wetland and waterway boundaries as mapped in revised Figure 6, 6A, and 6B of the report. Please replace all copies of the preliminary wetland maps with these final Department-approved maps.

Within the study area, 3 wetlands (Wetland A, B and C, totaling approximately 2.91 acres) and one roadside ditch were identified. The wetlands are subject to the permit requirements of the state Removal-Fill Law. Under current regulations, a state permit is required for cumulative fill or annual excavation of 50 cubic yards or more in wetlands or below the ordinary high-water line (OHWL) of the waterway (or the 2-year recurrence interval flood elevation if OHWL cannot be determined). The roadside ditch is exempt per OAR 141-085-0515(10) except for the area that may be contiguous with Wetland A offsite.

This concurrence is for purposes of the state Removal-Fill Law only. We recommend that you attach a copy of this concurrence letter to any subsequent state permit application to speed application review. Federal or local permit requirements may apply as well. The U.S. Army Corps of Engineers will determine jurisdiction under the Clean Water Act, which may require submittal of a complete Wetland Delineation Report.

Please be advised that state law establishes a preference for avoidance of wetland impacts. Since measures to avoid and minimize wetland impacts may include reconfiguring parcel layout and size or development design, we recommend that you work with Department staff on appropriate site design before completing the city or county land use approval process.

This concurrence is based on information provided to the agency. The jurisdictional determination is valid for five years from the date of this letter unless new information necessitates a revision. Circumstances under which the Department may change a determination are found in OAR 141-090-0045 (available on our web site or upon request). In addition, laws enacted by the legislature and/or rules adopted by the Department may result in a change in jurisdiction; individuals and applicants are subject to the regulations that are in effect at the time of the removal-fill activity or complete permit application. The applicant, landowner, or agent may submit a request for reconsideration of this determination in writing within six months of the date of this letter.

Thank you for having the site evaluated. If you have any questions, please contact Chris Stevenson, the Jurisdictional Coordinator for Clackamas County at (503) 986-5246.

Sincerely,


Peter Ryan, PWS
Aquatic Resource Specialist

## Enclosures

ec: Shawn Eisner, Pacific Habitat Services City of Sherwood Planning Department Carrie Bond, Corps of Engineers
Anita Huffman, DSL Lindsey Obermiller, Clean Water Services

## WETLAND DELINEATION / DETERMINATION REPORT COVER FORM

Fully completed and signed report cover forms and applicable fees are required before report review timelines are initiated by the Department of State Lands. Make the checks payable to the Oregon Department of State Lands. To pay fees by credit card, go online at: https://apps.oregon.gov/DSL/EPS/program?key=4.
Attach this completed and signed form to the front of an unbound report or include a hard copy with a digital version (single PDF file of the report cover from and report, minimum 300 dpi resolution) and submit to, Oregon Department of State Lands, 775 Summer Street NE, Suite 100, Salem, OR 97301-1279. A single PDF of the completed cover form and report may be e-mailed to
Wetland_Delineation@dsl.state.or.us. For submittal of PDF files larger than $10 \mathrm{MB}, \mathrm{e}$-mail DSL instructions on how to access the file from your ftp or other file sharing website.

## Contact and Authorization Information

$\boxtimes$ Applicant $\square$ Owner Name, Firm and Address:
Trammel Crow Company
Attn: Kirk Olsen
1300 SW Fifth Avenue, Suite 3050
Portland, OR 97201
$\square$ Authorized Legal Agent, Name and Address:

Business phone \# 503-946-4981
Mobile phone \# (optional)
E-mail: KOIsen@trammellcrow.com

Business phone \# Mobile phone \# E-mail:

I either own the property described below or I have legal authority to allow access to the property. I authorize the Department to access the property for the purpose of confirming the information in the report, after prior notification to the primary contact.
Typed/Printed Name: Kirk L. Olsen
Signature: KirkL. 0 Is en
Date: 1/7/20 Special instructions regarding site access:

## Project and Site Information

| Project Name: T-SCorporate Park | Latitude: Longitude: <br> $45.3682^{\circ} \mathrm{N}$ $-122.8103^{\circ} \mathrm{W}$ |
| :---: | :---: |
|  | decimal degree - centroid of site or start \& end points of linear project |
|  | Tax Map \# 2S128D <br> Tax Lot(s) 1100 (portion) |
| Proposed Use: Light Industrial | Tax Map \# Tax Lot(s) |
| Project Street Address (or other descriptive location): 12900 SW Tualatin-Sherwood Road, North portion of tax lot | Township Range Section QQ <br> 2S $1 W$ 28 SE $1 / 4$ |
|  | Waterway: NA <br> River Mile: n/a <br> NWI Quad(s): Sherwood |
| Wetland Delineation Information |  |
| Wetland Consultant Name, Firm and Address: Phone \# 503-570-0800 <br> Pacific Habitat Services Mobile phone \# <br> Attn: Shawn Eisner E-mail: se@pacifichabitat.com <br> 9450 SW Commerce Circle, Suite 180  <br> Wilsonville, OR 97070  <br> The information and conclusions on this form and in the attached report are true and correct to the best of my knowledge.  <br> Consultant Signature: Date: $\mathbf{1 / 3 / 2 0}$ |  |
| Primary Contact for report review and site access is $\boxtimes$ Consultant $\square$ Applicant/Owner $\square$ Authorized Agent |  |
|  |  |
| Check Applicable Boxes Below |  |
| $\square$   <br> $\square$ R-F permit application submitted $\square$ Fee payment submitted \$466 <br> $\square$ Mitigation bank site $\square$ Fee (\$100) for resubmittal of rejected report <br> $\square$ Industrial Land Certification Program Site $\square$ Request for Reissuance. See eligibility crite <br> $\square$ Wetland restoration/enhancement project (not mitigation)  DSL \# $\quad \square$ <br> $\boxtimes$ Previous delineation/application on parcel? $\square$ LWI shows wetlands or waters on parcel? <br> If Known, previous DSL \# 2014-0448, 2017-0006 \& 0008  Wetland ID Code |  |
| For Office Use Only |  |
| DSL Reviewer: C.S. Fee Paid Date: <br> Date Delineation Received: $1 / 19120 \quad$ Scan  | ' $\quad$ ' ${ }^{\prime}$ :DSL WD \# 2020-0015 |


\#6163
12/19/2019
PHS 券
General Location and Topography T-S Corporate Park-Sherwood, Oregon United States Geological Survey (USGS), Sherwood, Oregon, 7.5 Quadrangle, 2014
(viewer/nationalmap.gov/basic)

FIGURE 1





X:|Project Directories\6100|6163 ORR Property\AutoCAD\Plot Dwgs\Fig6B WetDel.dwg, 1/7/2020 12:48:36 PM, AutoCAD PDF (High Quality Print).pc3









## CleanWater Services

Our commitment is clear.

## Service Provider Letter

This form and the attached conditions will serve as your Service Provider Letter in accordance with Clean Water Services Design and Construction Standards (R\&O 19-5, as amended by R\&O 19-22).


Encroachments into Pre-Development Vegetated Corridor:

| Type and location of Encroachment: | Square Footage: |
| :--- | :--- | :--- |
| Grading \& retaining walls for road, parking lots, \& stormwater outfalls (Permanent encroachment; |  |
| Mitigation required) | 10,699 |
| Storm pipe installation (Temporary encroachment; Restoration and planting in-place required) | $-4,917$ |
|  | - |

Mitigation Requirements:

| Type/Location | Sq. Ft./Ratio/Cost |
| :--- | :--- |
| On-site Replacement Mitigation |  |
|  |  |

$\mathbf{X}$ Conditions Attached $\quad \mathbf{X}$ Development Figures Attached ( 4 ) $\square$ Planting Plan Attached $\square$ Geotech Report Required
This Service Provider Letter does NOT eliminate the need to evaluate and protect water quality sensitive areas if they are subsequently discovered on your property.

## In order to comply with Clean Water Services water quality protection requirements the project must comply with the following conditions:

1. No structures, development, construction activities, gardens, lawns, application of chemicals, uncontained areas of hazardous materials as defined by Oregon Department of Environmental Quality, pet wastes, dumping of materials of any kind, or other activities shall be permitted within the sensitive area or Vegetated Corridor which may negatively impact water quality, except those allowed in R\&O 19-5, Chapter 3, as amended by R\&O 19-22.
2. Prior to any site clearing, grading or construction the Vegetated Corridor and water quality sensitive areas shall be surveyed, staked, and temporarily fenced per approved plan. During construction the Vegetated Corridor shall remain fenced and undisturbed except as allowed by R\&O 19-5, Section 3.06.1, as amended by R\&O 19-22 and per approved plans.
3. If there is any activity within the sensitive area, the applicant shall gain authorization for the project from the Oregon Department of State Lands (DSL) and US Army Corps of Engineers (USACE). The applicant shall provide Clean Water Services or its designee (appropriate city) with copies of all DSL and USACE project authorization permits. No wetland or non-wetland work proposed with this project.
4. An approved Oregon Department of Forestry Notification is required for one or more trees harvested for sale, trade, or barter, on any non-federal lands within the State of Oregon.
5. Prior to ground disturbing activities, an erosion control permit is required. Appropriate Best Management Practices (BMP's) for Erosion Control, in accordance with Clean Water Services' Erosion Prevention and Sediment Control Planning and Design Manual, shall be used prior to, during, and following earth disturbing activities.
6. Prior to construction, a Stormwater Connection Permit from Clean Water Services or its designee is required pursuant to Ordinance 27, Section 4.B.
7. Activities located within the 100-year floodplain shall comply with R\&O 19-5, Section 5.10 , as amended by R\&O 19-22.
8. Removal of native, woody vegetation shall be limited to the greatest extent practicable.
9. The water quality swale and detention pond shall be planted with Clean Water Services approved native species, and designed to blend into the natural surroundings.
10. Should final development plans differ significantly from those submitted for review by Clean Water Services, the applicant shall provide updated drawings, and if necessary, obtain a revised Service Provider Letter.
11. The Vegetated Corridor width for sensitive areas within the project site shall be a minimum of 25-50 feet wide, as measured horizontally from the delineated boundary of the sensitive area.
12. For Vegetated Corridors that extend 35 feet from the break in slope, the width of Vegetated Corridors may be reduced to 15 feet wide if a stamped geotechnical report confirms that slope stability can be maintained with the reduced setback from the break in slope.
13. For Vegetated Corridors greater than 50 feet in width, the applicant shall enhance the first 50 feet closest to the sensitive area to meet or exceed good corridor condition as defined in R\&O 19-5, Section 3.14.2, Table 3-3, as amended by R\&O 19-22.
14. Removal of invasive non-native species by hand is required in all Vegetated Corridors rated ""good."" Replanting is required in any cleared areas larger than 25 square feet using low impact methods. The applicant shall calculate all cleared areas larger than 25 square feet prior to the preparation of the required Vegetated Corridor enhancement/restoration plan.
15. Prior to any site clearing, grading or construction, the applicant shall provide Clean Water Services with a Vegetated Corridor enhancement/restoration plan. Enhancement/restoration of the Vegetated Corridor shall be provided in accordance with R\&O 19-5, Appendix A, as amended by R\&O 19-22, and shall include planting specifications for all Vegetated Corridor, including any cleared areas larger than 25 square feet in Vegetated Corridor rated ""good.""
16. Prior to installation of plant materials, all invasive vegetation within the Vegetated Corridor shall be removed per methods described in Clean Water Services' Integrated Pest Management Plan, 2019. During removal of invasive vegetation care shall be taken to minimize impacts to existing native tree and shrub species.
17. Clean Water Services and/or the City shall be notified 72 hours prior to the start and completion of enhancement/restoration activities. Enhancement/restoration activities shall comply with the guidelines provided in Planting Requirements (R\&0 19-5, Appendix A, as amended by R\&O 19-22).
18. Maintenance and monitoring requirements shall comply with R\&O 19-5, Section 2.12.2, as amended by R\&O 19-22. If at any time during the warranty period the landscaping falls below the $80 \%$ survival level, the owner shall reinstall all deficient planting at the next appropriate planting opportunity and the two year maintenance period shall begin again from the date of replanting.
19. Performance assurances for the Vegetated Corridor shall comply with R\&O 19-5, Section 2.07.2, Table 2-1 and Section 2.11, Table 2-2, as amended by R\&O 19-22.
20. For any developments which create multiple parcels or lots intended for separate ownership, Clean Water Services shall require that the sensitive area and Vegetated Corridor be contained in a separate tract and subject to a ""STORM SEWER, SURFACE WATER, DRAINAGE AND DETENTION EASEMENT OVER ITS ENTIRETY"" to be granted to the City or Clean Water Services.
21. Clean Water Services shall require an easement over the Vegetated Corridor conveying storm and surface water management to Clean Water Services or the City that would prevent the owner of the Vegetated Corridor from activities and uses inconsistent with the purpose of the corridor and any easements therein.

## FINAL PLANS

22. Final construction plans shall include landscape plans. In the details section of the plans, a description of the methods for removal and control of exotic species, location, distribution, condition and size of plantings, existing plants and trees to be preserved, and installation methods for plant materials is required. Plantings shall be tagged for dormant season identification and shall remain on plant material after planting for monitoring purposes.
23. A Maintenance Plan shall be included on final plans including methods, responsible party contact information, and dates (minimum two times per year, by June 1 and September 30).
24. Final construction plans shall clearly depict the location and dimensions of the sensitive area and the Vegetated Corridor (indicating good, marginal, or degraded condition).
Sensitive area boundaries shall be marked in the field.
25. Protection of the Vegetated Corridors and associated sensitive areas shall be provided by the installation of permanent fencing and signage between the development and the outer limits of the Vegetated Corridors. Fencing and signage details to be included on final construction plans.

This Service Provider Letter is not valid unless CWS-approved site plan is attached.
Please call (503) 681-3653 with any questions.


Lindsey Obermiller
Environmental Plan Review



Site Development Plan and Vegetated Corridor Encroachment FIGURE


LEGEND

-     -         -             - Project Area Boundary
$\square$ Wetland
(Site Total 159,518 sf / 3.66 ac )
=ーー= Vegetated Corridor
Vegetated Corridor
Vegetated Corridor
Permanent Encroachment
(10,699 sf / 0.25 ac )
Vegetated Corridor
Temporary Encroachment
(4,917 sf / 0.11 ac )


(UsersLLisalDestopp|WorkFromHomel6163 Orr|Fig4C DevPlan.dwg, 3/19/2020 4:36:51 PM, AutoCAD PDF (High Quality Print).pc3


[^0]:    ${ }^{1}$ Actual traffic volumes will likely be lower since the TIA was based on 547,220 SF of building area while the current proposal is for 535,194 SF.

[^1]:    ${ }^{2}$ The TEA Implementation Plan built on the work in earlier planning documents including transportation and utility master plans, the Tonquin Employment Area concept plans, and the City's economic development strategy.

[^2]:    ${ }^{3}$ On the proposed grading plan, the SW Cipole Road/Tualatin-Sherwood Road intersection would be at elevation 189'; future Blake Road intersects SW 124th Avenue at elevation 233' and the existing topography slopes uphill as the corridor proceeds to the west.
    ${ }^{4}$ Designing for Truck Movements and other Large Vehicles in Portland, October 8, 2008, City of Portland, recommends a slope of no more than $5 \%$ for local streets in industrial areas. Recent communication with Bob Hillier, the current City of Portland Freight Coordinator, indicates that this best practice continues to be observed by City of Portland staff.

[^3]:    G. Streets Adjacent to Railroads

    Streets adjacent to railroads shall run approximately parallel to the railroad and be separated by a distance suitable to allow landscaping and buffering between the street and railroad. Due

[^4]:    ${ }^{5}$ The term Significant Natural Area, as used in this section, is not defined.

[^5]:    
     Revsonscheoule
    $\qquad$
    $\qquad$

[^6]:    ${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or critical movement delay (TWSC), HCM 6 ${ }^{\text {th }}$ Ed. Level-of-Service and average delay per vehicle in seconds (roundabout);
    ${ }^{2}$ HCM 2000 Volume-to-Capacity ratio. For TWSC intersections, the critical movement is shown in parenthesis;
    ${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan (RTFP).

[^7]:    圆图园
    
     and delete this message along with any attachments or links from your system．

[^8]:    Zone wide Queuing Penalty: 487

[^9]:    ${ }^{1}$ Tualatin Sherwood Road (Teton Avenue to Langer Farms Parkway) Project

[^10]:     illustrated in Figure 17.

[^11]:    ${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or critical movement delay (TWSC).
    ${ }^{2}$ HCM 2000 Volume-to-Capacity ratio (signalized). For TWSC intersections, the critical movement is shown;
    ${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan.

[^12]:    ${ }^{1}$ HCM 2000 Level-of-Service and average delay per vehicle in seconds (signalized) or critical movement delay (TWSC), HCM 6 ${ }^{\text {th }}$ Ed. Level-of-Service and average delay per vehicle in seconds (roundabout);
    ${ }^{2}$ HCM 2000 Volume-to-Capacity ratio (signalized) or HCM 6 ${ }^{\text {th }}$ Ed. Volume-to-Capacity ratio (roundabout). For TWSC intersections, the critical
    movement is shown;
    ${ }^{3}$ Regional jurisdiction is governed by the Regional Transportation Functional Plan.

[^13]:    Zone wide Queuing Penalty: 129

[^14]:    1. Source: NRCS WETS Table (period from 1995 to 2018) and Climatological Data for KGW-TV in Portland, OR (http:// http://agacis.rcc-acis.org/?fips=41051)
[^15]:    Remarks:

[^16]:    Remarks:

[^17]:    Remarks:

[^18]:    Remarks:

[^19]:    Remarks:

[^20]:    Remarks:

[^21]:    Remarks:

[^22]:    Remarks:

[^23]:    Remarks:

[^24]:    Remarks:

[^25]:    Pacific Habitat Services, Inc.
    Natural Resource Assessment for T-S Corporate Park / PHS \#6163

[^26]:    CMC:GJS:GPS:kt
    Attachments
    One copy submitted (via email only)
    Document ID: TrammellCr-74-01-020619-geolr.docx
    © 2019 GeoDesign, Inc. All rights reserved.

[^27]:    ${ }^{1}$ ASTM, 2006, D-5777-00 Standard Guide for Using the Seismic Refraction Method for Subsurface Investigation.

[^28]:    ${ }^{2}$ Handbook of Ripping, 1997; Caterpillar, Inc., 11 th edition, Peoria, IL, 30 p.

[^29]:    ${ }^{1}$ GeoDesign, Inc., 2019. Preliminary Geotechnical Engineering Services; Orr Property; SW $124^{\text {th }}$ Avenue and SW TualatinSherwood Road; Washington County, Oregon, dated February 6, 2019. GeoDesign Project: TrammellCr-74-01

[^30]:    Teragan \& Associates, Inc.
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